Hydrogeology of the Caulkins Water Farm Project

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EXECUTIVE SUMMARY

The Caulkins Water Farm Project (CWFP) encompasses a 3,014-acre surface water impoundment adjacent to the C-44 Canal (St. Lucie River) in southern Martin County, Florida. The CWFP includes five cells, separated by earthen berms, that range from approximately 400 to 798 acres in size. Approximately 20,192 acre-feet of water was pumped into the impoundment during the operational period of December 8, 2017 through November 2, 2018. Prior to construction of the CWFP, a pilot project comprising a 414-acre surface water impoundment in the southwestern portion (Cell 1) of the existing CWFP was operational from February 2014 through October 2016.

A total of 19 groundwater monitor wells and 6 surface water stations were installed within and adjacent to the project to characterize site lithology and conduct continuous water level monitoring and water quality sampling. Additional data sources for this report included lithologic logs, geophysical logs, and aquifer performance test data collected prior to construction of the project.

The surficial aquifer system (SAS) is approximately 140 to 150 feet (ft) thick and is divided into three units at the CFWP site. The uppermost unit (Unit 1) was found to consist of predominantly silty sand with irregular interbeds of clayey sand grading to sandy clay and sandy, calcareous clay from surface to approximately 18 ft below land surface (bls). Discontinuous clayey sand or sandy clay layers ranging from 2 to 12 ft in thickness were observed in each of the soil borings within the CWFP footprint at various depths. Slug tests and short-term aquifer performance tests at the CWFP indicated an average horizontal hydraulic conductivity (Kh) of 77 and 8 ft/day, respectively. Laboratory test data at the C-44 Reservoir/STA to the west indicated an average vertical hydraulic conductivity (Kv) of 0.10 ft/day. Unit 2 composes the bulk of the SAS and predominantly consists of poorly graded quartz sand, silty sand, shell, and a few interbeds of sandstone less than 2 ft thick. Locally interbedded units of clayey sand grading to sandy clay and coquina (Unit 3) were found within Unit 2. Twenty-four-hour, multi-well aquifer performance tests within the CWFP and the C-44 Reservoir/STA indicated a range of Kh from 20 to 51 ft/day, and a range of Kv from 0.35 to 1 ft/day. Unit 3 is a less permeable, poorly consolidated granular limestone and coquina with sand, shell and calcareous clay that occurs at the base of the SAS and also is interbedded at relatively shallow depths within Unit 2.

Site hydrogeology is spatially variable, with less permeable sediments observed in the upper and middle portions of the SAS in the eastern half (Cells 4 and 5) and northwestern cell (Cell 3) relative to Cells 1 (pilot project cell) and 2 of the CWFP. These sediments include thicker and potentially more continuous clayey sand in the shallow sand layer; interbedded coquina with shell in Unit 2; and a relatively thick section of clayey sand, grading to sandy clay interbedded in the middle of Unit 2. Well control in Cell 2 was not deep enough to evaluate permeability in Unit 2. Reduced permeability may be responsible for the lower seepage observed during the operational period of the expanded CWFP, approximately 0.022 ft/day, including evapotranspiration, compared to 0.051 ft/day during the pilot project operational period, excluding evapotranspiration. (Evapotranspiration and rain were observed to largely cancel out during the pilot test operational period.) During January 2019, flow between cells was restricted, and seepage from Cells 3, 4, and 5 was approximately 20 to 50 percent of the seepage observed from Cell 1, consistent with the low-permeability sediments observed.

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ACRONYMS AND ABBREVIATIONS

ac-ft	acre-feet
APT	aquifer performance test
bls	below land surface
cm/s	centimeters per second
CWFP	Caulkins Water Farm Project
ET	evapotranspiration
ft	foot
ft/day	feet per day
gpm	gallons per minute
К	hydraulic conductivity
Kh	horizontal hydraulic conductivity
Kv	vertical hydraulic conductivity
SAS	surficial aquifer system
SFWMD	South Florida Water Management District
SPT	standard penetration test
STA	stormwater treatment area
USACE	United States Army Corps of Engineers

1 INTRODUCTION

The Caulkins Water Farm Project (CWFP), implemented as part of the South Florida Water Management District's (SFWMD's) Dispersed Water Management Program, encompasses a 3,014-acre surface water impoundment adjacent to the C-44 Canal (St. Lucie River) in southern Martin County, Florida. Construction of the CWFP was completed in 2017. Pumping commenced on December 8, 2017 and continued intermittently through November 2, 2018. Approximately 20,192 acre-feet (ac-ft) of water was pumped into the impoundment during that period, with a seepage rate of 0.022 feet per day (ft/day), not including evapotranspiration (ET). Prior to construction of the CWFP, a pilot project comprising a 414-acre surface water impoundment in the southwestern cell (Cell 1) of the existing CWFP was constructed between August and December 2013. Pumping into the pilot project impoundment began on February 2, 2014 and continued intermittently through October 26, 2016 (approximately 33 months). In 2014 and 2015, 14 groundwater and 6 surface water stations were installed within and adjacent to the pilot project for continuous water level monitoring and water quality sampling. Previous reports documented station installation and provided analysis of site hydrogeology, water quality, and groundwater seepage at the pilot project site (Janzen et al. 2015, 2017).

2 SITE SETTING AND DESCRIPTION

The CWFP was constructed on former agricultural property. The area is bordered by agricultural land to the north and west and by agricultural and undeveloped land to the east. To the south is undeveloped property, County Highway 726 (Citrus Boulevard), and the C-44 Canal. Prior to construction of the CWFP and pilot project, the property was a citrus grove and leased for farming.

The CWFP includes five cells separated by earthen berms approximately 7 feet (ft) above grade (**Figure 1**). The cells range from approximately 414 to 798 acres in area and are connected to each other by 60-inch diameter gated culverts. Perimeter ditches are adjacent to the exterior of each cell and connected by emergency discharge pipes. A pump station is located approximately 300 ft south of Cell 1 for transferring water from the C-444 Canal (connected to the C-44 Canal) via three electric 35,000-gallon per minute (gpm) pumps.



Figure 1. Site map of the Caulkins Water Farm Project in Martin County, Florida.

3 METHODS

Hydrogeologic data collected during previous investigations (prior to construction of the pilot project) and recent investigations conducted by the SFWMD during operation of the pilot project in 2014-2015 and the CWFP in 2018 are summarized below.

Previous investigations within the CWFP footprint and vicinity include the following:

- A groundwater flow model report of the surficial aquifer system (SAS) in Martin County (Adams 1992) and hydrogeologic investigation data report (Lukasiewicz and Adams-Smith 1996) by the SFWMD. These reports provide detailed regional descriptions of the hydrogeology in Martin County, including the CWFP area, and site-specific data from installation and hydraulic testing of SAS monitor well Caulk_PW, installed to a depth of approximately 110 ft below land surface (bls) in Cell 3. Drilling methodology for Caulk_PW was mud-rotary, which does not provide a lithologic sample collection as representative as a standard penetration test (SPT), which was used for the other soil borings within the CWFP footprint.
- Eight SPT soil borings collected in 2006 within the impoundment footprint as part of a geotechnical investigation of the project area (Anderson Andre Consulting Engineers Inc., 2006). The borings, TW-1 through TW-8, were advanced to a depth of approximately 40 ft bls.
- Two aquifer performance tests (APTs) conducted at monitor wells approximately 1 to 6 miles west of the CWFP within the C-44 Reservoir/Stormwater Treatment Area (STA). The wells, W-101 and W-102, were installed to a depth of approximately 135 ft bls (United States Army Corps of Engineers [USACE] 2014).

Lithology and/or geophysical logs were obtained for three wells outside the CWFP footprint: W-12268 (completed 1974), approximately 600 ft south of the CWFP; M-1236, completed in 1988 and geophysically logged in 2017, approximately 1,600 ft east of the CWFP; and MF-52R, completed in 2016, approximately 4,000 ft west of the CWFP.

Recent investigations within the CWFP footprint and vicinity include:

- The SFWMD installed 14 groundwater monitor wells and 6 stilling wells within and adjacent to the impoundment during the pilot project (Janzen et al. 2017). **Table 1** provides details for the monitor well stations, CAU-1 through CAU-7. The monitor wells included three standalone shallow wells (approximately 15 to 20 ft bls) and four SAS well clusters, with well depths from 10 to 140 ft bls. Samples were collected for lithologic description using SPT methods, with plastic-lined cores and drill cuttings. Geophysical logs were run at CAU-1LD in the center of the impoundment. In 2014 and 2015, slug tests and/or short-term APTs were performed on the central and northern well clusters within the pilot project area and shallow perimeter wells and are described in detail in the first and second annual reports (Janzen et al. 2015, Janzen 2017).
- Five groundwater monitor wells were installed in February 2018, shortly after completion of the CWFP. The wells included a deep well at CAU-7 (CAU-7LD), south of the CWFP; a well cluster (CAU-8M and CAU-8LD) in the northern-central portion of the CWFP (on the levee between Cells 3 and 4); and a well cluster (CAU-9M and CAU-9LD) approximately 1,600 ft east of the CWFP (**Figure 1**). During well installation, samples were collected for lithologic description using SPT methods, with plastic-lined cores and drill cuttings. Geophysical logs were run on the two deepest wells at each well cluster. Short-term APTs were performed at CAU-8 and M-1236.

Lithologic descriptions of the four monitor wells installed to the lower-deep screen intervals (CAU-1LD, CAU-7LD, CAU-8LD, CAU-9LD) are included in **Appendix A**. Geophysical logs for those wells and for M-1236 are included in **Appendix B**.

Monitor Well	Total Depth (ft bls)	Cased Depth (ft)	Screen Slot (inches)	Screen Length (ft)	Ground Level Elevation	Top of Casing Elevation	Bottom Screen Elevation	Aquifer Depth	Location
CAU-1S	9.5	7.5	0.02	2	27.1	36.2	17.6	Shallow	CWFP Cell 1
CAU-1M	23	13	0.02	10	26.9	36.3	3.7	Middle	CWFP Cell 1
CAU-1D	72	62	0.02	10	27.0	36.2	-44.9	Deep	CWFP Cell 1
CAU-1LD	130	120	0.02	10	26.3	36.1	-103.4	Lower Deep	CWFP Cell 1
CAU-2S	16	14	0.02	2	32.6	32.2	16.3	Shallow	CWFP between Cells 1 and 5
CAU-3S	16	14	0.02	2	28.6	28.4	12.5	Shallow	CWFP south of Cell 1
CAU-4S	16	14	0.02	2	32.3	31.9	15.8	Shallow	West of Cell 1
CAU-5S	16	14	0.02	2	32.8	32.3	16.5	Shallow	CWFP between Cells 1 and 2
CAU-5M	31	21	0.02	10	32.8	32.4	1.9	Middle	CWFP between Cells 1 and 2
CAU-5D	79	69	0.02	10	32.8	32.5	-46.6	Deep	CWFP between Cells 1 and 2
CAU-6M	33	23	0.02	10	40.1	39.7	6.9	Middle	North of C-44 Canal
CAU-6D	79	69	0.02	10	40.1	39.6	-39.2	Deep	North of C-44 Canal
CAU-7M	32	22	0.02	10	35.6	35.3	3.4	Middle	North of C-44 Canal
CAU-7D	80	70	0.02	10	35.6	35.3	-44.2	Deep	North of C-44 Canal
CAU-7LD	140	130	0.02	10	35.2	35.2	-104.8	Lower Deep	North of C-44 Canal
CAU-8M	28	18	0.02	10	32.5	32.3	4.3	Middle	CWFP between Cells 2 and 3
CAU-8LD	136	126	0.02	10	32.5	32.5	-103.5	Lower Deep	CWFP between Cells 2 and 3
CAU-9M	25	15	0.02	10	17.6	17.0	-8.0	Middle	East of Cell 5
CAU-9LD	116	106	0.02	10	17.6	22.3*	-98.7	Lower Deep	East of Cell 5
M-1236	104	94	ND	10	ND	24.9	-79.1	Lower Deep	East of CWFP

 Table 1.
 Well construction details for the monitor wells at the Caulkins Water Farm Project.

bls = below land surface; ft = foot; ND = no data available.

Notes: Elevations are provided in feet National Geodetic Vertical Datum of 1929. Casing for all wells was polyvinyl chloride (PVC).

* Elevation was measured from the top of 2-inch stick-up casing.

4 SITE HYDROGEOLOGY

4.1 Hydrogeologic Framework

The SAS in Martin County is estimated to be approximately 140 to 150 ft thick in the vicinity of the CWFP and composed of a sequence of sand, silt, shell, and limestone. The SAS is unconfined to semi-confined in Martin County and underlain by the confining sediments of the Hawthorn Group. The Martin County SAS groundwater flow model (Adams 1992) and corresponding data report (Lukasiewicz and Adams-Smith 1996) divided the SAS into three hydrogeologic layers (units), each with lateral and vertical variability and discontinuity:

• The uppermost unit (Unit 1) is described as an unconsolidated sand/soil unit with very fine to course-grained quartz sand and interbedded lenses of shell, sandy clay, and silt. The unit has low to moderate permeability and is estimated to be approximately 20 ft thick in the vicinity of the CWFP.

- The bulk of the SAS is composed of Unit 2, also referred to as the production zone within the SAS in Martin and St. Lucie counties. Unit 2 consists of unconsolidated quartz sand and shell beds with thin beds of sandstone and has the highest permeability in the SAS. It interfingers with the less permeable granular limestone of Unit 3.
- Unit 3 is a less permeable, poorly consolidated, granular limestone with sand and calcareous clay. In the vicinity of the CWFP, Unit 3 is shown as the lowermost unit in the SAS and can be interbedded within Unit 2 as shallow as 15 ft bls.

Based on soil boring data gathered for this report, the hydrogeology of the CWFP is consistent with regional hydrogeology described by Adams (1992) and Lukasiewicz and Adams-Smith (1996). Geologic cross-sections A–A' and B–B', showing site lithology, are presented in **Figures 2** and **3**, respectively. Unit 1 was found to consist of predominantly silty sand, with irregular interbeds of clayey sand grading to sandy clay and sandy calcareous clay from the surface to approximately 18 ft bls. Discontinuous clayey sand or sandy clay layers, ranging from 2 to 13 ft in thickness, were observed in each of the SPT soil borings within the footprint of the CWFP. In geophysically logged wells, the clayey sections were indicated by reductions in induction conductivity to below 20 ohm-meters. As shown in **Figure 2**, these intervals appeared thicker and potentially more continuous in soil borings advanced in Cells 3, 4, and 5 relative to Cells 1 (pilot project cell) and 2. Thicknesses of the clayey sand and sandy clay observed typically were 9 to 13 ft in Cells 3, 4, and 5; compared to approximately 7 ft in Cell 1 and 8 ft in Cell 2 (**Figures 2** and **3**).

Two soil borings at CAU-8 in the northern portion of the CWFP (on the levee between Cells 3 and 4) and CAU-9 (approximately 1,600 ft east of the CWFP) found beds of clayey sand grading to sandy clay within Unit 2, from approximately 59 to 81 ft bls at CAU-8 and 64 to 72 ft bls at CAU-9. The clayey sections were indicated by a reduction in induction conductivity to approximately 20 ohm-meters. The consistent depths of these beds suggest they may be continuous across much of the northeastern and eastern portions of the CWFP (**Figure 2**). Up to 6 ft of sandy silt was encountered at a similar depth (60 to 72 ft bls) at CAU-1 in the center of Cell 1; however, this unit did not appear clayey and a reduction in induction conductivity was not observed.

Soil borings within the CWFP encountered shell beds and coquina representative of Unit 3 interbedded within Unit 2 at depths between 15 and 38 ft bls in the northeastern portion of the CWFP. The unit is described as poorly cemented, sandy coquina, with abundant unconsolidated shell fragments. The unit was between 5 and 10 ft thick in soil borings from CAU-8LD, TB-2, TB-3, and TB-5, all within or adjacent to Cell 4. The consistent depths of these beds suggest they may be continuous across much of the northeastern portion of the CWFP. Unit 3 was identified in lithology logs near the base of the SAS in M-1236 and W-12688 southeast of the impoundment.



Figure 2. Geologic cross-section A–A'.



Figure 3. Geologic cross-section B–B'.

4.2 Summary of Hydraulic Testing

The SFWMD conducted 11 slug tests and 8 short-term APTs within the CWFP at the wells installed in 2015 and 2016 (Janzen et al. 2015, 2017) and in 2018 as part of the current investigation. Additionally, published test data were reviewed, including an on-site APT and APTs conducted at the C-44 Reservoir/STA approximately 1 to 6 miles west of the CWFP.

4.2.1 Recent Short-Term Aquifer Performance Tests

The SFWMD conducted two short-term APTs as part of the current investigation to determine the hydraulic characteristics of the SAS underlying the expanded portions of the CWFP. Two locations were chosen to conduct additional APTs: CAU-8LD, in the northern end of the CWFP footprint between Cells 3 and 5; and M-1236, east of the CFWP (**Figure 4**). Construction information for the tested wells is provided in **Table 1**.



Figure 4. Location of aquifer performance test sites at the Caulkins Water Farm Project site.

<u>Methods</u>

On August 16, 2017, an APT was conducted at M-1236, which is completed in the SAS to a depth of 103 ft bls. A Level Troll® 700 was installed in M-1236 prior to the test in order to record changes in water levels, and the test was configured using Win-situ® (Version 5) software (In-Situ, Inc. 2012). A pumping rate of 6 gpm was initiated, and there was a temporary drop in pumping rate shortly after commencement of the pumping phase. After approximately 1 hour, pumping ceased, and water level data were recorded for an additional 12 minutes. M-1236 groundwater levels returned to background levels during this time. The maximum drawdown observed was 8.29 ft.

Following construction of CAU-8LD, an APT was performed on April 4, 2018. In addition to instrumentation and configuration as described above, a Level Troll® 700 was deployed in CAU-8M as an observation well for the APT. Both wells are completed in the SAS. No changes in water levels were recorded in CAU-8M during the test. CAU-8LD was pumped at a rate of 0.25 gpm, and a maximum drawdown of 34.43 ft was observed approximately 36 minutes into the test. At that time, the well had pumped dry and recovery was initiated. CAU-8LD was left overnight to allow enough time for groundwater levels to return to background conditions.

The results were graphed in Excel and the data sets imported into AQTESOLOV Pro (Version 4.5) software (HydroSOLVE, Inc. 2007) for analysis. The APTs were analyzed using an assumed hydraulic conductivity (K) anisotropy ratio of 0.03. This value is consistent with the previous CAU-1LD analysis (Janzen et al. 2015) and is based on averages of similar field testing (USACE 2014) at the C-44 Reservoir/STA west of the CWFP. Displacement versus time was plotted for each test with associated derivatives. Due to noise in the derivative, the Boudet et al. (1989) curve-smoothing method was applied to each plot. This produces better diagnostic plots that, in turn, assist in determining the best analytical solutions to apply.

<u>Results</u>

Figure 5 shows the drawdown and recovery at M-1236 for the duration of the test. The temporary drop in pumping rate is evident approximately 300 to 600 seconds into the test. Because the initial APT at CAU-8LD did not allow sufficient time for recovery, the test was repeated on April 4, 2018. These results were used for analysis of this well. **Figure 6** shows the drawdown and recovery for the CAU-8LD repeat test. Pumping ceased and recovery was initiated 2,172 seconds into the test, as the well had run dry, and left overnight to return to background conditions.



Figure 5. Drawdown and recovery graph of the M-1236 test.



Figure 6. Drawdown and recovery graph of the CAU-8LD repeat test.

Analysis

Displacement versus time and the derivatives for the M-1236 and CAU-8LD APTs are shown in **Figure 7**. The shape of the M-1236 displacement versus time curve is consistent with that of an unconfined aquifer (Renard et al. 2009). Several analytical solutions were applied to the test data, including Theis (1935), Cooper and Jacob (1946), Neuman (1974), Moench (1997), and Tartakovsky and Neuman (2007). While no solution fit the data well for M-1236, Moench (1997) was selected as the best fit available. **Figure 8** shows the calculated aquifer parameters using this solution.

The CAU-8LD well had more than 30 ft of drawdown and took several hours to return to background conditions. Given the lower K of the sediments in this well, several analytical solutions for unconfined, leaky confined, and confined aquifers were applied to the test data, including Theis (1935), Cooper and Jacob (1946), Hantush (1960), Cooley and Case (1973), Neuman (1974), Moench (1997), and Tartakovsky and Neuman (2007). None of the solutions gave a fair fit for the data in drawdown or recovery. No solutions fit the data satisfactorily, so the recovery data were analyzed as a slug test using the Hvorslev (1951) and Bouwer and Rice (1976) solutions. There was very little difference in calculated parameters, so the Hvorslev (1951) results are reported for consistency with previous publications. **Figure 9** shows the result of this analysis and the calculated aquifer parameters.



Figure 7. Displacement (blue) versus time with derivative (red) for M-1236 (left) and CAU-8LD repeat test (right).



Figure 8. Analysis of M-1236 test data using the Moench (1997) solution.



Figure 9. Analysis of CAU-8LD repeat test data using the Hvorslev (1951) solution.

Discussion

Based on the aquifer test analysis, the K of the sediments in M-1236 is in the middle of the range for silty sands and fine sands (10^{-5} to 10^{-3} centimeters per second [cm/s]) at 2×10^{-4} cm/s (0.5 ft/day), as described by Fetter (2001). The screened interval at M-1236 consisted of shell beds and poorly indurated limestone, and most likely is representative of Unit 3. However, the low Kh result is inconsistent with lithology, and M-1236 is an older well and the condition of the well screen is unknown. Therefore, the results from M-1236 are not considered reliable and are not included in the hydrologic summary. CAU-8LD had a lengthier recovery interval and the K of this well is in the range for silt, sandy silts, and clayey sands (10^{-6} to 10^{-4} cm/s) at 4×10^{-5} cm/s (0.1 ft/day) as described by Fetter (2001). Based on lithology, the screened interval at CAU-8LD intercepts a clayey sand and is not considered representative of Unit 2; therefore, it is not included in this summary.

4.2.2 Previous Slug Tests

SFWMD staff conducted 11 slug tests on 5 newly installed wells within the Caulkins pilot project in 2014 and 2015. Four of the tests were in Unit 1 and seven were in Unit 2. Average Kh was 77 ft/day for Unit 1 and 23 ft/day for Unit 2. Slug tests provide reasonable order-of-magnitude estimates for K values (Thompson, 1987).

4.2.3 Previous Aquifer Performance Tests

The following published K values were reviewed:

- Six short-term APTs were conducted in 2014 and 2015 and are described further in Janzen et al. (2015, 2017). Those tests included two APTs at CAU-1S, screened in Unit 1, and four APTs at CAU-1M, CAU-1D, or CAU-1LD, screened within Unit 2. Average Kh was 8 ft/day for Unit 1 and 4 ft/day for Unit 2.
- The USACE (2014) conducted two APTs in the footprint of the planned C-44 Reservoir/STA. The wells, W-101 and W-102, were approximately 1 and 6 miles west of the CWFP, respectively. The wells were installed to a depth of approximately 135 ft bls, and each test included a pumping well and an observation well. The tests were run for a minimum of 24 hours. Based on lithologic descriptions, the test wells were screened within Unit 2. The tests yielded horizontal hydraulic conductivity (Kh) results of 28 and 20 ft/day and vertical hydraulic conductivity (Kv) of 0.35 and 1 ft/day, respectively. The USACE also conducted laboratory tests on the shallow sand layer (Unit 1) and derived an average Kv of 0.10 ft/day.
- The SFWMD conducted an APT in the footprint of Cell 3 as part of data acquisition for the Martin County SAS groundwater flow model (Lukasiewicz and Adams-Smith 1996). The pumped well was screened from 30 to 110 ft bls, within Unit 2. The test was run for 19 hours and included one observation well. The test yielded a Kh of 51 ft/day.

4.2.4 Summary of Hydraulic Conductivity Test Results

Average Kh for Unit 1, the upper clayey unit, ranged from 77 ft/day in slug tests to 8 ft/day in short-term APTs. Due to the high variability of clay content in the upper unit, this range of values is considered reasonable. Average Kh for Unit 2, the production unit, were 23 ft/day in slug tests, 12 ft/day in short-term APTs, and 33 ft/day in 24-hour APTs. The 24-hour APTs included longer screened intervals and a paired monitor well and are considered a more reliable testing method. Results for Kh are summarized in **Table 2**.

Local	Slug Test Average	Short-Term Aquifer Test	C-44 Aquifer Test	Caulk_APT Aquifer
Hydrogeologic	Results – (Number of	Average Results – (Number	Average Results –	Test Results /
Unit	Tests)	of Tests)	(Number of Tests)	(Number of Tests)
Unit 1	77 – (2)	8 - (2)	N/A	N/A
Unit 2	23 – (7)	12 - (4)	24 - (2)	51/1
Unit 3	N/A	N/A	N/A	N/A

Table 2.Average horizontal hydraulic conductivity (Kh) derived from aquifer performance tests at the
Caulkins Water Farm Project and C-44 Reservoir/Stormwater Treatment Area.

All values are presented in feet per day. N/A = pat applicable, pat tasts were super

 $N\!/A = not$ applicable, no tests were run.

4.3 Groundwater Flow and Seepage

Based on relative water levels during operation of the pilot project (Cell 1), surface water from the impoundment flowed downward into deeper portions of the SAS and outward into perimeter canals. The hydraulic gradient between the shallow and intermediate wells, compared to intermediate to deep and deep to lower-deep wells, suggests semi-confinement in Unit 1 consistent with the clay constituents observed. Downward gradients from the shallow to deep wells (CAU-1) suggest predominantly downward flow, at least to the screened interval of the deep well at approximately 72 ft bls. The lowest gradient was between the deep and lower-deep wells (screened approximately 135 ft bls), likely due to a change in flow direction from vertical to horizontal towards the C-44 Canal to the south. The underlying Hawthorn Group, a confining unit, acts as the lower boundary for vertical flow.

During the pilot project, a seepage rate of 0.092 ft/day was observed (Janzen et al. 2017). During the 2018 operational period of the expanded CWFP, a much lower seepage rate of 0.022 ft/day was observed, not including effects of ET and rain, which largely cancel each other out (B. Gunsalus, pers. comm.). To evaluate seepage from individual cells, flow between cells was restricted via gated culverts from November 20, 2018 through January 31, 2019. During this period, seepage from Cell 1 was approximately 0.05 ft/day, compared to seepages from Cells 2, 3, and 5 of 0.01, 0.01, and 0.02 ft/day, respectively, roughly 20 to 40 percent of Cell 1. Measurements for Cell 4 were not provided. Subsurface lithology indicates spatial discontinuity of low-permeability units within the SAS, with less permeable sediments inferred in Cells 3, 4, and 5, representing most of the northern and eastern portions of the CWFP.

5 SUMMARY

The CWFP encompasses a 3,014-acre surface water impoundment adjacent to the C-44 Canal (St. Lucie River) in southern Martin County, Florida. The impoundment includes five cells, separated by earthen berms, that range from approximately 414 to 798 acres in area. Approximately 20,192 ac-ft of water was pumped into the impoundment during the operational period of December 8, 2017 through November 2, 2018. Prior to construction of the CWFP, a pilot project comprising a 414-acre surface water impoundment in the southwestern portion of the existing CWFP (Cell 1) was operational from February 2014 through October 2016. Since 2014, a total of 19 groundwater monitor wells and 6 surface water stations have been installed within and adjacent to the CWFP to assess lithology and facilitate continuous water level monitoring and water quality sampling. Additional data sources reviewed for this report included lithologic logs and APT data obtained prior to construction of the CWFP.

Site hydrogeology was found to be consistent with that described in the Martin County SAS groundwater flow model and data report (Adams 1992, Lukasiewicz and Adams-Smith 1996). Regionally, the SAS is unconfined to semi-confined and composed of three hydrogeologic units: the shallow, unconsolidated sand/soil unit (Unit 1); more permeable sandy shell with interbedded sandstone, which together compose

the production unit (Unit 2); and the less permeable granular limestone (Unit 3), which inter-fingers with and underlies the production unit.

Within the CWFP footprint, the uppermost unit (Unit 1) was found to consist of predominantly silty sand with irregular interbeds of clayey sand grading to sandy clay and sandy, calcareous clay from the surface to approximately 18 ft bls. Discontinuous clayey sand or sandy clay layers ranging from 2 to 12 ft in thickness were observed in each of the soil borings within the CWFP footprint at various depths. Average Kh values from slug tests (77 ft/day) and short-term APTs (8 ft/day) were estimated at the CWFP. Laboratory test data at the C-44 Reservoir/STA to the west indicated an average Kv of 0.10 ft/day. Unit 2 composes the bulk of the SAS. At the CWFP, Unit 2 consists of predominantly poorly graded quartz sand, silty sand, shell, and a few interbeds of sandstone less than 2 ft thick. Locally interbedded units of clayey sand grading to sandy clay, and coquina (Unit 3), were found within Unit 2. Twenty-four-hour, multi-well APTs within the CWFP and the C-44 Reservoir/STA to the west indicated a range of Kh from 20 to 51 ft/day, and a range of Kv from 0.35 to 1 ft/day. Unit 3 is a less permeable, poorly consolidated granular limestone and coquina with sand, shell and calcareous clay that occurs at the base of the SAS and also interbedded at relatively shallow depths within Unit 2.

Site hydrogeology is spatially variable, with lower-permeability sediments inferred in the eastern half (Cells 4 and 5) and in the northwestern cell (Cell 3), relative to the pilot project cell (Cell 1). Low-permeability sediments include thicker and potentially more continuous clayey sediments in the shallow sand layer, a poorly consolidated coquina from approximately 15 to 38 ft bls, and clayey sand grading to sandy clay from approximately 59 to 81 ft bls. Well control in Cell 2 was not deep enough to evaluate below approximately 40 ft bls. The lower permeability sediments in Cells 3, 4, and 5 may be responsible for the lower seepage observed in the operational period for the expanded CWFP (approximately 0.022 ft/day, including ET) compared to approximately 0.051 ft/day during the pilot project operational period. During a short test period in January 2019, cells were isolated and seepage from Cells 3, 4, and 5 was approximately 20 to 40 percent of the seepage observed from Cell 1, consistent with the low-permeability sediments observed beneath the cells.

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APPENDIX A: LITHOLOGIC DESCRIPTIONS

	Date: 21	5/15	Page	of_ <u>4'</u>		Mud Weights and Viscosity Drilling Parameters							
	¥		A. A.			Time	Weight	Viscosity	Time	Wt on Bit	Discharge		
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		26-28	16/18/26/2	7 20	p fine	to meel	shell s	l; 5%	Anes,	X57. ph	osphate		
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Date: 2/4/15 Page 4 of 4					Mud Weig	ghts and \	/iscosity	Drilling Parameters			
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	SFWMD Daily Drilling Form														
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	SFWMD Daily Drilling Form												
	Date: 2/5/1 B Page 2 of 4 Mud Weights and Viscosity Drilling Parameters												
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Date: 2/12/18 Page 1 of 4 Mud Weights and Viscosity Drilling Parameters														
Well Nan	ne CAL	1-9110)		Time	Weight (lbs/gal)	Viscosity (sec/qt)	Time	Wt on Bit (1,000 lbs)	Discharge (gpm)				
Contracto	or/Driller 1/	nal Pino	- Depa	od										
Site Geol	ogist	LAE CA	128											
Ending D	epth /	126				{								
Drilling N	otes: bit siz	e/fluid additiv	/es/mud type,	etc.				<i>a</i>	/					
		2"5	dis good	n is	acm	loz st.	eve to	Morel	h1					
	Benny with YUIT. Described by T. Tar zea													
	He	support 1	ny J. Ja	n ze n	<u> </u>									
	Depth	Sampling		ana ana amin'ny saratra amin'ny saratra amin'ny saratra amin'ny saratra amin'ny saratra amin'ny saratra amin'n		Litho	ology and	Formation	Notes					
Time	(5 ft int.)	Time		-	(lost	t circulatio	n zones, o	chatter, rate	e changes, etc	.)				
計曲	2-4	e -	Cost-1grosted	Sand	gerils	horeourg	12, v.+	The TO ;	+reg12	- 7 CMC/				
17/10	4-6		da											
3/1/16	6-8		ofa											
149/1 17/a/11	B-10		o/a											
1 lar	10-12		Shell for	og ner	13-1	Are sa	ndra;	9 band	3.2el.					
the la	12-14		e/a	<i>Q</i> ,	1	(1) DE ES	<u> </u>							
Q. 1	N-11		NO SI	7mol	e									
10gal	High 1 19-16 Shell Frogments and giz sand, brownsh grey, the to													
12726	11-10		Ce	<u> </u>	3 Barry	red.								
1/20/33	18-20		ga											
(R)	20-22		Ho Jany	<u>ne</u>	1	es Lun	o The Hard	» 672 X	mel. Acl	Ana menth.				
1/25/10	22-24		65-71 184 J.	and	tragy,	Sone	. Sand	3 TERC	<u> </u>	The Advances and				
132	24-26		51/47 50h	Onto	ex, i 7	Ave to 1 <u>e</u>	Hive g	72 SCM	S, Jose 30	Bell Hadgesson,				
10/21	26-28		Pan 1- 91	served	Barco	l - Mee r <i>aa</i> eee	lin (7 79. L	5%	he pred	sum 91 E SANCL				
12/33/23	28-30		0/4		. <u> <u>(</u> (</u>	1		- / T	/					
Intertan	30-32		a/a		_					-				
anghalus	32-34	//	ala				nearitle	(Dong	e	A				
2 /2/2/00	34-36		Paraly 90	acked	Sad	NE U. P.N.	e p A	ives 72	Sarey to	en stul thegang				
178/25	36.30	//	a	<u> </u>	1									
MAK	1.010		1	 П	epth to W	ater	Water	· Qualitv Me	asurements					
T:	Depth	New Rod	Dev.	(re	verse air	only)		(reverse ai	r only)	Laboratory				
Time	Time (feet) (feet) (deg) Ref Drill Pipe Annulus pH Sp. Cond. Temp. Sample ID													
L		+					-		, (
		†	1											
	1						8	1						

	SFWMD Daily Drilling Form													
	Date:	2/18	Page 📈	of <u> </u>		Mud We	ights and V	/iscosity		Drilling	Parameters			
	3/ Well Nan	13/18 ne	41)-911	2		Time	Weight (lbs/gal)	Viscosity (sec/qt)	Time	Wt on Bit (1,000 lbs)	Discharge (gpm)			
	Contracto	or/Driller	Malpn-	Tennod										
	Site Geol	logist	47 Ged	ideg						<u> </u>				
	Starting L	Jepth Jepth	$\frac{0}{1}$											
	Drilling N	otes: bit siz	ze/fluid additiv	ves/mud tvpe	. etc.									
	Alpwar page													
	Time	Depth (5 ft int.)	Sampling Time	ning and agent and an and an		(lost	Litho circulatio	ology and F on zones, c	Formation hatter, rate	Notes changes, etc	·.)			
7/X61	1051 Circulation Zones, Charlet, rate changes, etc., 1051 Circulation Zones, Charlet, rate changes, etc.,													
12/18)	26/31 40-42 Poraby graded sand - ofa													
19/26	131/45	42.44		ala			····			+ + + + + + + + + + + + + + + +				
15/20	/ 10/12	14.46	/	e/a		<u></u>	<u> </u>	rish ca	anse_	1)	all Thank row 2 2			
15/24/	20/18	16.4B		Posely 9	<u>nodece</u> 57.p	<u>l senc</u> Us spha	l V. Are <u>Rej</u>	Te fre	972.94	Net owner to	a ray and i			
u/13	111/8	48.50	/	0/a 6:14. 500	-l-la	ADD.C	N MA	entr	10.072	Sand unt	nedsesso Jo			
11/12/	11/12	50.72	4	THE JEW	Exercit 5	17eel y	leer A.	as sont	- 5 (5)	A plogste	we			
15/2	136/48	52. SY		<u> </u>), .						
6/2	120/32	54.56	/	0/a			y Grand			1	Schell			
10/20	20190	Z·JB		Pointy C	hodee	dim h	pedan <u>2 G have</u>	r 1797, L 5. 302	l slee	l troga	ents, 152 Aughter			
14/2	10-18	5e-60		<u> 1</u> a										
12/6	<u>/u/9</u>	50-02	`_/	a/a Shuar s	and	Tichurb	eronate la	CN 6	auls 7	el stall A	1-5- 100 (F =)			
3/5/	4/5	52-6Y		Sal. C	The in	Aven	Aine g	12-501	<u>D.</u> TS1480	Araconts				
4/4	5/4	64-66	4	Ala	the col	NEGAL NOSTO, A	well pl	gre sor	, calca	neors 5				
1/1/ , I	4/4	<u>K. 68</u>		Gland		VOINE				,				
U = Ala	1012	10 70		ya Sandi 100	Ruth	doye	1 Sand	? obiegy	1, fireg	tz-1shel	" scorel & Pore , n			
110	<u>10/15 -</u>	10.72	1	<u> </u>	seme	ng u	ich plo	splane.	; and c	lave the	etz sand			
ษโ	19/8	12-74		7117 Jen. 517	tero	Cgj l	r Hine" Horac	10 Five	r 1 Z 50 <u>Cal æv</u> Ouglity Mar	eec) (y Novel			
·	Time	Depth (feet)	New Rod Meas.	Dev. Survey (deg)	(re	epin to wa everse air c	only)	(reverse air	only)	Laboratory Sample ID			
				(uey)	Ket			р п	(µS/cm)	(deg C)				
		<u> </u>							<u> </u>					
			+					<u> </u>	+	\				

SFWMD Daily Drilling Form													
Date: 7	113/18	Page 🍏	of <u>4</u>		Mud Weights and Viscosity					rilling Parameters			
Well Na	me (/	40-91D			Time	Weight (lbs/gal)	Viscosity (sec/at)	Time	Wt on Bit (1,000 lbs)	Discharge (gpm)			
Contrac	tor/Driller	PALLANO	-JeMou	ĺ –			(
Site Geo	ologist	Int. Ged	lde's										
Starting	Depth	0											
Ending I	Depth	126	1227242*********************************										
Drilling I	Notes: bit si	ze/fluid addit	ives/mud type	e, etc.									
		12 0	hee.1										
		· · · · · · · · · · · · · · · · · · ·											
Time	Depth	Sampling		and a group of the second	······	Litho	logy and F	ormation	Notes	<u>anang muju muju kontri muju ni </u>			
	(5 ft Int.)	lime	Glass de		(lost	circulatio	n zoneș, cl	hatter, rate	changes, etc.) GALLOULUZON			
10/2	PY-76		Thele The	Herr	NO WIJU mars c	uth S.	itry 3 1	17 11 1000 1 <u>2-5 a</u> l	Sugrey,	97aul sites			
610/10	16-78		Sarl 5 to	ne, s	hel her	19.0053	silry 5	and, 1	Thrownsh send stee	Harnes +			
aluto	10 80		Sheer Anoy	newys c	175 51	In sale	4 17 hrs	MJ719	norl 1.7el	sleel trojners			
1/11/10 -	APP-UU		Siln Son	n <u>sil</u> d lidz	MATZ	Sand	lipe my	line sh	with fire s	id to spoul			
1 <u>10/11 1</u>	<u>v-02</u>	5 with	ABAUL 4	Zal	steer An	esnent							
19/8 9	p- 194		TOMIN 9 M	dul y	and - Insta	Edim 179	Y, fine	N 11 color	-972-9-27 9	; Leel They saw ,			
In the 9	4.06		da	/	1. 1.								
	01 84		Sand 3 ron	e-m	eclium ga	e-1, fine	10 redus	n sdr s	ul, cala	arears covert			
17 <u>12/14</u> [<u>р= 10</u>	2	al				4		<u>***</u>				
1/15 2	<u>p-90</u>		- Ta	- M	ithgrow	rel		to the	Company Co	D+c(apl			
3/20/7-9	10-12	4	<u>Silty 300</u>	thorn	which has has	A GRO	rl size	l sleel	Hagans	3, 65%ph.			
30/32/31	\$2. 9Y		Silty Sand	l ghel	Sandste	ne-y-	ectourly	7, as -	show with	calconeors			
9610	Qu. ac		Silty Sonte	1. y e	lourgh	94, 5-	Five 12	sect in	mqt z sa	ndt slell			
101/5	gr 40		Silly Sare	few with	frand.	4. Zel Vellou	<u>sial 74</u> 13494 ·	V. tine	3 14 6 5 16/ e to sedim	stzselt			
/////3	10.10	1		at us	h gran	1 31200	e she	1 Anaj	rong L	5 7. pho plate			
10/12	<u>18 - 10</u>		a/a										
10/11	a.102		2: 1+ yand	with co	d sone	ellowith pts	9/241V	the pro	ralan stz	sault stell			
10/7	102-IN		ala					/*/		<u></u>			
tin lu la	101		Silly Son	du g	Dove	fullous	hy, u	Anete	medin 1	TZ SelT			
	109-100		fil ha	that we	The gra	- al viza	1953	flul ;	tillagrand 3,	6510 ph.			
15/0/15	4p2-10B	ļ /	<u> </u>	- 40. 101	inavir -	ya:							
PINIC	108-110	1	a/a										
,.	Denth	New Rod	Dev.	D (re	epth to Wa everse air o	iter nlv)	Water C (r	Quality Mea everse air (surements	Laboratory			
Time	(feet)	Meas.	Survey			1		Sp. Cond	Temn	Sample ID			
		(teet)	(deg)	Ref	Drill Pipe	Annulus	рН	(µS/cm)	(deg C)	1			
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			+		<u> </u>	<u> </u>							
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					3	FWNDE	aily Dril	lling Fori	n		
	Date: 2	13/18	Page Y	of Y		Mud We	ights and \	/iscosity		Drilling I	Parameters
	Well Nan	ne <i>(</i> An	1-960			Time	Weight	Viscosity (sec/at)	Time	Wt on Bit (1,000 lbs)	Discharge (gpm)
	Contracto	or/Driller 1/	TAILONO	- Trenton	đ		(ibb/gul)	(300,41)		(1,000.20)	
	Site Geo	logist /	it fred	des						<u>├</u> ────┤─	
	Starting I	Depth (n	<u> </u>						{ <u> </u> -	
	Ending D	epth 7	26								
	Drillina N	otes: bit si	ze/fluid additi	ves/mud tvpe	e. etc.					<u></u>	
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			<i>\[1 \ f \ d</i>	ge J.							
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	Time	Depth	Sampling				Litho	ology and F	formation l	Notes	、 、
		<u>(5 ît înt.)</u>	lime	5.4.5.0			circulatio	n zones, c	hatter, rate	changes, etc	.)
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ŀ	holioli3	117-114		01a '							
Ċ		10-11	4	Paraly & An	Ned Sr.	nel	Que Lit	SV 11 A	linen.	nech 177 5	distell,
9	13/01/39	114-116			151	abra al	z te	171 - 1		1.0.	. /
	4.4/1		<u>د</u>	Poort.	sol in a l	and int	2 Gran	el-nerle	ml+N	, ale in-	6
- 16	16/37/42	116-118		the second	care)	R. S. Zel	5/10	Anna	man y	1 m/a m/	2
	1.1.1.	11 10 10 10		Pasty 5A	-del	Sand.	sedin	ltsv. H	fine ro.	red pt 2 T	shelsand,
9	696	118-120				Fer	of Noral	fleer 7	Nogrong	65% Al	us, late
14	1.1.10			Sily Se	L-Ye	llash	44.W	ApoAn	TES	c, ten sl	el franos,
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			New Rod	Dev	D ,	epth to Wa	ater	Water (Quality Mea	surements	
	Time	Depth	Meas.	Survey	(re	everse air c	oniy)	(reverse air	oniy)	Laboratory
		(teet)	(feet)	(deg)	Ref	Drill Pipe	Annulus	pH.	Sp. Cond.	. Temp.	Sample ID
	L	<u> </u>	l						(µS/cm)	(deg C)	
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APPENDIX B: GEOPHYSICAL LOGS



	LOG CODES												
3-arm caliper	CAL	long normal resistivity	RLN	deep induction conductivity	IDC								
natural gamma (CPS)	GAMM	8 inch resistivity	R8	shallow induction conductivity	ISC								
spontaneous potential	ESP	32 inch resistivity	R32	sonic interval velocity	DT								
single point resistance	RES	deep induction resistivity	ILD	sonic porosity (RHG method)	SPHI								
short normal resistivity	RSN	shallow induction resistivity	ILM	repeat designation	R								



CAU-8LE)	LOG CO	DES		
3-arm caliper	CAL	long normal resistivity	RLN	deep induction conductivity	IDC
natural gamma (CPS)	GAMM	8 inch resistivity	R8	shallow induction conductivity	ISC
spontaneous potential	ESP	32 inch resistivity	R32	sonic interval velocity	DT
single point resistance	RES	deep induction resistivity	ILD	sonic porosity (RHG method)	SPHI
short normal resistivity	RSN	shallow induction resistivity	ILM	R	
GAMM	Denth	COMMENT	•	II.D.	
0 CPS	200 1in:20ft	0		OHMM	50
				ILM	
		0		ОНММ	50
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	LOG CODES												
3-arm caliper	CAL	long normal resistivity	RLN	deep induction conductivity	IDC								
natural gamma (CPS)	GAMM	8 inch resistivity	R8	shallow induction conductivity	ISC								
spontaneous potential	ESP	32 inch resistivity	R32	sonic interval velocity	DT								
single point resistance	RES	deep induction resistivity	ILD	sonic porosity (RHG method)	SPHI								
short normal resistivity	RSN	shallow induction resistivity	ILM	repeat designation	R								



M-1236		LOG CODES										
3-arm caliper	CAL	long normal resistivity	RLN	deep induction conductivity	IDC							
natural gamma (CPS)	GAMM	8 inch resistivity	R8	shallow induction conductivity	ISC							
spontaneous potential	ESP	32 inch resistivity	R32	sonic interval velocity	DT							
single point resistance	RES	deep induction resistivity	ILD	sonic porosity (RHG method)	SPHI							
short normal resistivity	RSN	shallow induction resistivity	ILM	repeat designation	R							

