



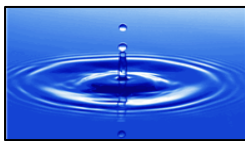
Updated STA Phosphorus Modeling For the 2010 Planning Period

Work Order No. CN040902-WO03R2

Prepared for



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FINAL REPORT

October 2007

October 30, 2007

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U.S. Army Corps of Engineers
South Florida Water Management District
Tetra Tech Contract No. CN040902-WO03R2
Updated STA Phosphorus Modeling for the 2010 Period

Dear Ms. Gracie:

I am pleased to submit this final report titled "Updated STA Phosphorus Modeling for the 2010 Planning Period". This document constitutes Deliverable 7.5.2 under Tetra Tech EC Purchase Order 1020342 dated June 25, 2007.

I gratefully acknowledge the valuable contributions of the staff of the South Florida Water Management District, and the technical review by yourself, staff of the District and of the U. S. Army Corps of Engineers, in the development of the information contained in this report.

Certification

I hereby certify, as a Professional Engineer in the State of Florida, that the information in this document was assembled under my direct personal charge. This report is not intended or represented to be suitable for reuse without specific verification or adaptation by the Engineer. This certification is made in accordance with the provisions of the Laws and Rules of the Florida Board of Professional Engineers under Chapter 61G15-29, Florida Administrative Code.



Gary F. Goforth, P.E. Florida P.E. # 35525

Date: 10/30/07

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1. Introduction

As part of the adaptive implementation strategy of the *Everglades Protection Area Tributary Basins Long-Term Plan for Achieving Water Quality Goals (LTP)*, the analyses presented in the *Baseline Data for the Basin-Specific Feasibility Studies to Achieve the Long-Term Water Quality Goals for the Everglades* (Goforth and Piccone 2001) are to be regularly updated to improve the degree of confidence in the projected total phosphorus loads in inflows to the STAs, or in some instances, discharged directly to the Everglades Protection Area (Burns & McDonnell, 2003 as amended). A previous report updated the basin data sets from Water Year (WY) 1995 through WY2007, covering the period May 1, 1994 through April 30, 2007 (Goforth 2007a). Using the flow and phosphorus data developed in that effort, a subsequent report updated the STA inflow data sets for regional conditions anticipated for the 2010 planning period (Goforth 2007b). This report presents the results of updated STA performance projections based on phosphorus removal modeling using the DMSTA model (Walker and Kadlec 2005).

1.1. Scope of Work

This work constitutes Task 7 of CN040902-WO03.Ta18 - Preparation of an Environmental Impact Statement for Everglades Agricultural Area Conveyance and Regional Treatment Project Plus Compartments B and C - between the District and Tetra Tech EC, Inc. This work is being performed under Purchase Order No. 1030342, which was issued on June 25, 2007, between Tetra Tech EC, Inc., and Gary Goforth, Inc.

The scope of work for Task 7 consists of three major elements:

1. Update Flow and Phosphorus Data Sets for ECP Basins
2. Update STA Inflow Data Sets
3. Conduct DMSTA Modeling

This report presents the results of modeling the phosphorus performance of the STAs, the EAA Storage Reservoir and the treatment areas on Compartments B and C using the updated STA inflow data sets for the 2010 planning period.

1.2. Regional Conditions for the 2010 Planning Period

The previous update of the STA inflow data sets was completed in 2005 as part of the *EAA Regional Feasibility Study* (ADA/Burns & McDonnell 2005). That Study evaluated the regional water management conditions for two time periods, 2006-2009 and 2010-2014. This present analysis focuses on the regional conditions that are anticipated to be present in the 2010 time frame. The anticipated status of the water resources projects within the basins tributary to the STAs (shown in **Figure 1-1**) is provided in the **Table 1-1**. Appendix A contains a more complete summary of the key modeling assumptions used in this simulation throughout the South Florida area.



Figure 1-1: Overview of EAA And Surrounding Basins.

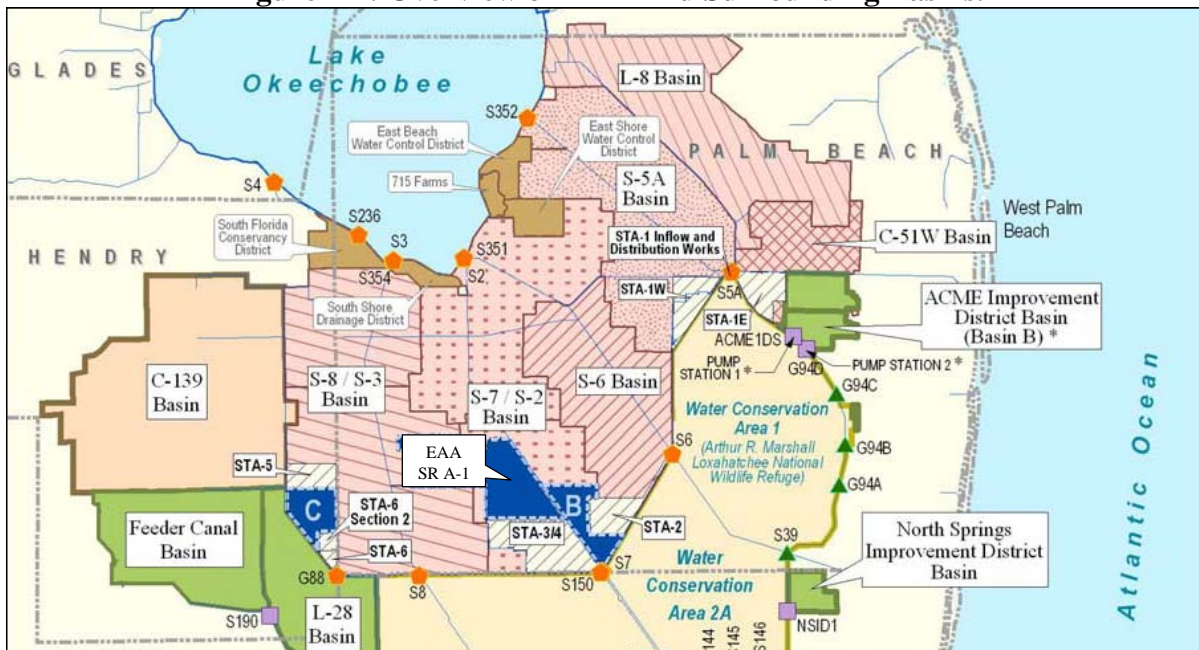


Table 1-1: Anticipated Status of Regional Water Resource Projects in the 2010 Period.

| Project | Status During the 2010 Period |
|--|--|
| Original Everglades Construction Project | All 6 STAs fully operational. Approximately 20% of the S-5A Basin runoff diverted to the Hillsboro Canal through existing facilities. Ch. 298 District and 715 Farms diversions in place. No EAA runoff reduction adjustment necessary to account for Best Management Practices. |
| Compartment B | Build-out completed and flow-capable by December 2010, including ability to re-direct STA-2 inflow to the North Build-out area. |
| Compartment C | Build-out completed and flow-capable by December 2010 |
| EAA Storage Reservoir A-1 | 16,000-acre reservoir operable with a 12-ft depth |
| Acme Basin B | Runoff directed away from WCA-1 and discharged to C-51W, and then to STA-1E |
| L-8 Reservoir | Partially completed: 870 acres, depth 44 ft. Facilities not completed for diversion away from S-5A/C-51W. |
| Everglades Agricultural Area Conveyance and Regional Treatment Project (ECART) | Not completed |



1.3. Phosphorus Modeling

The phosphorus removal performance of the STAs, EAA Storage Reservoir and treatment areas of Compartments B and C were estimated using the July 5, 2007 release of the Dynamic Model for Stormwater Treatment Areas, Model Version 2 (DMSTA, 09/30/2005), developed for the U.S. Department of the Interior and the U.S. Army Corps of Engineers by W. Walker and R. Kadlec. Outflow phosphorus concentrations are calculated based on:

- daily input data, consisting of flow, phosphorus concentrations, rainfall evapotranspiration (ET), depth (optional) and releases (optional);
- mass and water balances calculated for each time step for each treatment cell or reservoir compartment,
- treatment area configuration, cell size, flow path width, vegetation type, estimates of hydraulic mixing, outflow hydraulics, seepage estimates;
- phosphorus removal rates that can be either user-defined or available within DMSTA based on calibration data sets extracted from numerous vegetation types, phosphorus characteristics and hydraulic regimes of many south Florida wetland treatment systems through early 2005.

DMSTA was used to predict annual and long-term average flow-weighted mean concentrations, with a 365-day averaging period. In addition, STA performance uncertainty analyses were conducted, using the 10%, mean and 90% values of the settling rates for the specific vegetation types. These projections are subject to the assumptions, constraints and limitations of DMSTA modeling and STA operations, including the following.

- DMSTA calibrations are based upon data from fully functional treatment cells with viable vegetation communities that have near optimal performance. The range of treatment characteristics for each vegetation type is summarized in **Table 1-2**.
- In addition to consideration of the range of calibration treatment characteristics, other important factors not yet incorporated into the model include calcium requirements, antecedent soils, and assumed intensive management, particularly for the enhanced vegetation types.
- DMSTA generates error/warning notices if simulated conditions exceed the range of the calibration characteristics presented in **Table 1-3**.
- The use of the DMSTA calibration vegetation types, e.g., SAV, assumes that the vegetation will be maintained in the long-term. This assumption may produce overly optimistic long-term performance projections for treatment areas subject to periodic disturbance such as hurricanes, droughts and other extreme conditions.
- DMSTA does not allow a treatment cell to dry out, and hence does not reproduce the vegetative responses and phosphorus dynamics (e.g., post-dry-out spikes) observed in treatment cells that periodically go dry. Hence the phosphorus removal performance simulated for large wetland systems with limited water availability, such as Compartment C, may be overly optimistic. Other methods should be used to estimate



- the supplemental water required to either avoid dryout or to estimate the phosphorus performance for these large systems that experience periodic dryout.
- STA performance projections are subject to the complete set of DMSTA assumptions, which can be found at <http://www.wwwalker.net/DMSTA/index.htm>.
 - Additional uncertainty exists in flow estimates and regional water management.

Table 1-2: Calibration Dataset Ranges (Draft, from www.wwwalker.net/dmsta/).

| Variable | Units | EMERG | | PEW | | SAV | | PSTA | | RESERV | |
|------------------|----------|-------|------|------|------|------|------|------|------|--------|-------|
| | | Min | Max | Min | Max | Min | Max | Min | Max | Min | Max |
| Calib Period | days | 641 | 4017 | 693 | 4017 | 693 | 1522 | 245 | 1062 | 1460 | 5843 |
| Calib K | m/yr | 12 | 23 | 27 | 49 | 46 | 64 | 18 | 34 | 1 | 8 |
| Mean Depth | cm | 35 | 76 | 38 | 66 | 62 | 87 | 13 | 52 | 90 | 304 |
| Max Depth | cm | 47 | 131 | 71 | 123 | 75 | 132 | 22 | 65 | 187 | 457 |
| Freq Z < 10 cm | % | 0% | 9% | 0% | 13% | 0% | 0% | 0% | 38% | 0% | 0% |
| Hydraulic Load | cm/day | 1.1 | 6.5 | 3.0 | 6.9 | 5.1 | 12.7 | 2.8 | 14.6 | 0.4 | 17.6 |
| Residence Time | days | 8 | 66 | 7 | 22 | 5 | 17 | 1 | 19 | 8 | 714 |
| Velocity | cm/sec | 0.04 | 0.45 | 0.16 | 0.48 | 0.30 | 0.64 | 0.01 | 1.12 | 0.05 | 1.32 |
| Flow/Width | md/2 | 26 | 210 | 69 | 276 | 162 | 374 | 3 | 132 | 68 | 1135 |
| Areal Load | mg/m2-yr | 382 | 2908 | 222 | 1919 | 1649 | 5279 | 142 | 1447 | 212 | 11781 |
| Storage | mg/m2 | 921 | 4299 | 171 | 1494 | 903 | 2959 | 96 | 911 | 200 | 4994 |
| Inflow Conc | ppb | 39 | 283 | 17 | 110 | 36 | 153 | 7 | 56 | 78 | 1144 |
| FWM Outflow Conc | ppb | 20 | 150 | 8 | 28 | 15 | 57 | 6 | 15 | 50 | 767 |
| Outflow GeoMean | ppb | 19 | 125 | 8 | 21 | 15 | 55 | 5 | 15 | 39 | 725 |
| Marsh GeoMean | ppb | 19 | 128 | 8 | 20 | 16 | 56 | 5 | 15 | 38 | 725 |

Table 1-3: Variable Ranges for Model Applications - Used to Trigger Warning Messages (Draft, from www.wwwalker.net/dmsta/).

| Calibration | K (m/yr) | CV(K) | Depth (cm) | | Q/W (m2/d) | | Conc (ppb) | | Freq Z < 10 cm (%) | |
|----------------|----------|-------|------------|-----|------------|------|------------|------|--------------------|-----|
| | | | Min | Max | Min | Max | Min | Max | Min | Max |
| EMERG_3 | 16.8 | 0.20 | 35 | 76 | 26 | 210 | 19.5 | 800 | 0% | 9% |
| PEW_3 | 34.9 | 0.21 | 38 | 66 | 69 | 276 | 8.0 | 110 | 0% | 13% |
| SAV_3 | 52.5 | 0.16 | 62 | 87 | 162 | 374 | 14.9 | 153 | 0% | 0% |
| PSTA_3 | 23.6 | 0.22 | 13 | 60 | 3 | 132 | 5.9 | 56 | 0% | 38% |
| RES_3 | 5.0 | 0.45 | 90 | 304 | 68 | 1135 | 50.3 | 1144 | 0% | 0% |

When evaluating DMSTA results, particular attention needs to be given to the simulated outflow concentration, in that DMSTA does not constrain the reported values to minimum levels observed in the calibration data sets reported in Table 1-2. In other words, the model may forecast outflow concentrations lower than have been observed in the field. Forecast error is inherent when using any simulation model. These errors result from limitations of the calibration datasets (measurement error, short duration, etc.) and other sources that are difficult to quantify. Based on information from the DMSTA website (<http://www.wwwalker.net/DMSTA/index.htm>) and Walker (personal communication), the DMSTA forecast error for flow-weighted mean concentrations is approximately +/-23% of the expected value. In addition, the following disclaimer is offered by the authors of DMSTA:

DMSTA2 is a modeling tool with a constrained range of applicability. It has been developed and calibrated to information specific to South Florida. It is intended for use in evaluating Everglades Protection Project by individuals with experience in hydrologic & water quality



modeling. It should not be exercised in any situation without careful examination of all features, assumptions and calibrations, as they relate to a given application and to the supporting research upon which the calibrations are based. When properly calibrated by the user, the hydraulics portion of DMSTA2 is thought to generate predictions that are adequate for the purpose of simulating phosphorus dynamics. The hydraulic simulations should not be relied upon for designing flood control measures, designing levees, for any other purposes in which life and/or property may be at risk. The user assumes all risks associated with using the model for designing treatment areas or any other purpose.

Proper use of DMSTA2 requires thorough understanding of calibration results & limitations & further documentation provided below. Sample input files are for demonstration purposes. None reflect actual designs. Atmospheric deposition, hydraulic, or seepage input values should not be interpreted as defaults or recommended values. While P cycling parameters are suggested for various situations and within well-defined calibration boundaries, users must decide which calibration is appropriate in any situation.

Additional information on the development, calibration and application of DMSTA can be found at: www.wwwalker.net/dmsta.

The development of the daily flow and phosphorus input data sets was described in *Updated STA Inflow Data Sets for the 2010 Period* (Goforth 2007b). Daily rainfall and ET for all the treatment areas except STA-5 and STA-6 were provided by the District as part of the SFWMM modeling. For STA-5 and STA-6, actual rainfall and ET were used based on local gauges. Treatment cell dimensions, hydraulic characteristics and vegetation types were consistent with values used in the 2005 EAA Regional Feasibility Study (ADA/Burns & McDonnell 2005), modified for consistency with updated information obtained from the ongoing Compartments B and C design (Brown & Caldwell 2007, URS 2007). All STA enhancements described in the Everglades *Long-Term Plan* scheduled for completion by the end of 2010 are assumed to be completed (Burns & McDonnell 2003, as amended).

1.4. Revised C-139 Basin Data

Subsequent to the transmittal of the final report *Updated STA Inflow Data Sets for the 2010 Period*, a discrepancy was identified between the flows and loads developed for the C-139 Basin and the data set utilized for the C-139 Basin BMP regulatory program. A review of the method used to develop the regulatory program data set revealed that certain flows and loads from the District's DBHYDRO database were revised to better represent the runoff from the C-139 Basin (Walker 2000). The net effect of this revision was to reduce the annual flow and phosphorus loads from the C-139 Basin to be treated in STA-5 and STA-6 by approximately 5,500 AF/yr and 3,000 kg/yr. For consistency with the regulatory program, the data set and STA inflow data used in this work effort for the C-139 Basin were revised. The revised tables and figures for the C-139 Basin and the inflow data sets for STA-5 and STA-6 are presented in Appendix B, along with the revised summary tables.



Table 2-1: Summary of Long-term Average Annual Inflow to STA-1E.

| Source | Flow (AF/yr) | TP Load (kg/yr) | TP Conc (ppb) |
|--------------|----------------|-----------------|---------------|
| S-5A Basin | 18,766 | 3,802 | 164 |
| EBWCD | 1,028 | 593 | 468 |
| L-8 Basin | 8,571 | 1,019 | 96 |
| Acme Basin B | 35,066 | 4,915 | 114 |
| C-51W Basin | 130,375 | 31,529 | 196 |
| Total | 193,818 | 41,864 | 175 |

Prior to construction, the existing ground elevation at STA-1E exhibited a slope from the northeast to the southwest of more than 7 feet. When constructed by the U. S. Army Corps of Engineers, the majority of the treatment cells were leveled to minimize hydraulic short-circuiting and areas of deep depths. However, the East and West Distribution Cells were not leveled and still retain the relatively steep slope that existing prior to the STA construction. As a result, the cells are characterized by areas of high ground without wetland vegetation, areas of deep ponds, and an irregular inundation/dry out cycle. Unlike the Buffer Cell of the prototype STA (the Everglades Nutrient Removal Project), phosphorus removal within the East and West Distribution Cells is not anticipated to be reliable. Hence, these cells are not considered as part of the effective treatment area of STA-1E, and were modeled with an effective settling rate of 0.01 m/yr.

A summary of STA-1E phosphorus performance for the 2010 period is presented in the table and figures below. A copy of the DMSTA output is presented in Appendix C. A long-term flow-weighted mean outflow concentration range of 20-35 ppb was forecast, however this includes years when DMSTA forecast levels that fell below the calibration range of 15 ppb for an SAV system, and hence may be optimistic. DMSTA generates various warning and error messages based on the simulation results compared to the calibration data sets; these are displayed in the DMSTA results in Appendix C. For Cells 2, 4S and 4S, the mean depths were slightly lower than the SAV calibration range. Also for Cell 2, the mean flow/width was slightly lower than the range in the SAV calibration data sets.



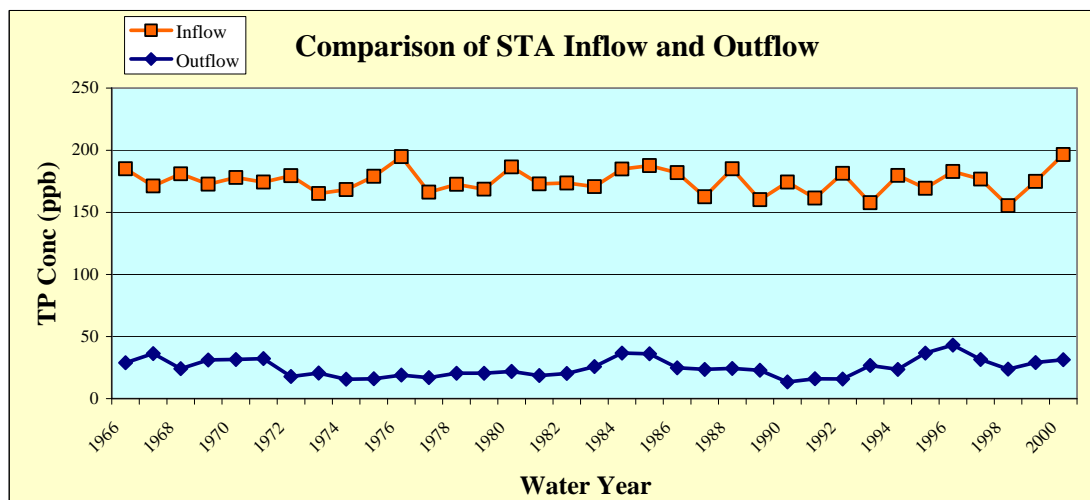
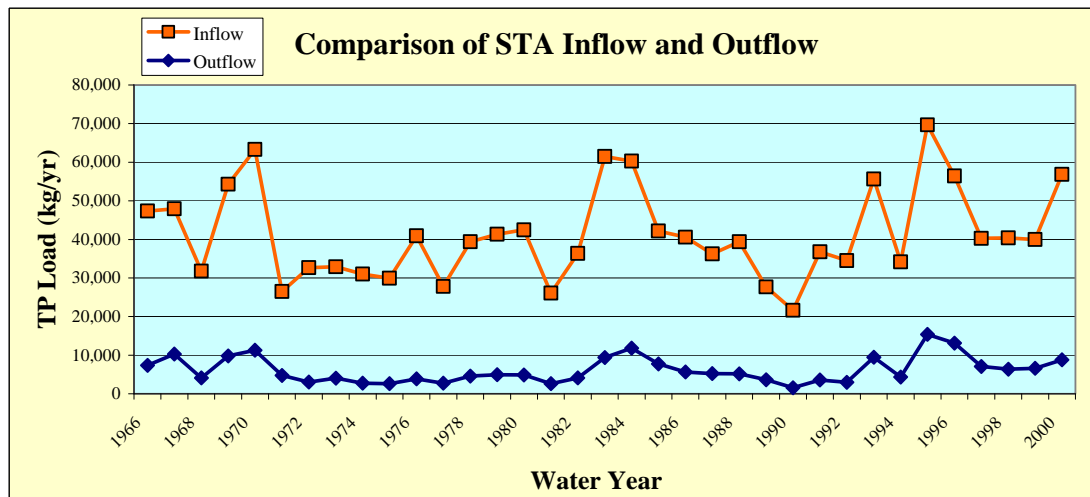
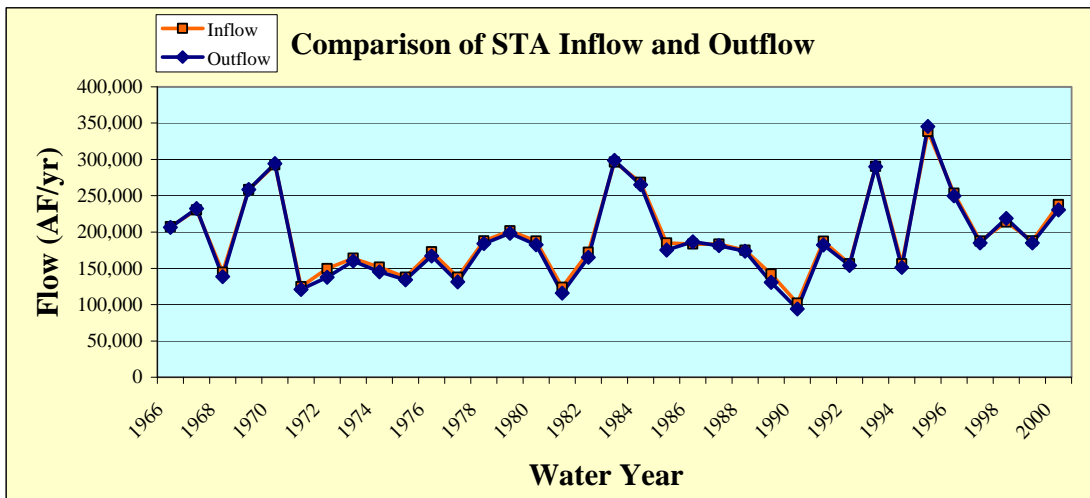
Table 2-2: Summary of DMSTA Results for STA-1E.

| Parameter | Unit | STA-1E |
|--|-------|---------|
| Effective Treatment Area | acres | 5,132 |
| Average Annual Inflow | | |
| Volume | AF/yr | 193,818 |
| TP Load | kg/yr | 41,864 |
| TP Concentration | ppb | 175 |
| Average Annual Outflow | | |
| Volume | AF/yr | 190,599 |
| Flow-weighted Mean TP Concentration | | |
| Upper Conf. Limit for Settling Rate | ppb | 20 (10) |
| Mean Estimate of Settling Rate | ppb | 27 (1) |
| Lower Conf. Limit for Settling Rate | ppb | 35 |
| Geometric Mean TP Concentration | | |
| Upper Conf. Limit for Settling Rate | ppb | 18 |
| Mean Estimate of Settling Rate | ppb | 24 |
| Lower Conf. Limit for Settling Rate | ppb | 32 |
| TP Load (Using Mean TP Conc.) | kg/yr | 6,240 |
| Diversion Volumes and Loads | | |
| Volume | AF/yr | 26,186 |
| TP Load | kg/yr | 4,027 |
| TP Concentration | ppb | 125 |

- Notes: 1. The DMSTA forecast error for flow-weighted mean concentrations is approximately +/-23% of the expected value. There is associated uncertainty in these predictions and actual performance will vary.
2. Outflow values highlighted in yellow had one or more years of the 36 years simulated below the low end of the calibration data set (enumerated by the number in parentheses).
3. Diversions related to STA-1E are directed to eastern C-51.



Figure 2-2: Comparison of Inflows and Outflows for STA-1E.



3. STA-1W

Working in concert with STA-1E, STA-1W will capture and treat runoff from the C-51 Basin, Acme Basin B, L-8 Basin, S-5A Basin and the East Beach Water Control District. A schematic of STA-1W is presented in **Figure 3-1**. The long-term average annual inflow to STA-1W by source is summarized in **Table 3-1**. Although the long-term goal is to treat less inflow in STA-1W than shown in **Table 3-1**, it is recognized that during the interim period before ECART and the L-8 Basin projects are complete, STA-1W inflows will be higher than the long-term goals. With complete diversion of the L-8 Basin runoff and without implementation of ECART, the long-term average annual inflows to STA-1W will be lower than presented in **Table 3-1**.

Figure 3-1: Schematic of STA-1W (Not to Scale).

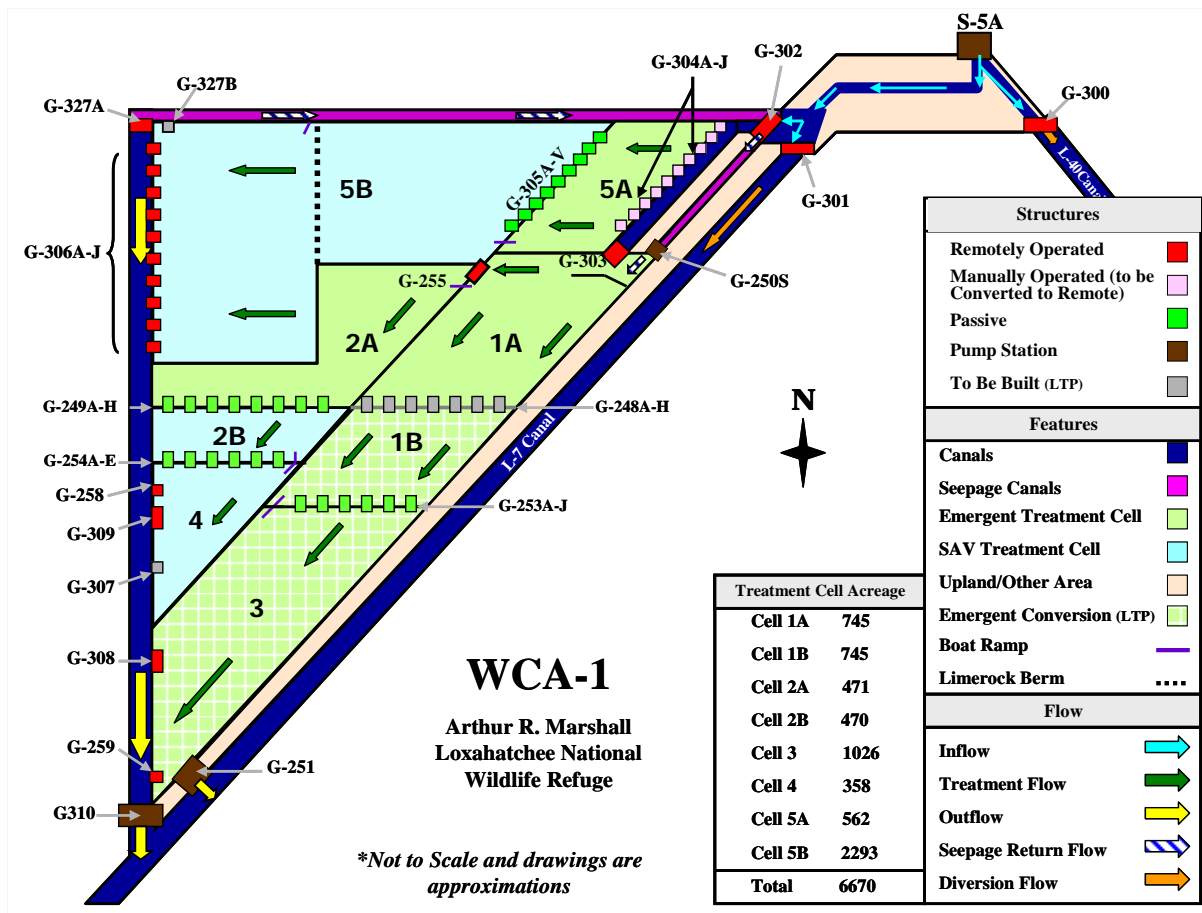


Table 3-1: Summary of Long-term Average Annual Inflow to STA-1W.

| Source | Flow (AF/yr) | TP Load (kg/yr) | TP Conc (ppb) |
|--------------|----------------|-----------------|---------------|
| S-5A Basin | 228,143 | 45,753 | 163 |
| EBWCD | 15,005 | 8,651 | 467 |
| Total | 243,172 | 54,409 | 181 |

A summary of STA-1W phosphorus performance for the 2010 period is presented in the table and figures below. A copy of the DMSTA output is presented in Appendix C. A long-term flow-weighted mean outflow concentration range of 21-35 ppb was forecast, however this includes years when DMSTA forecast levels that fell below the calibration range of 15 ppb for an SAV system, and hence may be optimistic. DMSTA generates various warning and error messages based on the simulation results compared to the calibration data sets; these are displayed in the DMSTA results in Appendix C. For Cells 2B, 4 and 5B the mean depths and flow/width were slightly below the range of the SAV calibration data sets. For Cell 1A, the flow/width was about 30% above the range of the emergent calibration data sets.

Table 3-2: Summary of DMSTA Results for STA-1W.

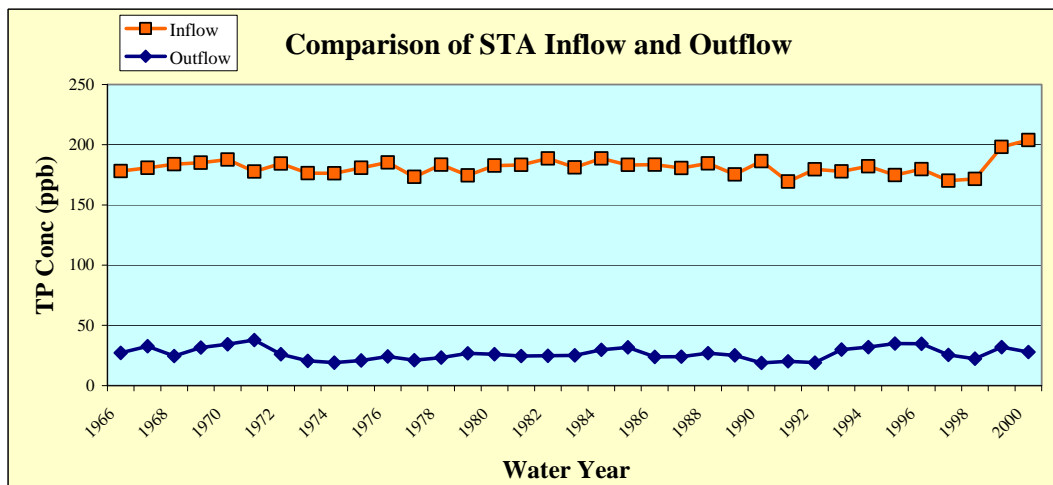
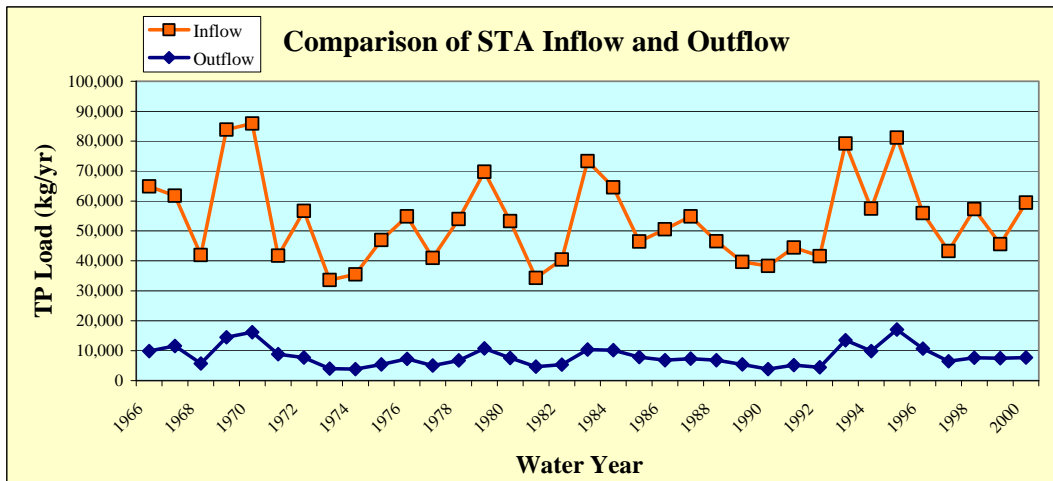
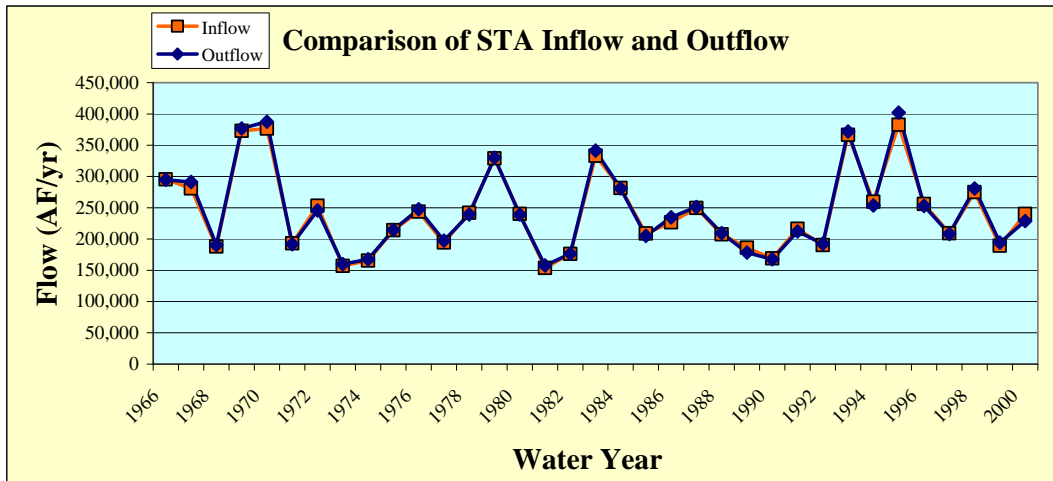
| Parameter | Unit | STA-1W |
|--|-------|---------|
| Effective Treatment Area | acres | 6,670 |
| Average Annual Inflow | | |
| Volume | AF/yr | 243,172 |
| TP Load | kg/yr | 54,409 |
| TP Concentration | ppb | 181 |
| Average Annual Outflow | | |
| Volume | AF/yr | 244,928 |
| Flow-weighted Mean TP Concentration | | |
| Upper Conf. Limit for Settling Rate | ppb | 21 (4) |
| Mean Estimate of Settling Rate | ppb | 27 |
| Lower Conf. Limit for Settling Rate | ppb | 35 |
| Geometric Mean TP Concentration | | |
| Upper Conf. Limit for Settling Rate | ppb | 20 |
| Mean Estimate of Settling Rate | ppb | 26 |
| Lower Conf. Limit for Settling Rate | ppb | 33 |
| TP Load (Using Mean TP Conc.) | kg/yr | 8,222 |
| Diversion Volumes and Loads | | |
| Volume | AF/yr | 0 |
| TP Load | kg/yr | 0 |
| TP Concentration | ppb | - |

Notes: 1. The DMSTA forecast error for flow-weighted mean concentrations is approximately +/-23% of the expected value. There is associated uncertainty in these predictions and actual performance will vary.

2. Outflow values highlighted in yellow had one or more years of the 36 years simulated below the low end of the calibration data set (enumerated by the number in parentheses).



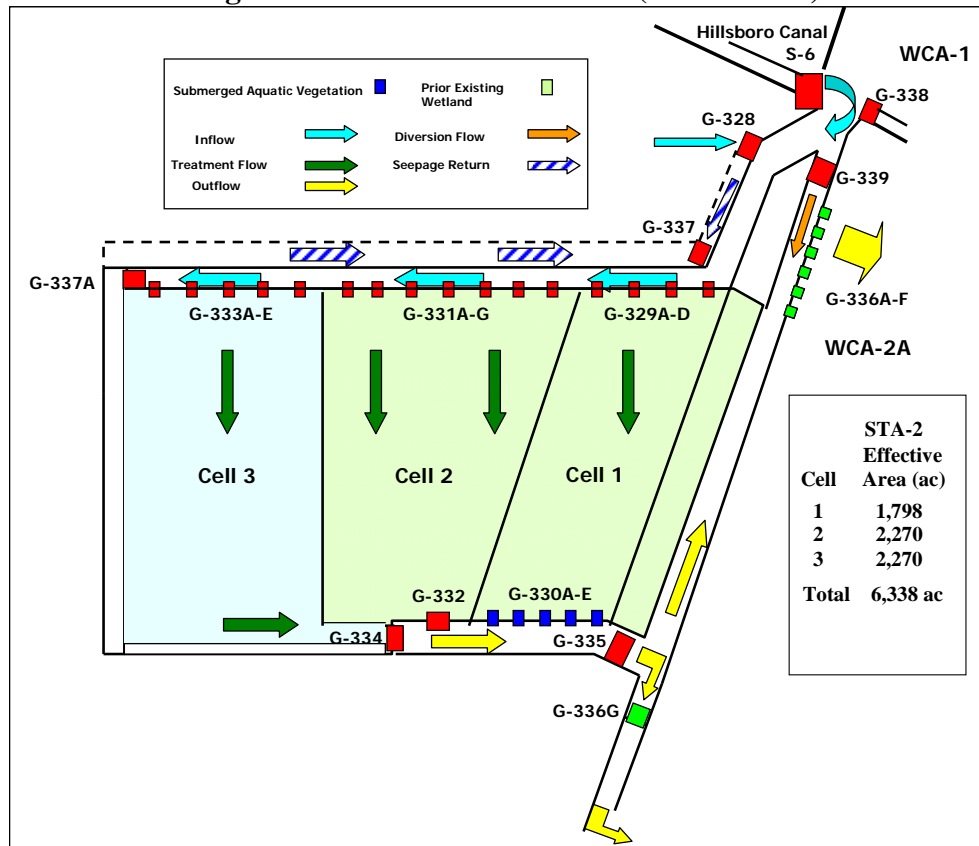
Figure 3-2: Comparison of Inflows and Outflows for STA-1W.



4. STA-2

A schematic of STA-2 is presented in **Figure 4-1**.

Figure 4-1: Schematic of STA-2 (Not to Scale).



When the SFWMM Alt1 simulation was developed, the capability did not exist in the model to re-direct a portion of STA-2 inflows west to Compartment B for treatment, with the result that a phosphorus loading rate (PLR) for STA-2 of $1.6 \text{ g/m}^2/\text{yr}$ was associated with this SFWMM simulation. By comparison for this same SFWMM simulation, the PLR for Compartment B with no re-direction from STA-2 was approximately $0.36 \text{ g/m}^2/\text{yr}$, indicating available treatment capacity in Compartment B. Consistent with the Compartment B Basis of Design Report (Brown & Caldwell 2007), and because the structural components needed to re-direct water from the S-6 pump station west to Compartment B will be in place upon completion of the Compartment B STA, a portion of STA-2 inflows can in fact be directed to Compartment B in order to reduce the phosphorus loading rate for STA-2. For the purpose of optimizing the treatment performance for the 2010 scenario simulated by the SFWMM Alt1, even though the model did not simulate such a redirection, the District can in reality re-direct STA-2 inflows to Compartment B to better balance the PLR among the treatment areas. A PLR of $1.0 \text{ g/m}^2/\text{yr}$ would balance the loading rate between STA-2 and the North Build-out area of Compartment B (the South Build-out will not receive STA-2 re-direction).



Hence for the purpose of optimizing the treatment performance for the 2010 scenario simulated by SFWMM Alt1, a sufficient quantity of STA-2 inflows was re-directed to Compartment B North Build-out in order to achieve a PLR of 1.0 g/m²/yr. **It is important to note that a PLR of 1.0 g/m²/yr was used for this analysis as a rough target for balancing the loading rate between STA-2 and Compartment B, specifically the North Build-out area, and not as an ultimate PLR goal for STAs in general.** In the future, for example, upon the completion of ECART, the re-distribution of a portion of STA-2 inflows to Compartment B will be re-evaluated to optimize regional benefits. A long-term average annual re-direction of 118,810 AF/yr accomplishes this balanced PLR of 1.0 g/m²/yr for STA-2 and the North Build-out of Compartment B. The PLR for the South Build-out, which will receive the balance of runoff from the S-7/S-2 Basin, is estimated as 0.5 g/m²/yr. The resulting PLR for the entire Compartment B is approximately 0.8 g/m²/yr, indicating some remaining unused treatment capacity in Compartment B even with the above re-distribution scenario. The resulting long-term average annual inflow to STA-2 by source is summarized in **Table 4-1**, showing the inflows before and after the re-direction.

Table 4-1: Long-term Average Annual Inflow to STA-2 For a PLR of 1.0 g/m²/yr.

| Source | STA-2 Inflows To Achieve a PLR of 1.0 g/m ² /yr | | |
|-------------------------------------|--|-----------------|---------------|
| | Flow (AF/yr) | TP Load (kg/yr) | TP Conc (ppb) |
| WCA-2A Seepage | 27,530 | 509 | 15 |
| S-5A Basin | 61,148 | 12,289 | 163 |
| ESWCD & 715 Farms | 31,129 | 5,215 | 136 |
| S-6/S-2 Basin | 181,700 | 23,661 | 106 |
| Inflow Prior to Re-direction | 301,507 | 41,675 | 112 |
| Re-direct to Compartment B | -118,810 | -16,012 | 109 |
| Net Inflow | 182,697 | 25,662 | 114 |

A summary of STA-2 phosphorus performance for the 2010 period with a PLR of 1.0 g/m²/yr is presented in the table and figures below. A copy of the DMSTA output is presented in Appendix C. A long-term flow-weighted mean outflow concentration range of 19-27 ppb was forecast, however this includes years when DMSTA forecast levels that fell below the calibration range of 15 ppb for an SAV system, and hence may be optimistic. DMSTA generates various warning and error messages based on the simulation results compared to the calibration data sets; these are displayed in the DMSTA results in Appendix C. For Cells 1 and 2, the long-term average inflow concentration was approximately 4% above the range of the prior existing wetland (PEW) calibration data sets. For Cell 3 the mean depth and flow/width were slightly below the range of the SAV calibration data sets.



Table 4-2: Summary of DMSTA Results for STA-2 with a PLR of 1.0 g/m²/yr.

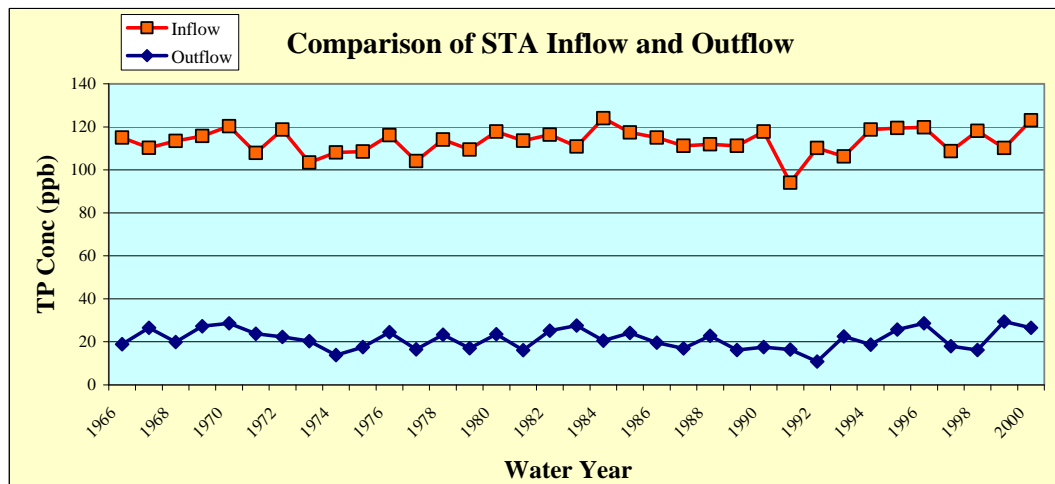
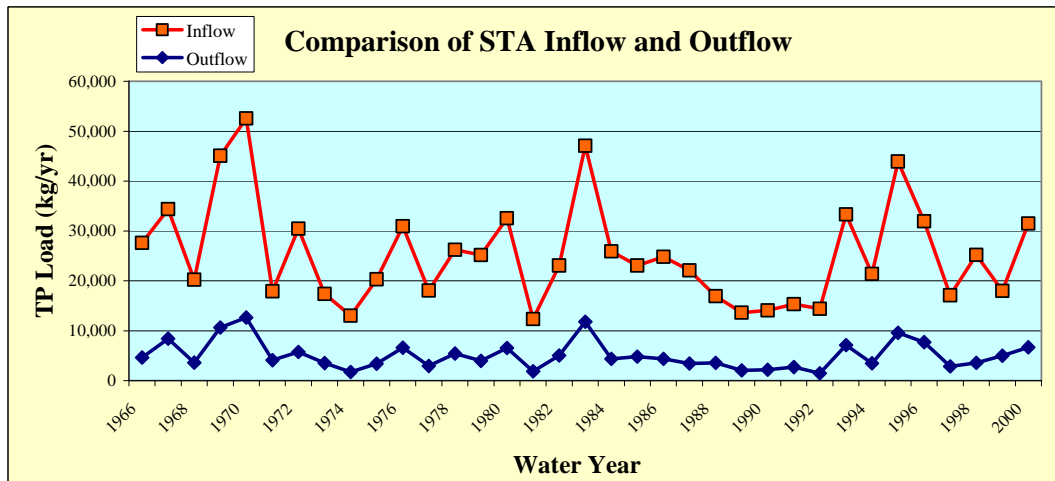
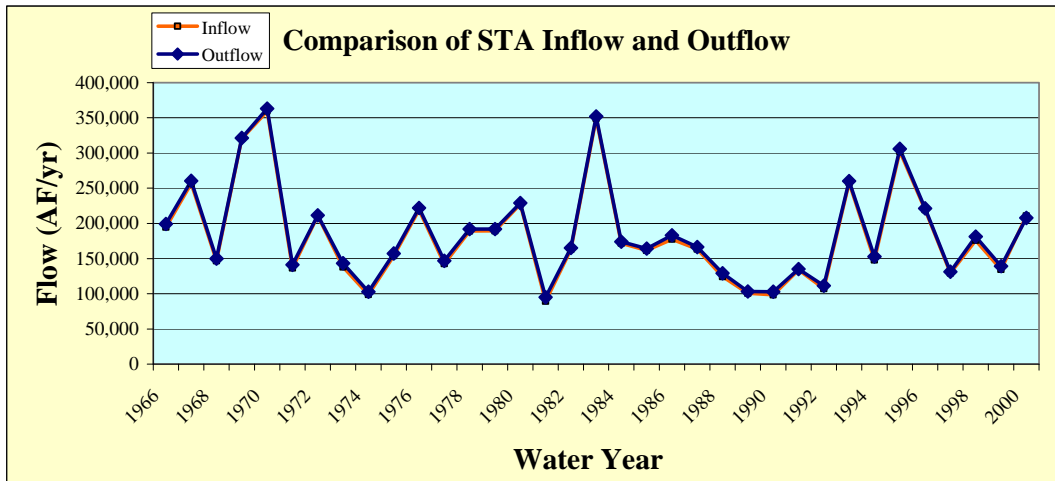
| Parameter | Unit | STA-2 - 1.0 g/m ² /yr |
|--|-------|-------------------------------------|
| Effective Treatment Area | acres | 6,338 |
| Average Annual Inflow | | |
| Volume | AF/yr | 182,697 |
| TP Load | kg/yr | 25,662 |
| TP Concentration | ppb | 114 |
| Average Annual Outflow | | |
| Volume | AF/yr | 186,047 |
| Flow-weighted Mean TP Concentration | | |
| Upper Conf. Limit for Settling Rate | ppb | 19 (2) |
| Mean Estimate of Settling Rate | ppb | 22 (1) |
| Lower Conf. Limit for Settling Rate | ppb | 27 |
| Geometric Mean TP Concentration | | |
| Upper Conf. Limit for Settling Rate | ppb | 17 |
| Mean Estimate of Settling Rate | ppb | 21 |
| Lower Conf. Limit for Settling Rate | ppb | 25 |
| TP Load (Using Mean TP Conc.) | kg/yr | 5,145 |
| Diversion Volumes and Loads | | |
| Volume | AF/yr | 0 |
| TP Load | kg/yr | 0 |
| TP Concentration | ppb | - |

Notes: 1. The DMSTA forecast error for flow-weighted mean concentrations is approximately +/-23% of the expected value. There is associated uncertainty in these predictions and actual performance will vary.

2. Outflow values highlighted in yellow had one or more years of the 36 years simulated below the low end of the calibration data set (enumerated by the number in parentheses).



Figure 4-2: Comparison of Inflows and Outflows for STA-2 with a PLR of 1.0 g/m²/yr.



4.1. Sensitivity Analysis: Redirection to Compartment B to Achieve an STA-2 Phosphorus Loading Rate of 1.3 g/m²/yr

An alternative re-direction target was investigated as a sensitivity analysis. An investigation of the performance of STA treatment cells and other Florida treatment wetlands by Juston and DeBusk (2005) identified a PLR of 1.3 g/m²/yr as a potential breakpoint between “well-performing” and “challenged” treatment areas. **The simulation of a PLR for STA-2 of 1.3 g/m²/yr was conducted for the purpose of comparing alternatives and does not represent an ultimate PLR goal for STAs.** The long-term PLR is just one of many factors that influence the phosphorus removal performance of an STA; others include

- Vegetation type
- Soil type
- Antecedent land use
- Phosphorus loading history
- Inflow concentrations
- Hurricanes, droughts and other disturbances

As a reference, STA-2 is presently receiving a PLR of 1.34 g/m²/yr and is one of the highest performing STAs, with a long-term average flow-weighted mean of 21 ppb (Pietro et al. 2007). In addition, STA-3/4 has been receiving phosphorus at a PLR averaging 1.5 g/m²/yr and is producing the lowest outflow phosphorus concentration of all the STAs, with a 3-year average flow-weighted mean of 19 ppb (Pietro et al. 2007). These STA performance results suggest that STAs loaded above a PLR 1.3 g/m²/yr may still achieve optimal performance, which could allow a greater inflow to STA-2 than presently modeled, thus increasing the available treatment capacity in Compartment B. A long-term average annual re-direction of 61,225 AF/yr accomplishes this PLR of 1.3 g/m²/yr for the SFWMM Alt1 simulation. The PLR for the North Build-out under this re-direction scenario is estimated as 0.6 g/m²/yr. The PLR for the South Build-out, which will receive the balance of runoff from the S-7/S-2 Basin, is estimated as 0.5 g/m²/yr. The PLR for the entire Compartment B is approximately 0.6 g/m²/yr, well below the 1.3 g/m²/yr value and indicating surplus treatment capacity in Compartment B. Using this re-direction quantity, the long-term average annual inflow to STA-2 by source is summarized in **Table 4-3**.

Table 4-3: Long-term Average Annual Inflow to STA-2 For a PLR of 1.3 g/m²/yr.

| Source | Flow (AF/yr) | TP Load (kg/yr) | TP Conc (ppb) |
|-------------------------|----------------|-----------------|---------------|
| WCA-2A Seepage | 27,530 | 509 | 15 |
| S-5A Basin | 61,148 | 12,289 | 163 |
| ESWCD & 715 Farms | 31,129 | 5,215 | 136 |
| S-6/S-2 Basin | 181,700 | 23,661 | 106 |
| Divert to Compartment B | -61,225 | -8,322 | 110 |
| Total | 240,282 | 33,353 | 113 |



A summary of STA-2 phosphorus performance for the 2010 period with a PLR of 1.3 g/m²/yr is presented in the table and figures below. A copy of the DMSTA output is presented in Appendix C. A long-term flow-weighted mean outflow concentration range of 19-29 ppb was forecast compared to 19-27 ppb for a PLR of 1.0 g/m²/yr, however this includes years when DMSTA forecast levels that fell below the calibration range of 15 ppb for an SAV system, and hence may be optimistic. DMSTA generates various warning and error messages based on the simulation results compared to the calibration data sets; these are displayed in the DMSTA results in Appendix C. For Cells 1 and 2, the long-term average inflow concentration was approximately 3% above the range of the prior existing wetland (PEW) calibration data sets. For Cell 3 the mean depth and flow/width were slightly below the range of the SAV calibration data sets.

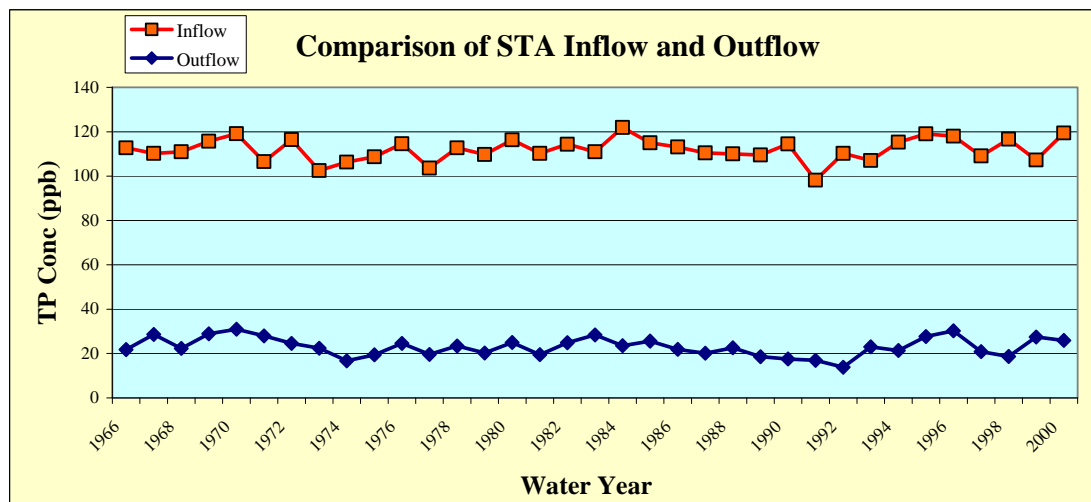
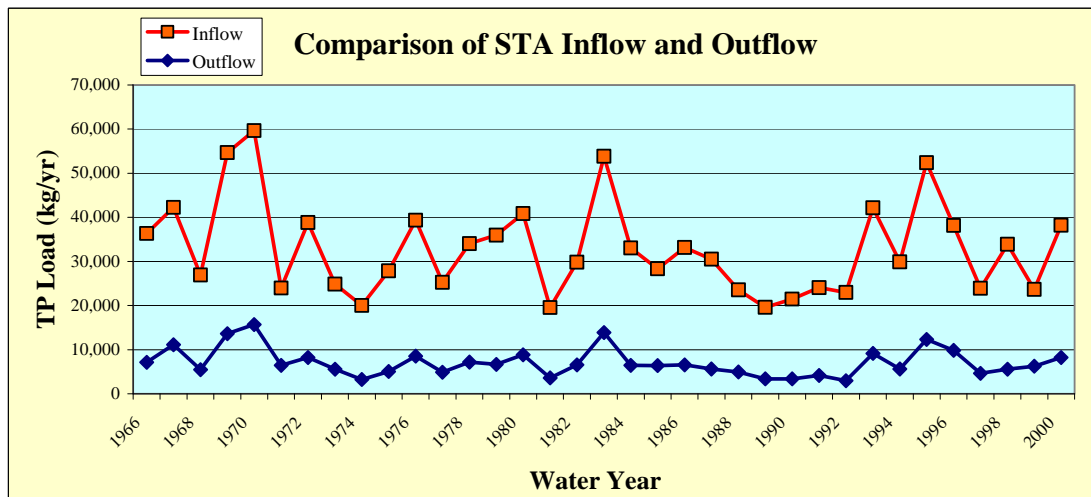
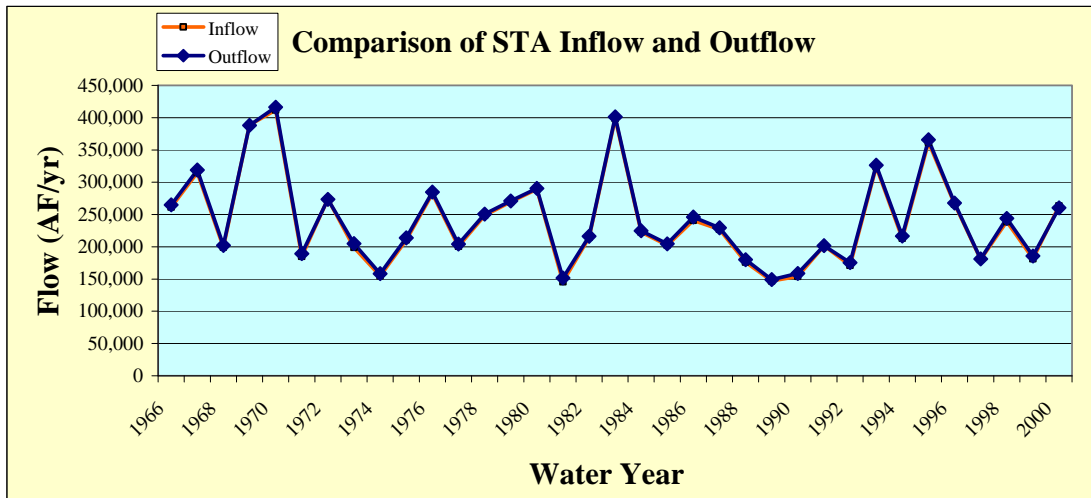
Table 4-4: Summary of DMSTA Results for STA-2 with a PLR of 1.3 g/m²/yr.

| Parameter | Unit | STA-2 - 1.3 g/m ² /yr |
|--|-------|----------------------------------|
| Effective Treatment Area | acres | 6,338 |
| Average Annual Inflow | | |
| Volume | AF/yr | 240,282 |
| TP Load | kg/yr | 33,353 |
| TP Concentration | ppb | 113 |
| Average Annual Outflow | | |
| Volume | AF/yr | 243,339 |
| Flow-weighted Mean TP Concentration | | |
| Upper Conf. Limit for Settling Rate | ppb | 19 (1) |
| Mean Estimate of Settling Rate | ppb | 24 |
| Lower Conf. Limit for Settling Rate | ppb | 29 |
| Geometric Mean TP Concentration | | |
| Upper Conf. Limit for Settling Rate | ppb | 18 |
| Mean Estimate of Settling Rate | ppb | 22 |
| Lower Conf. Limit for Settling Rate | ppb | 28 |
| TP Load (Using Mean TP Conc.) | kg/yr | 7,151 |
| Diversion Volumes and Loads | | |
| Volume | AF/yr | 0 |
| TP Load | kg/yr | 0 |
| TP Concentration | ppb | - |

Notes: 1. The DMSTA forecast error for flow-weighted mean concentrations is approximately +/-23% of the expected value. There is associated uncertainty in these predictions and actual performance will vary.
 2. Outflow values highlighted in yellow had one or more years of the 36 years simulated below the low end of the calibration data set (enumerated by the number in parentheses).



Figure 4-3: Comparison of Inflows and Outflows for STA-2 with a PLR of 1.3 g/m²/yr.



As discussed in Section 4 above, a PLR to STA-2 of 1.0 g/m²/yr would balance the loading rate between STA-2 and the North Build-out area of Compartment B (the South Build-out will not receive STA-2 re-direction). Hence for the purpose of optimizing the treatment performance for the 2010 scenario simulated by SFWMM Alt1, a sufficient quantity of STA-2 inflows was re-directed to Compartment B North Build-out in order to achieve a PLR of 1.0 g/m²/yr. **It is important to note that a PLR of 1.0 g/m²/yr was used for this analysis as a rough target for balancing the loading rate between STA-2 and Compartment B, specifically the North Build-out area, and not as an ultimate PLR goal for STAs in general.** In the future, for example, upon the completion of ECART, the re-distribution of a portion of STA-2 inflows to Compartment B will be re-evaluated to optimize regional benefits. The resulting long-term average annual inflow to Compartment B by source is summarized in **Table 5-1**, showing the inflows before and after the re-direction.

Table 5-1: Long-term Average Annual Inflow to Compartment B For an STA-2 PLR of 1.0 g/m²/yr.

| Source | Flow (AF/yr) | TP Load (kg/yr) | TP Conc (ppb) |
|-------------------------|----------------|-----------------|---------------|
| S-7/S-2 Basin | 106,069 | 12,411 | 95 |
| Re-direction from STA-2 | | | |
| WCA-2A Seepage | 10,848 | 201 | 15 |
| S-6/S-2 Basin | 71,599 | 9,324 | 106 |
| S-5 Basin | 24,096 | 4,843 | 163 |
| ESWCD & 715 Farms | 12,267 | 2,055 | 136 |
| Total | 224,879 | 28,833 | 104 |

A summary of Compartment B phosphorus performance for the 2010 period is presented in the table and figures below. A copy of the DMSTA output is presented in Appendix C. DMSTA forecast long-term average annual phosphorus concentrations of 9.1-14.6 ppb, which are below the minimum of the calibration data sets for SAV (15 ppb), and those forecasts were replaced in Table 5-2 by 15 ppb. The adjusted outflow phosphorus levels in Table 5-2 may still portray optimistic results in that the best performing STA (STA-3/4) is presently averaging about 19 ppb, with a 5 ppb standard deviation on annual values. DMSTA generates various warning and error messages based on the simulation results compared to the calibration data sets; these are displayed in the DMSTA results in Appendix C. For the North and South Build-out cells, the mean depth and flow/width were slightly below the range of the SAV calibration data sets.



Table 5-2: DMSTA Results for Comp. B with an STA-2 PLR of 1 g/m²/yr.

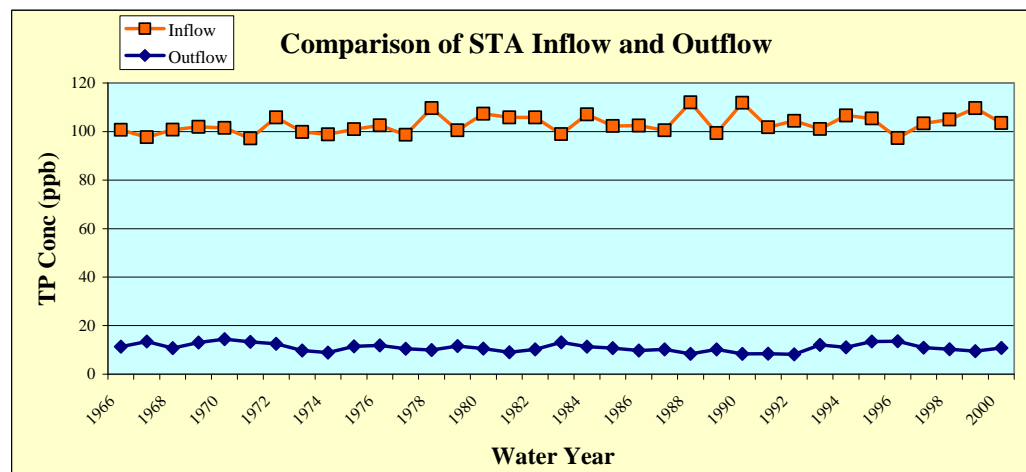
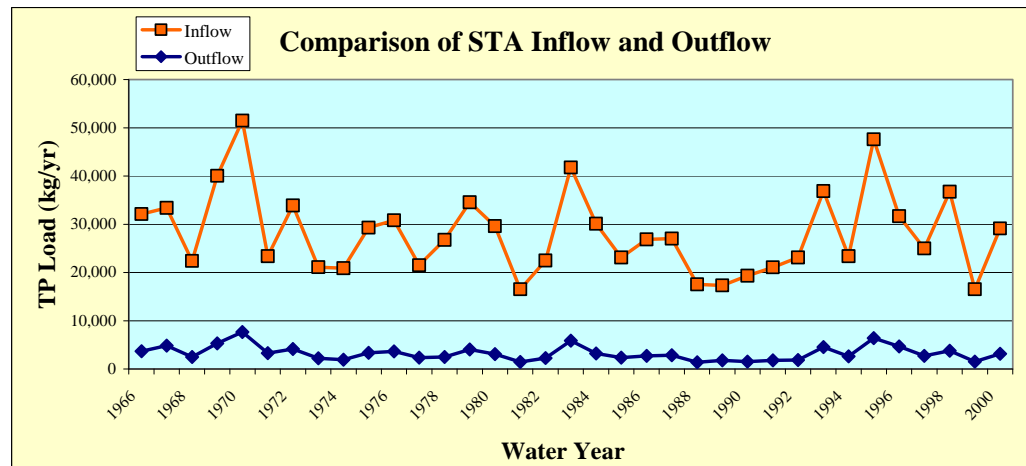
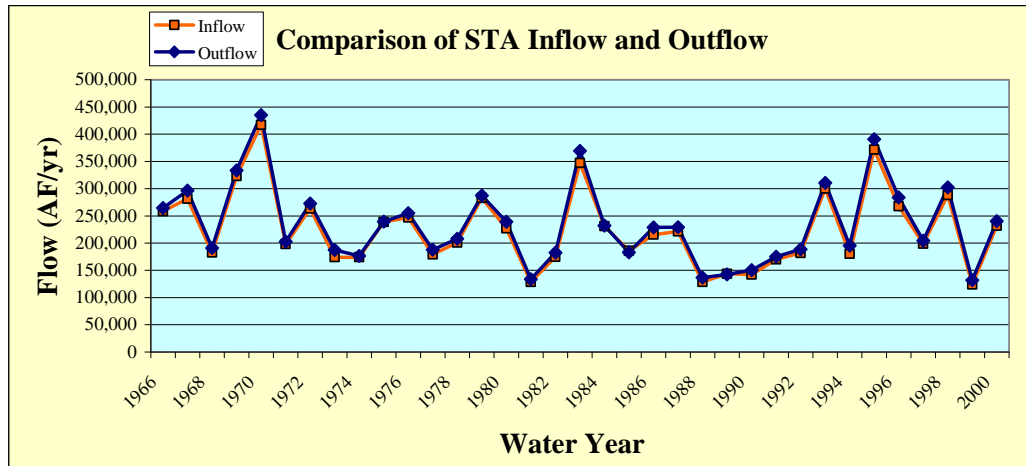
| Parameter | Unit | Comp. B North | Comp. B South | Comp. B Combined |
|--|-------|---------------|---------------|------------------|
| Effective Treatment Area | acres | 5,824 | 2,796 | 8,620 |
| Average Annual Inflow | | | | |
| Volume | AF/yr | 177,228 | 47,651 | 224,879 |
| TP Load | kg/yr | 22,932 | 5,553 | 28,833 |
| TP Concentration | ppb | 105 | 94 | 104 |
| Average Annual Outflow | | | | |
| Volume | AF/yr | 180,541 | 53,378 | 233,919 |
| Flow-weighted Mean TP Concentration | | | | |
| Upper Conf. Limit for Settling Rate | ppb | 15 (35) | 15 (35) | 15 (35) |
| Mean Estimate of Settling Rate | ppb | 15 (35) | 15 (34) | 15 (35) |
| Lower Conf. Limit for Settling Rate | ppb | 15 (27) | 15 (25) | 15 (27) |
| Geometric Mean TP Concentration | | | | |
| Upper Conf. Limit for Settling Rate | ppb | 15 | 15 | 15 |
| Mean Estimate of Settling Rate | ppb | 15 | 15 | 15 |
| Lower Conf. Limit for Settling Rate | ppb | 15 | 15 | 15 |
| TP Load (Using Mean TP Conc.) | kg/yr | 3,340 | 988 | 4,328 |
| Diversion Volumes and Loads | | | | |
| Volume | AF/yr | 0 | 0 | 0 |
| TP Load | kg/yr | 0 | 0 | 0 |
| TP Concentration | ppb | - | - | - |

Notes: 1. The DMSTA forecast error for flow-weighted mean concentrations is approximately +/-23% of the expected value. There is associated uncertainty in these predictions and actual performance will vary.

2. Outflow values highlighted in yellow had one or more years of the 35 year simulated below the low end of the calibration data set (enumerated by the number in parentheses). The lowest sustainable STA outflow phosphorus concentration is 19 ppb (STA-3/4), with a standard deviation of 5 ppb.



Figure 5-2: Comparison of Inflows and Outflows for Compartment B with an STA-2 PLR of 1.0 g/m²/yr.



Note: TP concentrations below 15 ppb have not been sustained.



5.1. Sensitivity Analysis: Redirection to Compartment B to Achieve an STA-2 Phosphorus Loading Rate of 1.3 g/m²/yr

An alternative re-direction target was investigated as a sensitivity analysis. An investigation of the performance of STA treatment cells and other Florida treatment wetlands by Juston and DeBusk (2005) identified a PLR of 1.3 g/m²/yr as a potential breakpoint between “well-performing” and “challenged” treatment areas. **The simulation of a PLR for STA-2 of 1.3 g/m²/yr was conducted for the purpose of comparing alternatives and does not represent an ultimate PLR goal for STAs.** The long-term PLR is just one of many factors that influence the phosphorus removal performance of an STA; others include

- Vegetation type
- Soil type
- Antecedent land use
- Phosphorus loading history
- Inflow concentrations
- Hurricanes, droughts and other disturbances

As a reference, STA-2 is presently receiving a PLR of 1.34 g/m²/yr and is one of the highest performing STAs, with a long-term average flow-weighted mean of 21 ppb (Pietro et al. 2007). In addition, STA-3/4 has been receiving phosphorus at a PLR averaging 1.5 g/m²/yr and is producing the lowest outflow phosphorus concentration of all the STAs, with a 3-year average flow-weighted mean of 19 ppb (Pietro et al. 2007). These STA performance results suggest that STAs loaded above a PLR 1.3 g/m²/yr may still achieve optimal performance, which could allow a greater inflow to STA-2 than presently modeled, thus increasing the available treatment capacity in Compartment B. A long-term average annual re-direction of 61,225 AF/yr accomplishes this PLR of 1.3 g/m²/yr for the SFWMM Alt1 simulation. The PLR for the North Build-out under this re-direction scenario is estimated as 0.6 g/m²/yr. The PLR for the South Build-out, which will receive the balance of runoff from the S-7/S-2 Basin, is estimated as 0.5 g/m²/yr. The PLR for the entire Compartment B is approximately 0.6 g/m²/yr, well below the 1.3 g/m²/yr value and indicating surplus treatment capacity in Compartment B. Using this re-direction quantity, the long-term average annual inflow to Compartment B by source is summarized in **Table 5-3**.

Table 5-3: Long-term Average Annual Inflow to Compartment B For an STA-2 PLR of 1.3 g/m²/yr.

| Source | Flow (AF/yr) | TP Load (kg/yr) | TP Conc (ppb) |
|-------------------|----------------|-----------------|---------------|
| S-7/S-2 Basin | 106,069 | 12,411 | 95 |
| WCA-2A Seepage | 5,590 | 103 | 15 |
| S-6/S-2 Basin | 36,896 | 4,805 | 106 |
| S-5 Basin | 12,417 | 2,496 | 163 |
| ESWCD & 715 Farms | 6,321 | 1,059 | 136 |
| Total | 167,294 | 20,873 | 101 |



As a reference, STA-2 is presently receiving a PLR of 1.34 g/m²/yr and is one of the highest performing STAs, with a long-term average flow-weighted mean of 21 ppb (Pietro et al. 2007). In addition, STA-3/4 has been receiving phosphorus at a PLR averaging 1.5 g/m²/yr and is producing the lowest outflow phosphorus concentration of all the STAs, with a 3-year average flow-weighted mean of 19 ppb (Pietro et al. 2007). These STA performance results, in combination with the results of this sensitivity analysis, suggest that STAs loaded above a PLR 1.3 g/m²/yr may still achieve optimal performance, which could allow a greater loading to STA-2 than presently modeled, thus increasing the available treatment capacity in Compartment B.

A summary of Compartment B phosphorus performance for the 2010 period with a PLR of 1.3 g/m²/yr to STA-2 is presented in the table and figures below. A copy of the DMSTA output is presented in Appendix C. As was the case for an STA-2 PLR of 1.0 g/m²/yr, DMSTA forecast long-term outflow concentrations below the low end of the SAV calibration data set (15 ppb) and these were set to the low end value, and hence may be optimistic. Judgment should be applied before using these results as a true indicator of long-term performance. By reference, the lowest sustainable outflow concentration from an STA to date is 19 ppb with a standard deviation of 5 ppb for STA-3/4. DMSTA generates various warning and error messages based on the simulation results compared to the calibration data sets; these are displayed in the DMSTA results in Appendix C. For the North and South Build-out cells, the mean depth and flow/width were slightly below the range of the SAV calibration data sets. For Cell 4, the mean depth was slightly below the range of the SAV calibration data sets.



Table 5-4: DMSTA Results for Compartment B with an STA-2 PLR of 1.3 g/m²/yr.

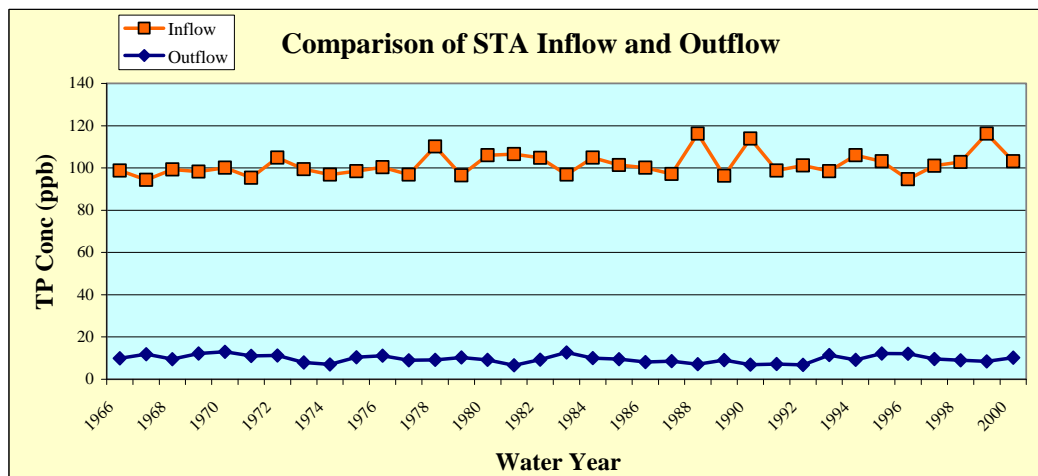
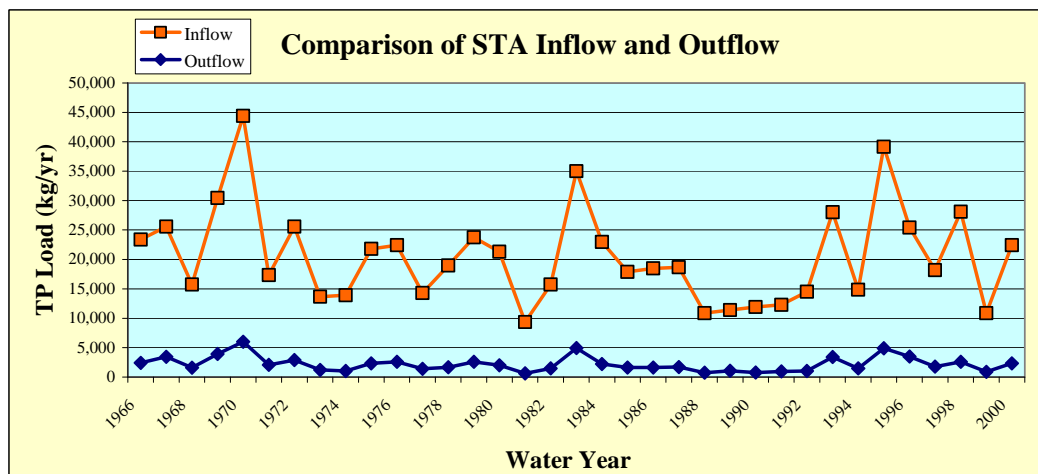
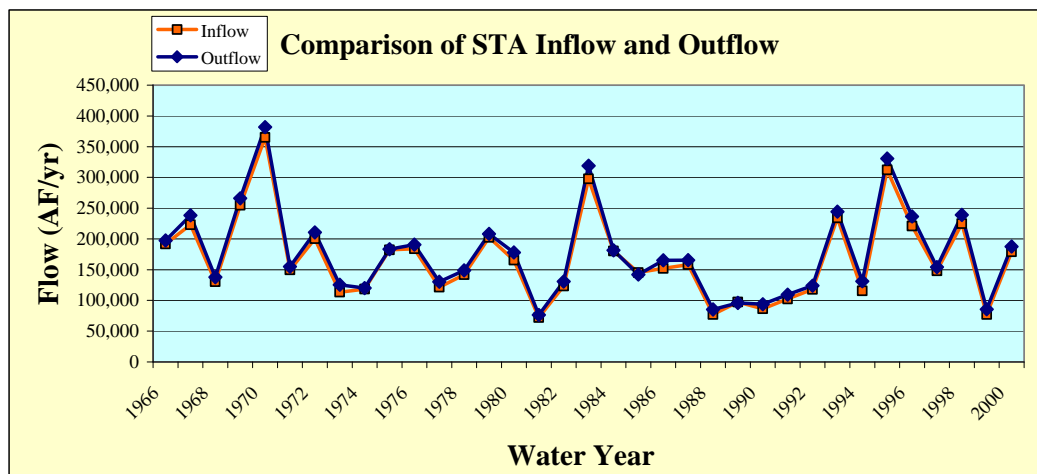
| Parameter | Unit | Comp. B North | Comp. B South | Comp. B Combined |
|--|-------|---------------|---------------|------------------|
| Effective Treatment Area | acres | 5,824 | 2,796 | 8,620 |
| Average Annual Inflow | | | | |
| Volume | AF/yr | 119,643 | 47,651 | 167,294 |
| TP Load | kg/yr | 15,242 | 5,553 | 20,873 |
| TP Concentration | ppb | 103 | 94 | 101 |
| Average Annual Outflow | | | | |
| Volume | AF/yr | 124,376 | 53,378 | 177,754 |
| Flow-weighted Mean TP Concentration | | | | |
| Upper Conf. Limit for Settling Rate | ppb | 15 (35) | 15 (35) | 15 (35) |
| Mean Estimate of Settling Rate | ppb | 15 (35) | 15 (34) | 15 (35) |
| Lower Conf. Limit for Settling Rate | ppb | 15 (35) | 15 (25) | 15 (34) |
| Geometric Mean TP Concentration | | | | |
| Upper Conf. Limit for Settling Rate | ppb | 15 | 15 | 15 |
| Mean Estimate of Settling Rate | ppb | 15 | 15 | 15 |
| Lower Conf. Limit for Settling Rate | ppb | 15 | 15 | 15 |
| TP Load (Using Mean TP Conc.) | kg/yr | 2,301 | 988 | 3,289 |
| Diversion Volumes and Loads | | | | |
| Volume | AF/yr | 0 | 0 | 0 |
| TP Load | kg/yr | 0 | 0 | 0 |
| TP Concentration | ppb | - | - | - |

Notes: 1. The DMSTA forecast error for flow-weighted mean concentrations is approximately +/-23% of the expected value. There is associated uncertainty in these predictions and actual performance will vary.

2. Outflow values highlighted in yellow had one or more years of the 35 year simulated below the low end of the calibration data set (enumerated by the number in parentheses). The lowest sustainable STA outflow phosphorus concentration is 19 ppb (STA-3/4), with a standard deviation of 5 ppb.



Figure 5-3: Comparison of Inflows and Outflows for Compartment B with an STA-2 PLR of 1.3 g/m²/yr.



Note: TP concentrations below 15 ppb have not been sustained.



6. EAA Storage Reservoir A-1

The network feature of DMSTA was used to model the combined EAA Storage Reservoir A-1 (EAASR A-1) and STA-3/4 system. This simulation generated a daily time series of flow and phosphorus levels from the reservoir back to the EAA for irrigation releases and to STA-3/4 for subsequent treatment. Upon review of the outflow time series to STA-3/4, it was observed that DMSTA was simulating releases during the dry season when the SFWMM results indicated no releases from the reservoir to STA-3/4, e.g., for January–May 1981 DMSTA simulated 12,620 AF in releases when SFWMM had 1,926 AF in releases. Further, the phosphorus concentrations associated with these dry season releases were quite high, exceeding 1,600 ppb. For the purpose of the STA-3/4 inflow, the DMSTA-generated time series was replaced by the daily flows from the SFWMM results and the phosphorus concentrations from DMSTA. Although this won't resolve the high concentrations during the dry seasons, it will reduce the frequency of their occurrence. This new time series was then used in combination with basin runoff flows for an independent DMSTA simulation of STA-3/4 that allowed an uncertainty analysis of the STA performance, using the 10%, mean and 90% values of the effective settling rates for the specific vegetation types within STA-3/4.

Several assumptions were incorporated in the DMSTA modeling of the EAASR A-1:

- The SFWMM models the EAASR as two reservoir compartments, EARSN and EARSS. These were simulated in DMSTA as a single reservoir cell.
- Many reservoir characteristics were identical to those evaluated during the Basis of Design Report for the EAASR A-1 (Black and Veatch 2006):
 - The effective treatment area of the EAASR A-1 is 15,200 acres
 - The minimum depth for releases is 15.2 cm
 - The outflow weir depth for bypass is 12.5 ft
 - Seepage characteristics
- Other reservoir characteristics include:
 - The average flow width is 4.5 miles which is the average of the east-west width at the north end of EAASR and the width at the south end
 - The mean settling rate for the reservoir calibration data sets (5 m/yr) was used to simulate phosphorus removal in the reservoir.
- Daily rainfall and evapotranspiration from the SFWMM Alt1 scenario were used in the DMSTA simulation.
- Reservoir depths from the SFWMM Alt1 scenario were used in the DMSTA simulation. The depth time series provides an appropriate range of depths in the reservoir, based on model assumptions of footprint and volume, but is not intended as a true estimate of reservoir depth. Reservoir depth time series are not recommended for calculations outside the 2x2, however, the depth time series can be used as a reference for feasibility-level work such as the present analysis.



- The daily flow time series from the SFWMM Alt simulation quantifying the flow from the EAASR to STA-3/4 (WCS4S and EVBLSS) were used as outflow time series within the DMSTA simulation of the reservoir.

Using the recent phosphorus concentration of Lake Okeechobee releases as 100 ppb, the long-term average annual inflow to the EAASR A-1 by source is summarized in **Table 6-1**.

Table 6-1: Summary of Long-term Average Annual Inflow to EAASR A-1.

| Source | Flow (AF/yr) | TP Load (kg/yr) | TP Conc (ppb) |
|---------------|----------------|-----------------|---------------|
| S-7/S-2 Basin | 119,549 | 14,011 | 95 |
| S-8/S-3 Basin | 97,242 | 10,012 | 83 |
| Lake Okee. | 323,222 | 39,862 | 100 |
| Total | 540,013 | 63,885 | 96 |

A summary of EAASR A-1 phosphorus performance for the 2010 period is presented in the table and figures below. For WY1990, the simulated flow-weighted mean outflow concentration exceeded the inflow concentration, although the outflow loads were only 25% of the inflow loads. A copy of the DMSTA output is presented in Appendix C. No warning or error messages were generated during the DMSTA simulation for the EAASR A-1. However, the minimum simulated depth was 1 cm, indicating that DMSTA numerically prevented dry-out by reducing the rates of ET, seepage, etc. In reality the reservoir would have dried out and phosphorus levels may temporarily increase upon rewetting; depending on how much time passes between rewetting and discharge to the STA, the impact of this temporary increase in phosphorus levels could vary, however, the long-term concentration shown in Table 6-2 may be optimistic.



Table 6-2: Summary of DMSTA Results for EAASR A-1 (Lake TP Conc. of 100 ppb).

| Parameter | Unit | EAASR A-1 Note 2 |
|--|-------|---------------------|
| Effective Treatment Area | acres | 15,200 |
| Average Annual Inflow | | |
| Volume | AF/yr | 540,013 |
| TP Load | kg/yr | 63,885 |
| TP Concentration | ppb | 96 |
| Average Annual Outflow | | |
| Volume | AF/yr | 363,442 |
| Flow-weighted Mean TP Concentration | | |
| Upper Conf. Limit for Settling Rate | ppb | - |
| Mean Estimate of Settling Rate | ppb | 80 |
| Lower Conf. Limit for Settling Rate | ppb | - |
| Geometric Mean TP Concentration | | |
| Upper Conf. Limit for Settling Rate | ppb | - |
| Mean Estimate of Settling Rate | ppb | 93 |
| Lower Conf. Limit for Settling Rate | ppb | - |
| TP Load (Using Mean TP Conc.) | kg/yr | 35,730 |
| Diversion Volumes and Loads | | |
| Volume | AF/yr | 0 |
| TP Load | kg/yr | 0 |
| TP Concentration | ppb | - |

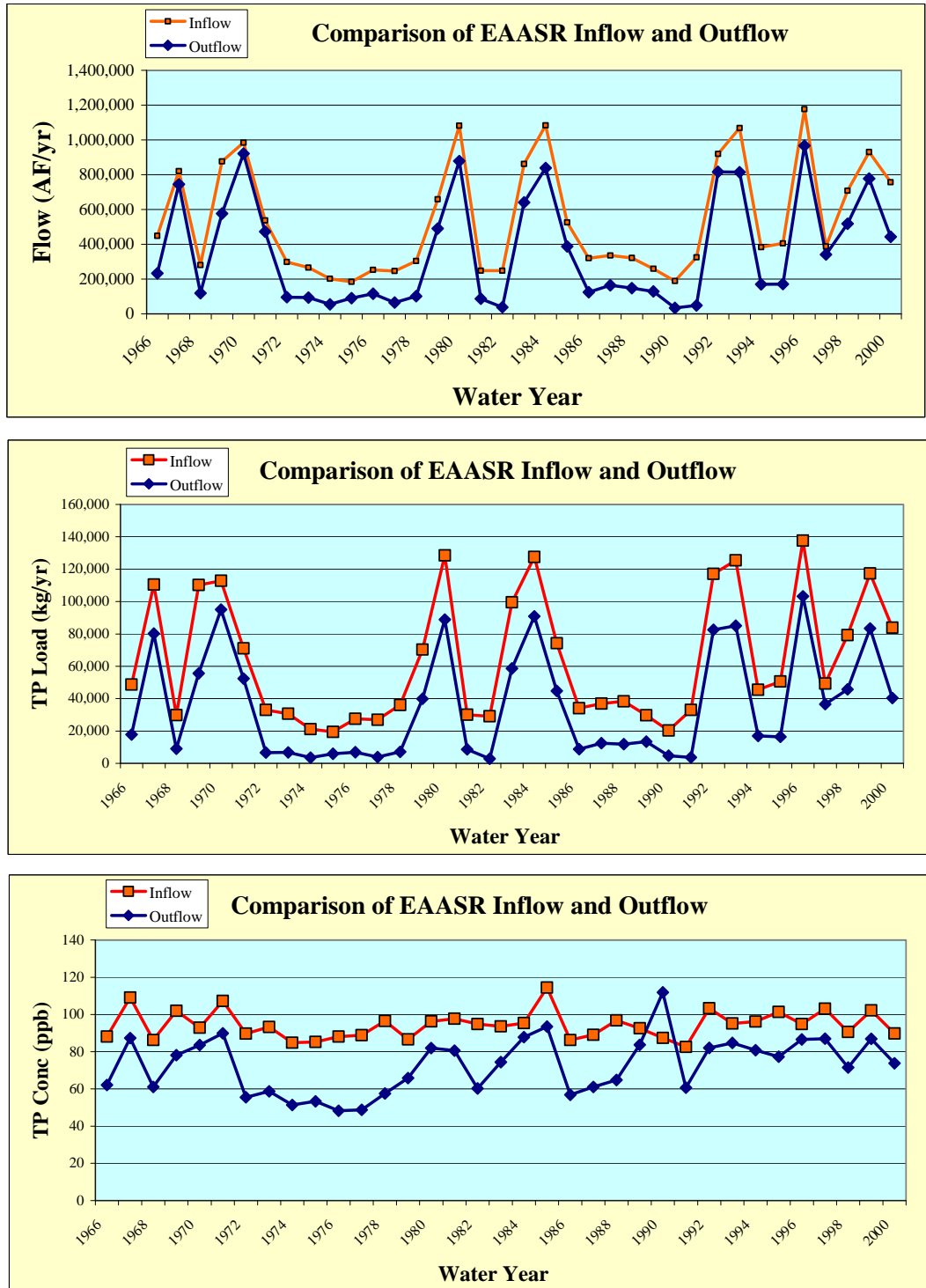
Notes: 1. The DMSTA forecast error for flow-weighted mean concentrations is approximately +/-23% of the expected value.

There is associated uncertainty in these predictions and actual performance will vary.

2. Shown are outflows to STA-3/4 after adjustment (see text); total outflows, including irrigation releases, were 514,607 AF/yr and 48,699 kg/yr at 77 ppb.



Figure 6-1: Comparison of Inflows and Outflows for EAASR A-1 (Lake TP Concentration of 100 ppb).



6.1. Sensitivity Analysis: Lake Okeechobee TP Concentration of 150 ppb

As an analysis of the sensitivity of reservoir and STA-3/4 performance to the phosphorus concentration of Lake Okeechobee releases, this section describes the performance of EAASR A-1 if the phosphorus concentration of Lake releases is 150 ppb at the inflow to the reservoir. With this assumption, the long-term average annual inflow to the EAASR A-1 by source is summarized in **Table 6-3**.

Table 6-3: Summary of Long-term Average Annual Inflow to EAASR.

| Source | Flow (AF/yr) | TP Load (kg/yr) | TP Conc (ppb) |
|---------------|----------------|-----------------|---------------|
| S-7/S-2 Basin | 119,549 | 14,011 | 95 |
| S-8/S-3 Basin | 97,242 | 10,012 | 83 |
| Lake Okee. | 323,222 | 59,799 | 150 |
| Total | 540,013 | 83,822 | 126 |

A summary of EAASR A-1 phosphorus performance for the 2010 period is presented in the table and figures below. A copy of the DMSTA output is presented in Appendix C. **The net effect of assuming a Lake Okeechobee phosphorus concentration of 150 ppb appears to be about 27 ppb at the outflow of the reservoir to STA-3/4, resulting in a long-term average additional 12.1 metric tons/yr of phosphorus to STA-3/4.**

No warning or error messages were generated during the DMSTA simulation for the EAASR A-1. However, the minimum simulated depth was 1 cm, indicating that DMSTA numerically prevented dry-out by reducing the rates of ET, seepage, etc. In reality the reservoir would have dried out and phosphorus levels would likely increase upon rewetting; thus the long-term concentration shown in Table 6-4 may be optimistic.



Table 6-4: Summary of DMSTA Results for EAASR A-1 (Lake TP Conc. of 150 ppb).

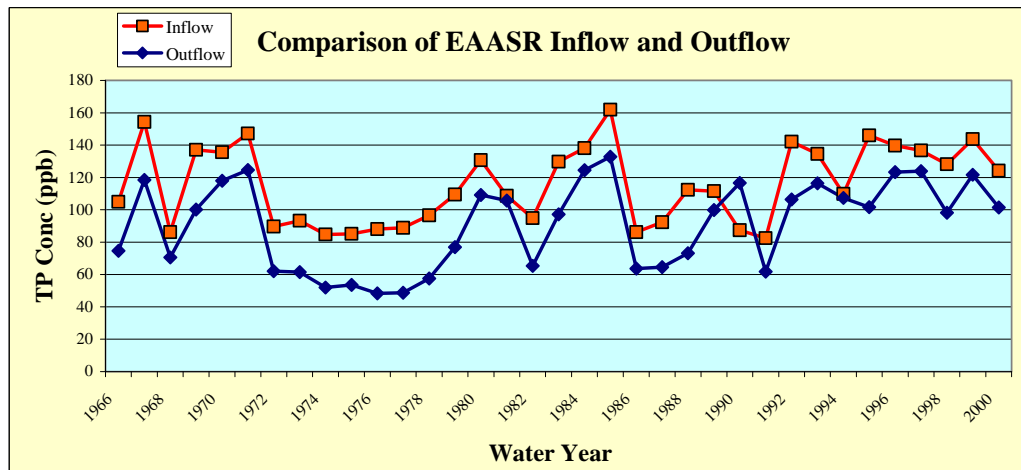
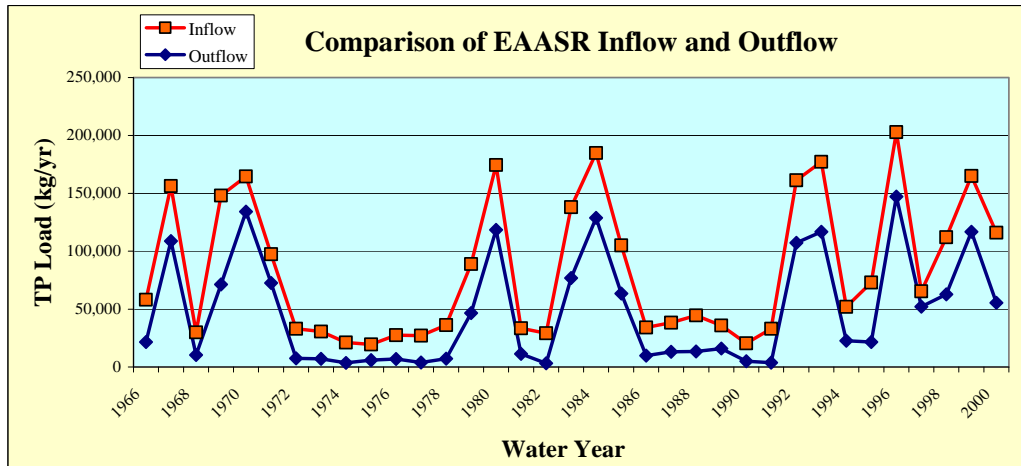
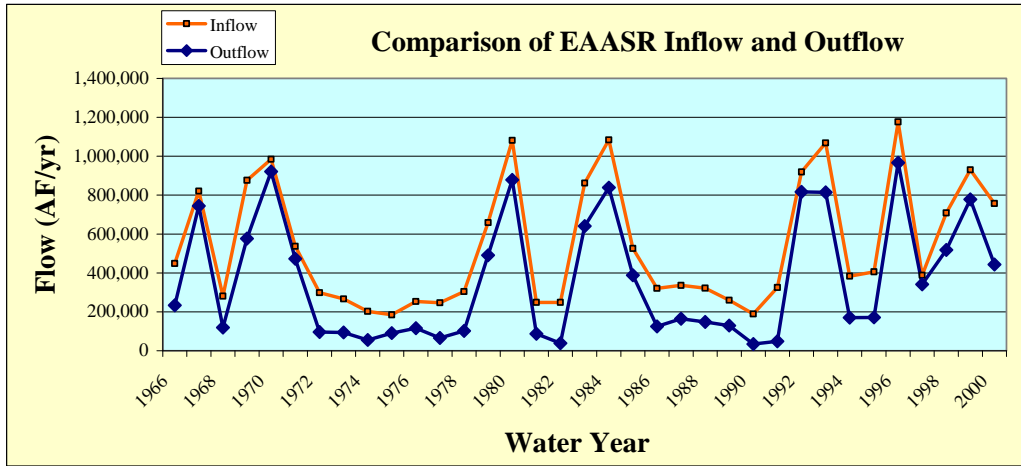
| Parameter | Unit | EAASR A-1 Note 2 |
|--|-------|---------------------|
| Effective Treatment Area | acres | 15,200 |
| Average Annual Inflow | | |
| Volume | AF/yr | 540,417 |
| TP Load | kg/yr | 83,883 |
| TP Concentration | ppb | 126 |
| Average Annual Outflow | | |
| Volume | AF/yr | 363,442 |
| Flow-weighted Mean TP Concentration | | |
| Upper Conf. Limit for Settling Rate | ppb | - |
| Mean Estimate of Settling Rate | ppb | 107 |
| Lower Conf. Limit for Settling Rate | ppb | - |
| Geometric Mean TP Concentration | | |
| Upper Conf. Limit for Settling Rate | ppb | .- |
| Mean Estimate of Settling Rate | ppb | 115 |
| Lower Conf. Limit for Settling Rate | ppb | - |
| TP Load (Using Mean TP Conc.) | kg/yr | 47,791 |
| Diversion Volumes and Loads | | |
| Volume | AF/yr | 0 |
| TP Load | kg/yr | 0 |
| TP Concentration | ppb | - |

Notes: 1. The DMSTA forecast error for flow-weighted mean concentrations is approximately +/-23% of the expected value. There is associated uncertainty in these predictions and actual performance will vary.

2. Shown are outflows to STA-3/4 after adjustment (see text); total outflows, including irrigation releases, were 514,607 AF/yr and 64,262 kg/yr at 101 ppb.



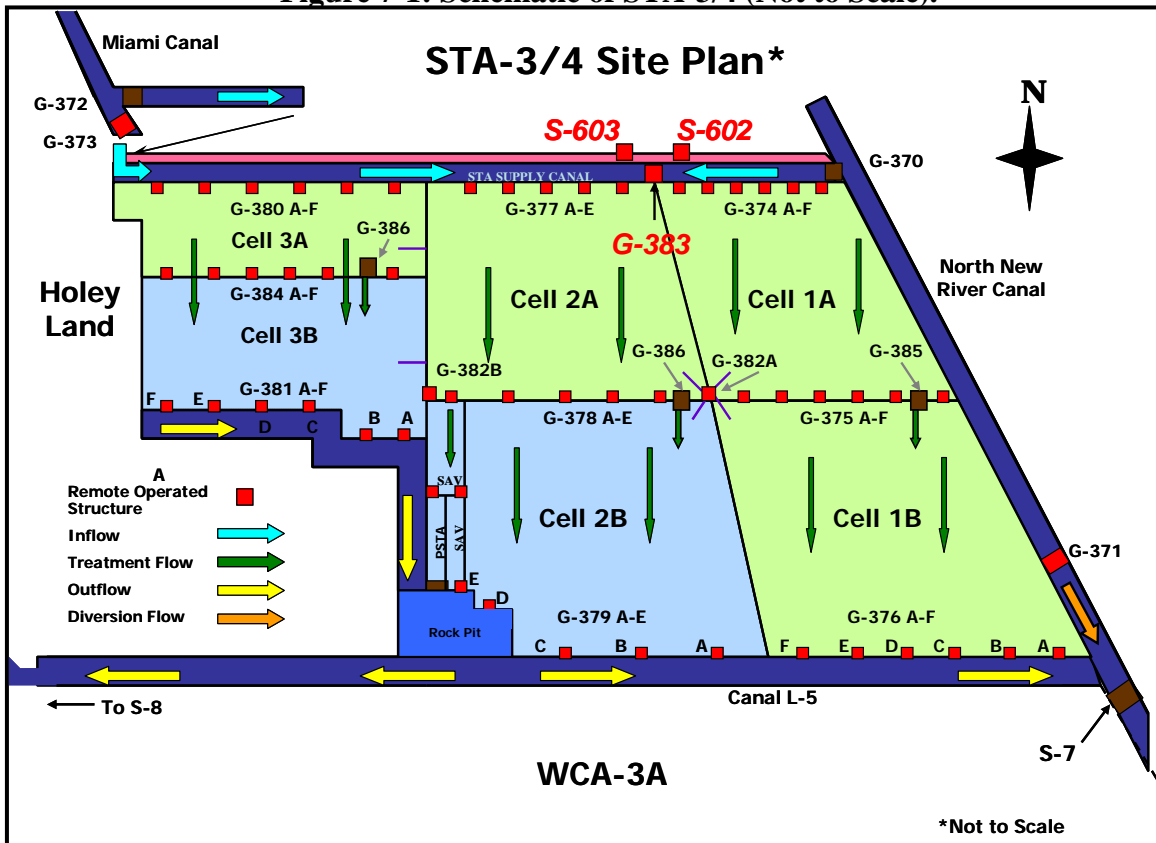
Figure 6-2: Comparison of Inflows and Outflows for EAASR A-1 (Lake TP Concentration of 150 ppb).



7. STA-3/4

A schematic of STA-3/4 is presented in **Figure 7-1B**, showing the two proposed structures (S-602 and S-603) that will allow transfer of water from the EAA Storage Reservoir to the STA.

Figure 7-1: Schematic of STA-3/4 (Not to Scale).



In developing the *Updated STA Inflow Data Sets*, an assumption had to be made of the flows and phosphorus levels from the EAASR A-1 to STA-3/4. After the DMSTA simulation of the reservoir discussed in Section 6 above, the simulated inflow phosphorus levels were increased 2%, and are summarized in **Table 7-1**.

A summary of STA-3/4 phosphorus performance for the 2010 period is presented in the table and figures below. A copy of the DMSTA output is presented in Appendix C. A long-term flow-weighted mean outflow concentration range of 16-24 ppb was forecast, however this includes years when DMSTA forecast levels that fell below the calibration range of 15 ppb for an SAV system, and hence may be optimistic. DMSTA generates various warning and error messages based on the simulation results compared to the calibration data sets; these are displayed in the DMSTA results in Appendix C. For Cells 1A and 2A, the mean flow/width



was about 10% above the range of the emergent calibration data sets. For Cell 3B, the mean flow/width was about 30% below the range of the SAV calibration data sets. For the three SAV cells, the simulated mean depths were slightly below the range of the SAV calibration data sets, although the simulated minimum depths were above 30 cm. Diversion around the STA was simulated to provide water supply to downstream users, including the Big Cypress Basin Seminole Indian Reservation.

Table 7-1: Summary of Long-term Average Annual Inflow to STA-3/4 (Lake TP Conc of 100 ppb).

| Source | Flow (AF/yr) | TP Load (kg/yr) | TP Conc (ppb) |
|---------------|----------------|-----------------|---------------|
| S-7/S-2 Basin | 35,102 | 4,230 | 98 |
| S-8/S-3 Basin | 132,584 | 13,383 | 82 |
| C-139 Basin | 13,201 | 3,401 | 209 |
| SSDD | 10,539 | 1,324 | 102 |
| SFCD | 24,110 | 3,363 | 113 |
| EAA SR | 363,442 | 35,730 | 80 |
| Total | 583,360 | 61,743 | 86 |

Totals are less than the sum of the components due to daily net negative values within the basin.

Table 7-2: Summary of DMSTA Results for STA-3/4 (Lake TP Conc. of 100 ppb).

| Parameter | Unit | STA-3/4 |
|--|-------|---------|
| Effective Treatment Area | acres | 16,543 |
| Average Annual Inflow | | |
| Volume | AF/yr | 583,360 |
| TP Load | kg/yr | 61,743 |
| TP Concentration | ppb | 86 |
| Average Annual Outflow | | |
| Volume | AF/yr | 568,441 |
| Flow-weighted Mean TP Concentration | | |
| Upper Conf. Limit for Settling Rate | ppb | 16 (20) |
| Mean Estimate of Settling Rate | ppb | 20 (16) |
| Lower Conf. Limit for Settling Rate | ppb | 24 (9) |
| Geometric Mean TP Concentration | | |
| Upper Conf. Limit for Settling Rate | ppb | 14 |
| Mean Estimate of Settling Rate | ppb | 16 |
| Lower Conf. Limit for Settling Rate | ppb | 20 |
| TP Load (Using Mean TP Conc.) | kg/yr | 13,853 |
| Diversion Volumes and Loads | | |
| Volume | AF/yr | 64,699 |
| TP Load | kg/yr | 7,897 |
| TP Concentration | ppb | 99 |

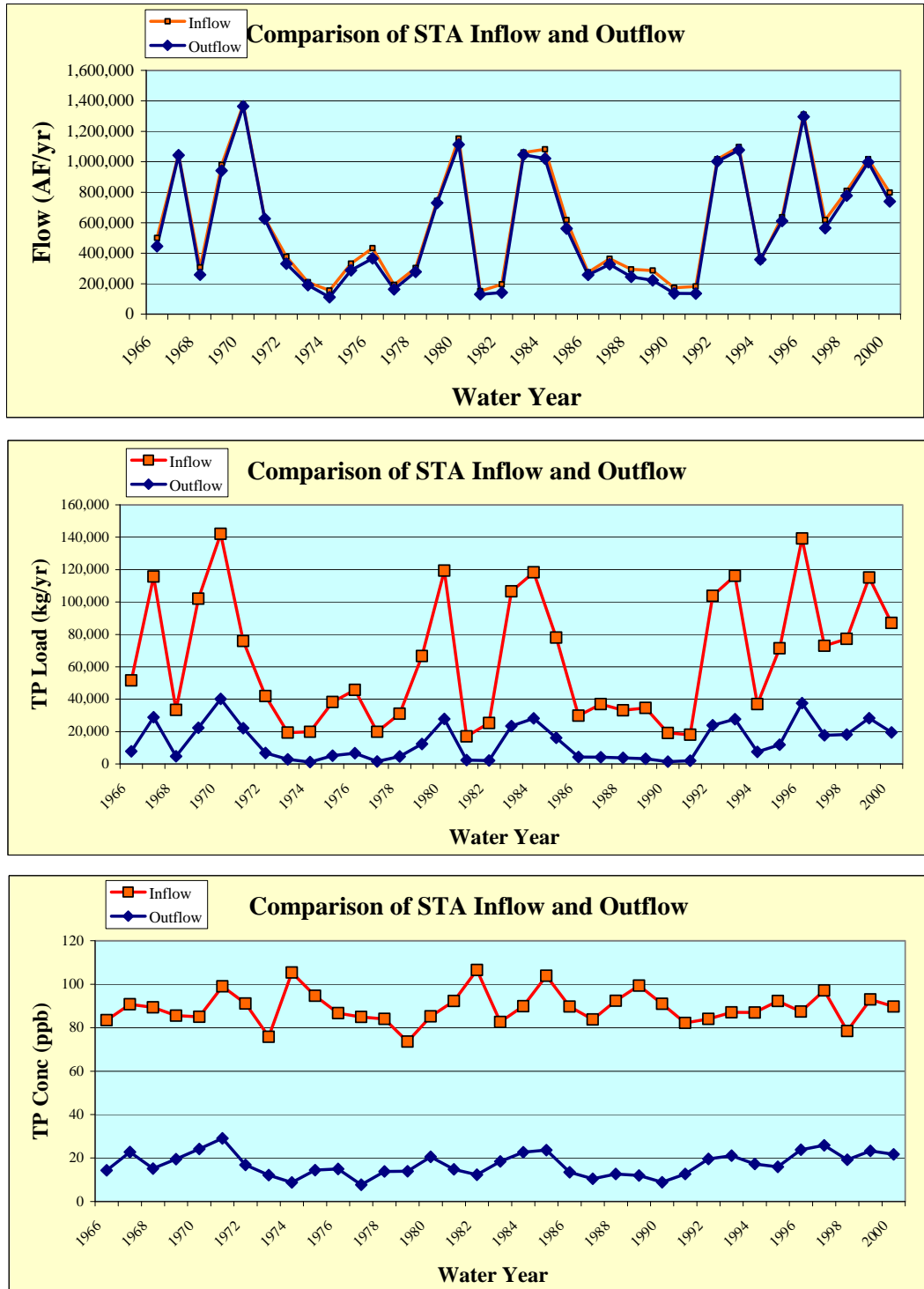
Notes: 1. The DMSTA forecast error for flow-weighted mean concentrations is approximately +/-23% of the expected value. There is associated uncertainty in these predictions and actual performance will vary.

2. Outflow values highlighted in yellow had one or more years of the 36 years simulated below the low end of the calibration data set (enumerated by the number in parentheses). The lowest sustainable STA outflow phosphorus concentration is 19 ppb (STA-3/4), with a standard deviation of 5 ppb.

3. Diversion is for downstream water supply users.



Figure 7-2: Comparison of Inflows and Outflows for STA-3/4 (Lake TP Concentration of 100 ppb).



Note: TP concentrations below 15 ppb have not been sustained.



7.1. Sensitivity Analysis: Lake Okeechobee TP Concentration of 150 ppb

As an analysis of the sensitivity of STA performance to the phosphorus concentration of Lake Okeechobee releases, this section describes the performance of STA-3/4 if the phosphorus concentration of Lake releases is 150 ppb at the inflow to the EAASR. In developing the *Updated STA Inflow Data Sets*, an assumption had to be made of the flows and phosphorus levels from the EAASR A-1 to STA-3/4. After the DMSTA simulation of the reservoir discussed in Section 6.1 above, the inflow phosphorus levels were increased 2% and are summarized in **Table 7-3**.

Table 7-3: Summary of Long-term Average Annual Inflow to STA-3/4 (Lake TP Conc of 150 ppb).

| Source | Flow (AF/yr) | TP Load (kg/yr) | TP Conc (ppb) |
|---------------|----------------|-----------------|---------------|
| S-7/S-2 Basin | 35,102 | 4,230 | 98 |
| S-8/S-3 Basin | 132,584 | 13,383 | 82 |
| C-139 Basin | 13,201 | 3,401 | 209 |
| SSDD | 10,539 | 1,324 | 102 |
| SFCD | 24,110 | 3,363 | 113 |
| EAA SR | 363,442 | 47,791 | 107 |
| Total | 583,360 | 73,804 | 103 |

A summary of STA-3/4 phosphorus performance for the 2010 period is presented in the table and figures below. A copy of the DMSTA output is presented in Appendix C. A long-term flow-weighted mean outflow concentration range of 19-28 ppb was forecast, however this includes years when DMSTA forecast levels that fell below the calibration range of 15 ppb for an SAV system, and hence may be optimistic. These results compare to a simulated range of 16-24 ppb with a concentrations of Lake releases averaging 100 ppb. **The net effect of assuming a Lake Okeechobee phosphorus concentration of 150 ppb appears to be about 3 ppb at the outflow of STA-3/4, resulting in a long-term average annual increase of 2.2 metric tons of phosphorus to the Everglades compared to an average Lake release concentration of 100 ppb.**

DMSTA generates various warning and error messages based on the simulation results compared to the calibration data sets; these are displayed in the DMSTA results in Appendix C. For Cells 1A and 2A, the mean flow/width were about 10% above the range of the emergent calibration data sets. For Cell 3B, the mean flow/width was about 30% below the range of the SAV calibration data sets. For the three SAV cells, the simulated mean depths were slightly below the range of the SAV calibration data sets, although the simulated minimum depths were above 30 cm. Diversion around the STA were simulated to provide water supply to downstream users, including the Big Cypress Basin Seminole Indian Reservation.



Table 7-4: Summary of DMSTA Results for STA-3/4 (Lake Conc. of 150 ppb).

| Parameter | Unit | STA-3/4 |
|--|-------|---------|
| Effective Treatment Area | acres | 16,543 |
| Average Annual Inflow | | |
| Volume | AF/yr | 583,360 |
| TP Load | kg/yr | 73,804 |
| TP Concentration | ppb | 103 |
| Average Annual Outflow | | |
| Volume | AF/yr | 568,441 |
| Flow-weighted Mean TP Concentration | | |
| Upper Conf. Limit for Settling Rate | ppb | 19 (18) |
| Mean Estimate of Settling Rate | ppb | 23 (13) |
| Lower Conf. Limit for Settling Rate | ppb | 28 (6) |
| Geometric Mean TP Concentration | | |
| Upper Conf. Limit for Settling Rate | ppb | 15 |
| Mean Estimate of Settling Rate | ppb | 18 |
| Lower Conf. Limit for Settling Rate | ppb | 22 |
| TP Load (Using Mean TP Conc.) | kg/yr | 16,056 |
| Diversion Volumes and Loads | | |
| Volume | AF/yr | 64,699 |
| TP Load | kg/yr | 11,971 |
| TP Concentration | ppb | 150 |

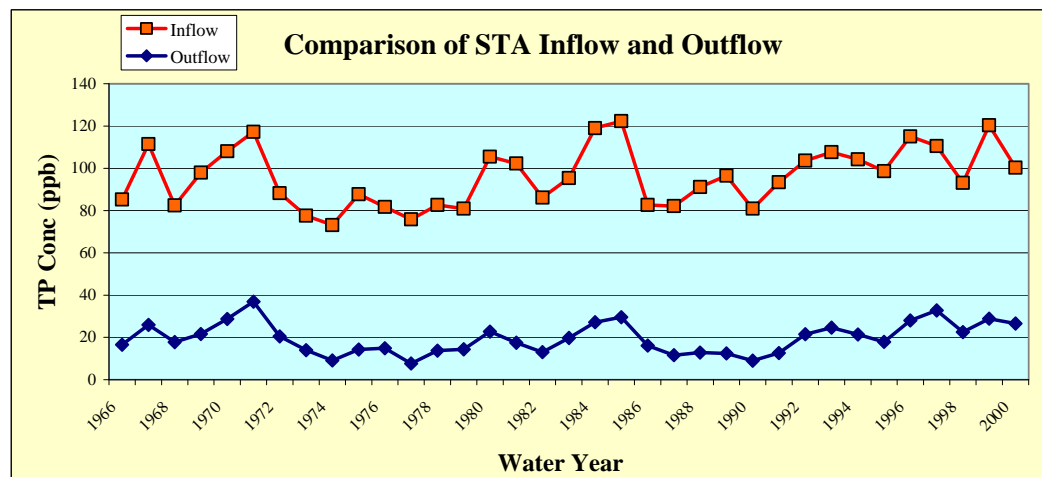
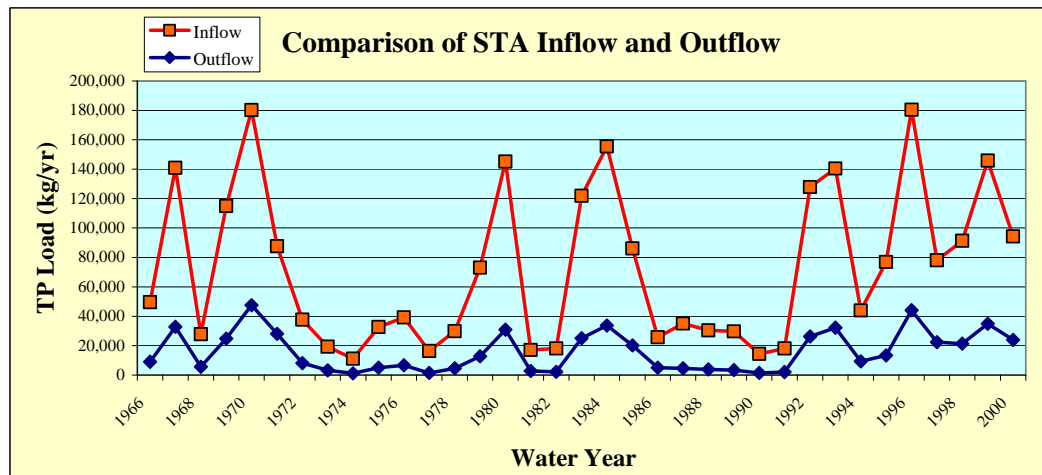
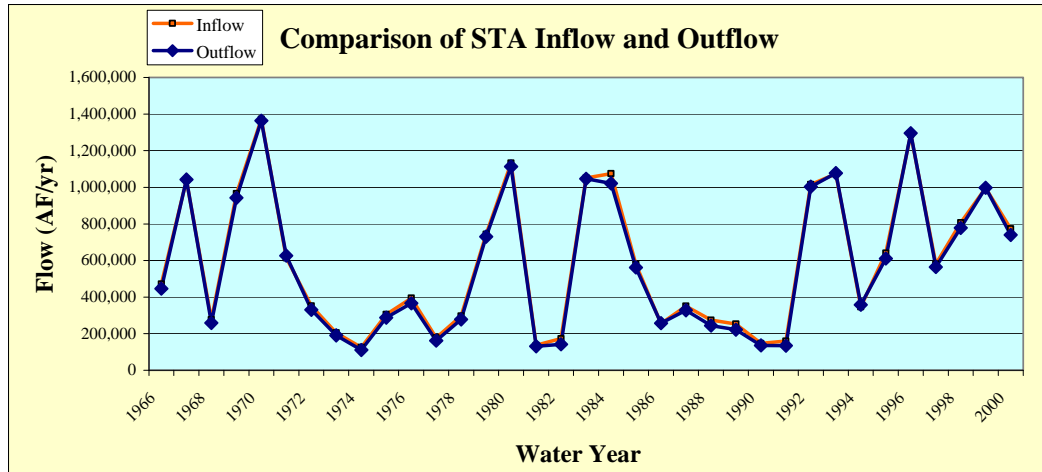
Notes: 1. The DMSTA forecast error for flow-weighted mean concentrations is approximately +/-23% of the expected value. There is associated uncertainty in these predictions and actual performance will vary.

2. Outflow values highlighted in yellow had one or more years of the 36 years simulated below the low end of the calibration data set (enumerated by the number in parentheses). The lowest sustainable STA outflow phosphorus concentration is 19 ppb (STA-3/4), with a standard deviation of 5 ppb.

3. Diversion is for downstream water supply users.



Figure 7-3: Comparison of Inflows and Outflows for STA-3/4 (Lake TP Concentration of 150 ppb).



Note: TP concentrations below 15 ppb have not been sustained.



8. STA-5

Inflow data sets for STA-5 and STA-6 utilized the historic flows and phosphorus loads for the WY1995-2007 period. For the purpose of developing the STA-5 inflow data set, STA-5 was assumed to be comprised of the existing 3 flow-ways of STA-5, along with the 4th and 5th flow-ways of Compartment C that are soon to be constructed (see **Figure 8-1**; URS 2007). The combined C-139 Basin and C-139 Annex runoff will likely be distributed to STA-5 and STA-6 in an attempt to balance the phosphorus loading rate among the flow-ways of the STAs (see **Table 8-1**). A summary of inflows to STA-5 is presented in **Table 8-2**.

Table 8-1: Estimate of Inflow Distribution to Balance the PLR to STA-5 and STA-6.

| Flow-way | Area | TP inflow | Flow at PLR | Load | PLR |
|----------|--------|-----------|-------------|--------|------|
| STA-5 1 | 2,055 | 229 | 28,177 | 7,945 | 0.96 |
| STA-5 2 | 2,055 | 229 | 28,177 | 7,945 | 0.96 |
| STA-5 3 | 1,985 | 229 | 27,217 | 7,674 | 0.96 |
| STA-5 4 | 2,176 | 229 | 29,836 | 8,413 | 0.96 |
| STA-5 5 | 2,669 | 229 | 36,595 | 10,319 | 0.96 |
| STA-6 3 | 1,857 | 178 | 32,692 | 7,180 | 0.96 |
| STA-6 5 | 652 | 97 | 20,970 | 2,521 | 0.96 |
| STA-6 2 | 245 | 97 | 7,880 | 947 | 0.96 |
| Total | 13,694 | 203 | 211,544 | 52,944 | 0.96 |

At this time, it is extremely difficult to forecast the phosphorus removal performance of STA-5. Since the first full water year of operation (2001), the annual inflow concentrations have ranged from 165 ppb to 299 ppb, with a 7-yr average of 235 ppb (Pietro et al. 2007). The phosphorus loading rate has been considerably higher than the other STAs, ranging from 0.94 g/m²/yr to 4.01 g/m²/yr with a 7-yr average of 2.1 g/m²/yr. The long-term outflow concentration has ranged from 82-192 ppb, with a 7-yr average of 105 ppb. These characteristics are more similar to the DMSTA emergent vegetation calibration data set than to the SAV data set, which may be related to different soil type than in the EAA STAs (more mineral content) and less calcium in the inflow waters than the EAA STAs. In consideration of the anticipated reduced phosphorus loading rate when the additional treatment cells within Compartment C begin operation, the 2005 Study estimated the performance of STA-5 using the average performance of two DMSTA simulations:

1. Assuming the phosphorus removal downstream cell in each flow-way performs similar to the emergent calibration data set; and
2. Assuming the phosphorus removal downstream cell in each flow-way performs similar to the SAV calibration data set.

For comparison purposes, a similar set of simulations were conducted during the present analysis, *however, until such time that the STA-5 performance improves, and until performance data for the newly constructed Flow-way 3 is available, the forecast performance will be based on assuming the phosphorus removal of the downstream cell in each flow-way performs similar to the emergent calibration data set.*



Figure 8-1: Preliminary Layout of Compartment C Build-out; Subject to Revision (modified from URS 2007).

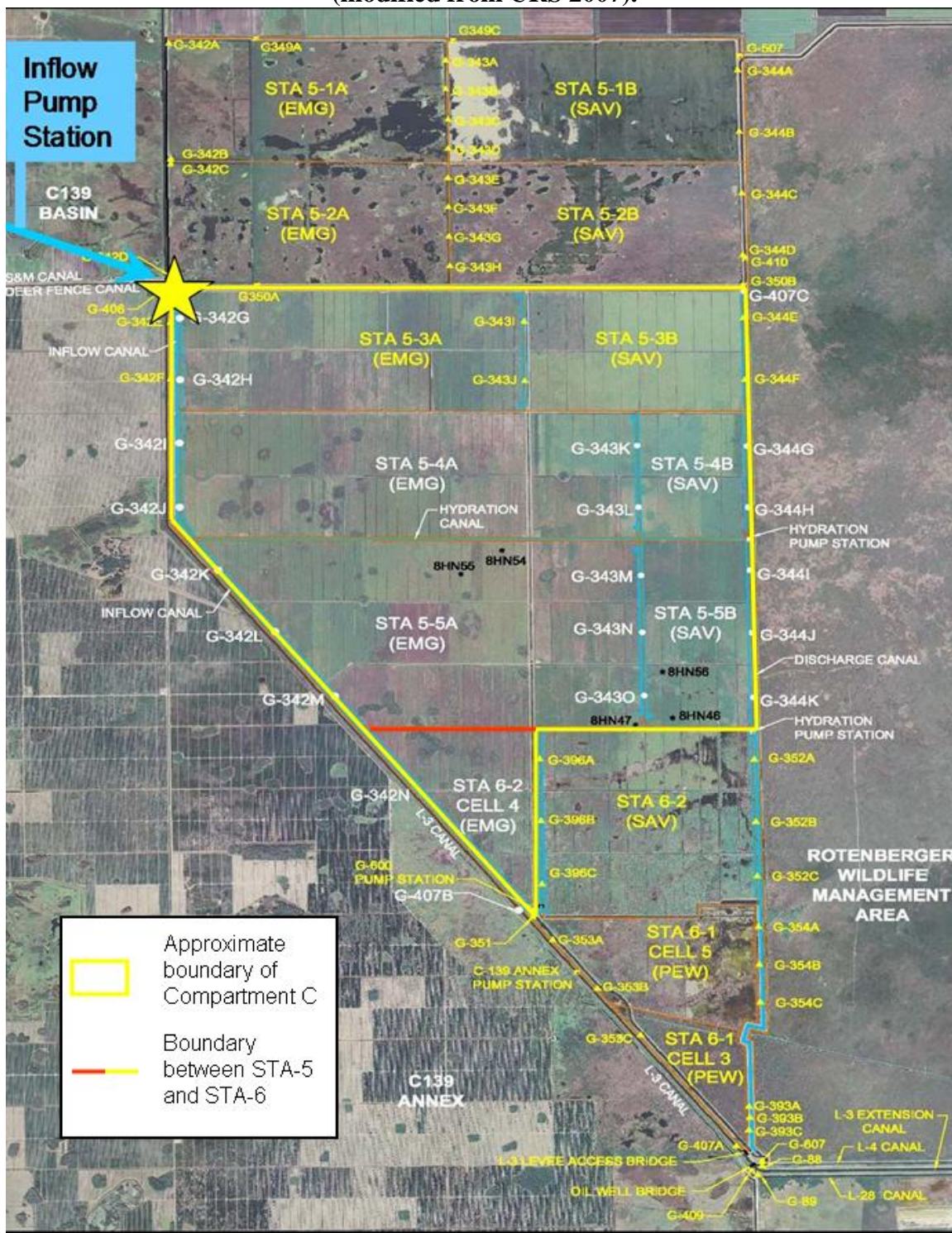


Table 8-2: Summary of Long-term Average Annual Inflow to STA-5, Including Compartment C Build-out.

| Source | Flow (AF/yr) | TP Load (kg/yr) | TP Conc (ppb) |
|-------------|--------------|-----------------|---------------|
| C-139 Basin | 150,001 | 42,300 | 229 |

A summary of STA-5 phosphorus performance for the 2010 period is presented in the tables and figures below. Copies of the DMSTA output are presented in Appendix C. For the emergent vegetation scenario, a long-term flow-weighted mean outflow concentration range of 22-47 ppb was forecast, however this includes six of the thirteen years when DMSTA forecast levels that fell below the calibration range of 20 ppb for an emergent system; hence this range may be optimistic, particularly in light of STA-5 performance history. Based on the historic performance of STA-5 and the high degree of uncertainty as to whether these DMSTA forecasts accurately represent actual phosphorus removal within the STAs in Compartment C, it is premature to conclude that there is excess treatment capacity in Compartment C. For the SAV scenario, *presented only for comparison and not as a forecast of STA-5 performance*, DMSTA forecast long-term outflow concentrations below the low end of the SAV calibration data set (15 ppb) and these were set to the low end value; hence this range is overly optimistic particularly in light of STA-5 performance history. DMSTA generates various warning and error messages based on the simulation results compared to the calibration data sets; these are displayed in the DMSTA results in Appendix C. For the emergent vegetation scenario no warnings messages were generated. However, DMSTA forecast 7% of the time the depth in the upstream cells was less than 10 cm, indicating the potential for insufficient water availability to maintain the minimum desirable water depth of 15 cm. For the SAV scenario, thirteen messages were generated, identifying that mean depths, flow/widths and outflow phosphorus concentrations were below the range of the SAV calibration data sets. In addition, DMSTA forecast 7% of the time the depth in the upstream cells was less than 10 cm, indicating the potential for insufficient water availability to maintain the minimum desirable water depth of 15 cm.



Table 8-3: Summary of DMSTA Results for STA-5, Including Comp. C Build-out.

| Parameter | Unit | STA-5 - Emergent | STA-5 - SAV |
|--|-------|------------------|-------------|
| Effective Treatment Area | acres | 10,940 | 10,940 |
| Average Annual Inflow | | | |
| Volume | AF/yr | 150,001 | 150,001 |
| TP Load | kg/yr | 42,300 | 42,300 |
| TP Concentration | ppb | 229 | 229 |
| Average Annual Outflow | | | |
| Volume | AF/yr | 148,717 | 148,717 |
| Flow-weighted Mean TP Concentration | | | |
| Upper Conf. Limit for Settling Rate | ppb | 22 (6) | 15 (13) |
| Mean Estimate of Settling Rate | ppb | 32 | 15 (13) |
| Lower Conf. Limit for Settling Rate | ppb | 47 | 19 (11) |
| Geometric Mean TP Concentration | | | |
| Upper Conf. Limit for Settling Rate | ppb | 20 | 15 |
| Mean Estimate of Settling Rate | ppb | 30 | 15 |
| Lower Conf. Limit for Settling Rate | ppb | 44 | 17 |
| TP Load (Using Mean TP Conc.) | kg/yr | 5,866 | 2,752 |
| Diversion Volumes and Loads | | | |
| Volume | AF/yr | 0 | 0 |
| TP Load | kg/yr | 0 | 0 |
| TP Concentration | ppb | - | - |

Notes: 1. The DMSTA forecast error for flow-weighted mean concentrations is approximately +/-23% of the expected value.

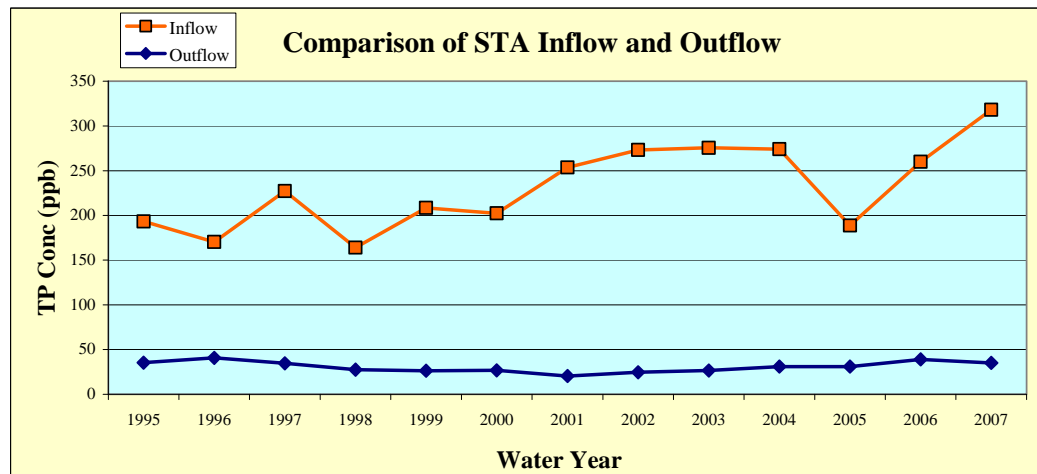
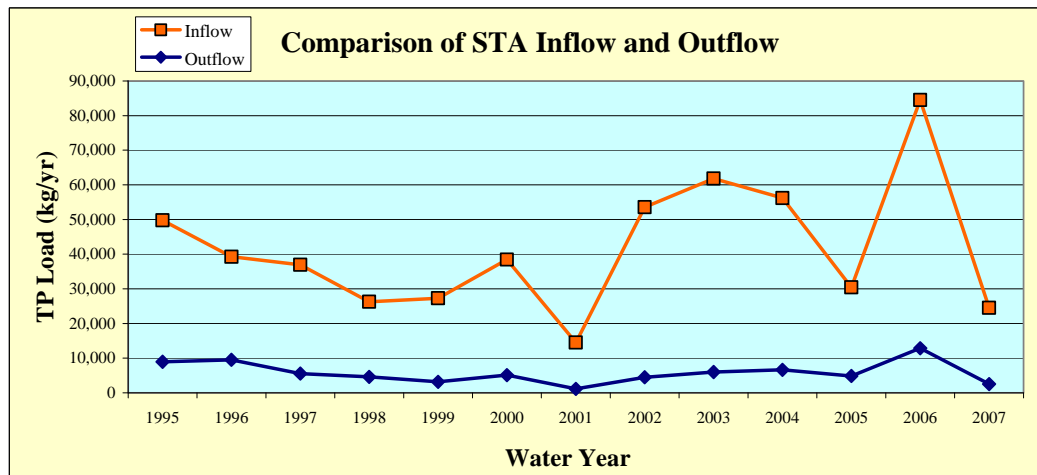
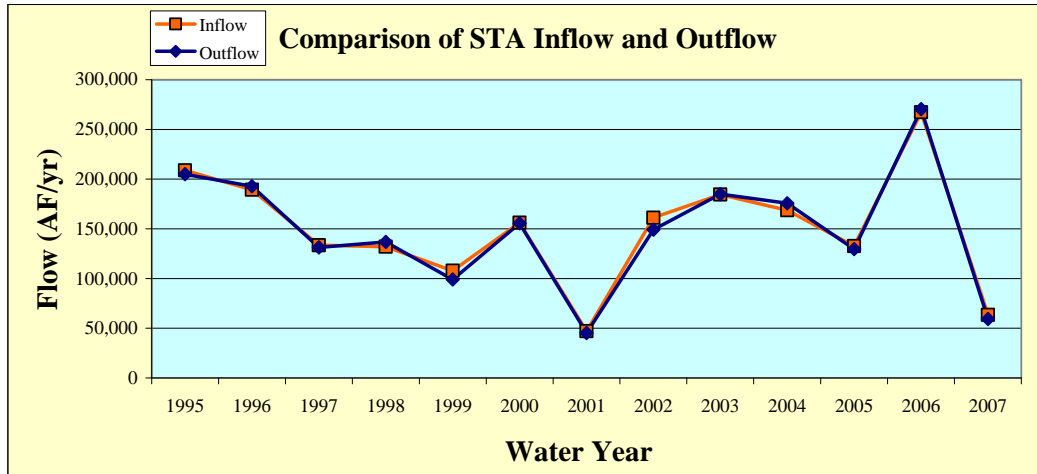
There is associated uncertainty in these predictions and actual performance will vary.

2. Outflow values highlighted in yellow had one or more years of the 13 years simulated below the low end of the calibration data set (enumerated by the number in parentheses).

3. Results for SAV in the downstream cells are presented for comparison only; the emergent vegetation scenario is currently used as a forecast for STA-5



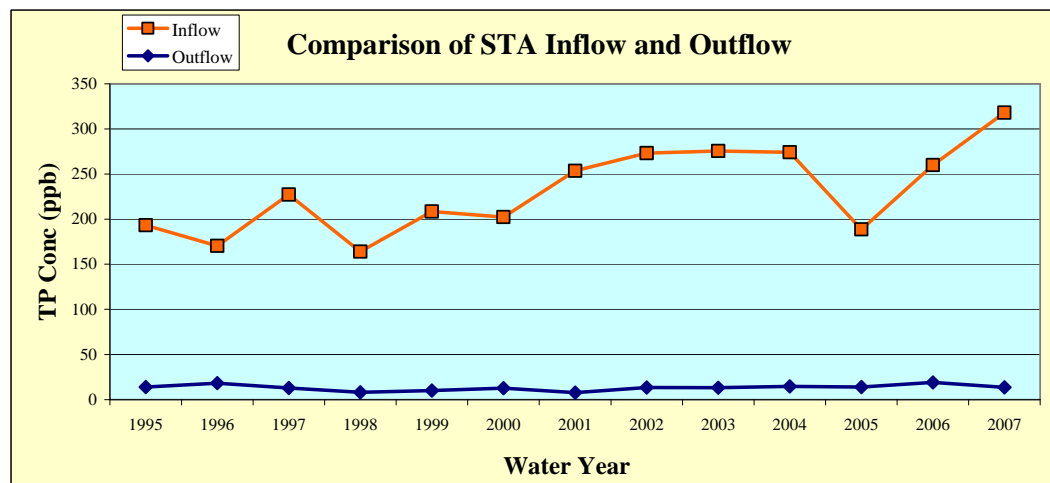
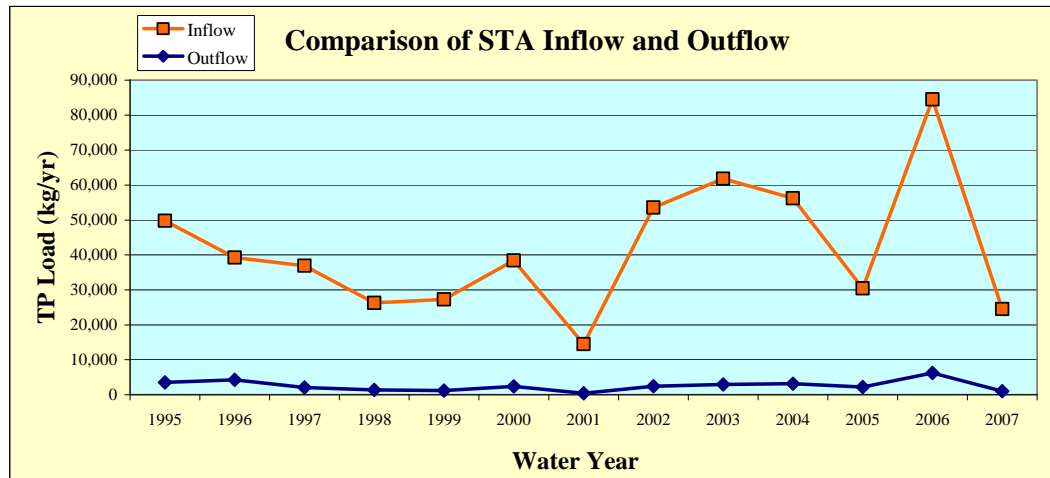
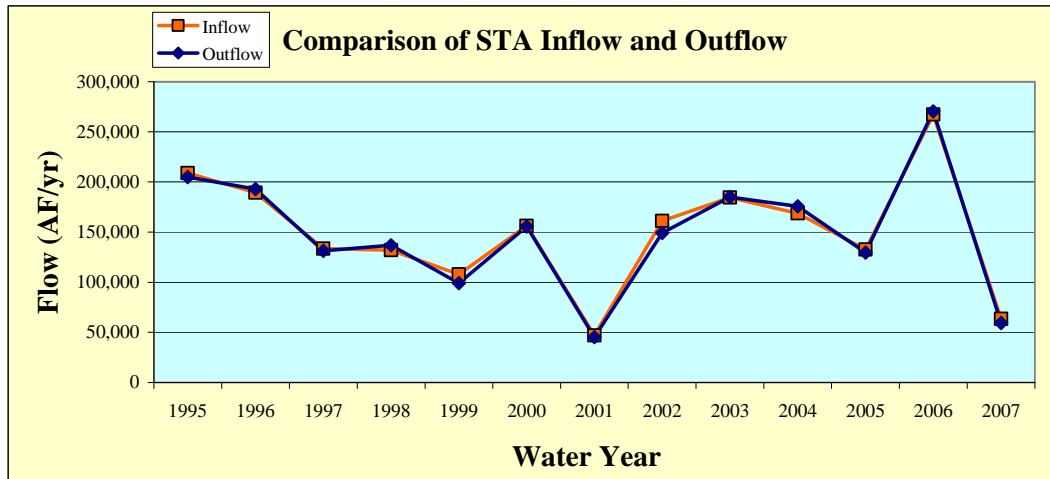
Figure 8-2: Comparison of Inflows and Outflows for STA-5 With Emergent Vegetation in All Cells, Including Compartment C Build-out.



Note: TP concentrations below 15 ppb have not been sustained.



Figure 8-3: Summary of DMSTA Results for STA-5 With SAV in Downstream Cells.



Note: TP concentrations below 15 ppb have not been sustained.



9. STA-6

The combined C-139 Basin and C-139 Annex runoff will be distributed to STA-5 and STA-6 to balance the phosphorus loading rate among the flow-ways of the STAs (see **Table 8-1** above). A summary of the long-term average inflows to STA-6 is presented in **Table 9-1**.

Table 9-1: Summary of Long-term Average Annual Inflow to STA-6, Including Compartment C Build-out.

| Source | Flow (AF/yr) | TP Load (kg/yr) | TP Conc (ppb) |
|--------------|---------------|-----------------|---------------|
| C-139 Basin | 20,062 | 5,657 | 229 |
| C-139 Annex | 41,480 | 4,987 | 97 |
| Total | 61,542 | 10,644 | 140 |

A summary of STA-6 phosphorus performance for the 2010 period is presented in the table and figures below. A copy of the DMSTA output is presented in Appendix C. DMSTA forecast a best case scenario long-term flow-weighted mean phosphorus concentration of 10 ppb which is below the minimum of the calibration data sets for the vegetation in the treatment cells, and that forecast was replaced in Table 9-2 by 13 ppb¹. The adjusted outflow phosphorus levels in Table 9-2 may still portray optimistic results in that the best performing STA (STA-3/4) is presently averaging about 19 ppb, with a 5 ppb standard deviation on annual values. Although the simulated performance appears to suggest additional treatment capacity may be available in the 2,754 acres of STA-6, based on the historic performance of STA-5 and the high degree of uncertainty as to whether these DMSTA forecasts accurately represent actual phosphorus removal within the STAs in Compartment C, it is premature to conclude that there is excess treatment capacity in Compartment C. DMSTA generates various warning and error messages based on the simulation results compared to the calibration data sets; these are displayed in the DMSTA results in Appendix C. Eight messages were generated, identifying that mean depths, flow/widths and outflow phosphorus concentrations were below the range of the calibration data sets. In addition, the mean inflow concentration (140 ppb) was above the calibration range for the Prior Existing Wetland (PEW) data sets, further suggesting that the forecast outflow concentration may be optimistic.

¹ For STA-6, two flow-ways have SAV in the downstream cell which has a minimum calibration concentration of 15 ppb, and one has Prior Existing Wetlands (PEW) which has a minimum calibration concentration of 15 ppb. The flow-weighted minimum concentration for the three flow-ways is 13 ppb.



Table 9-2: Summary of DMSTA Results for STA-6, Including Comp. C Build-out.

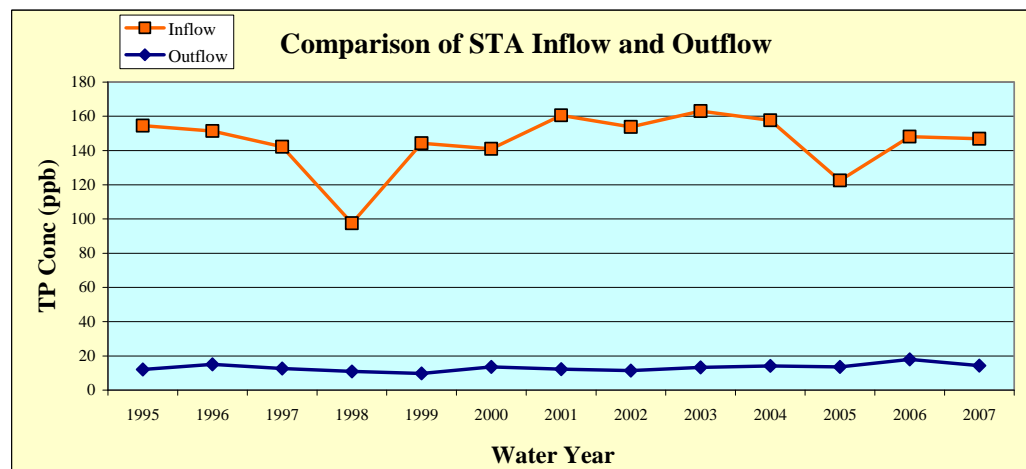
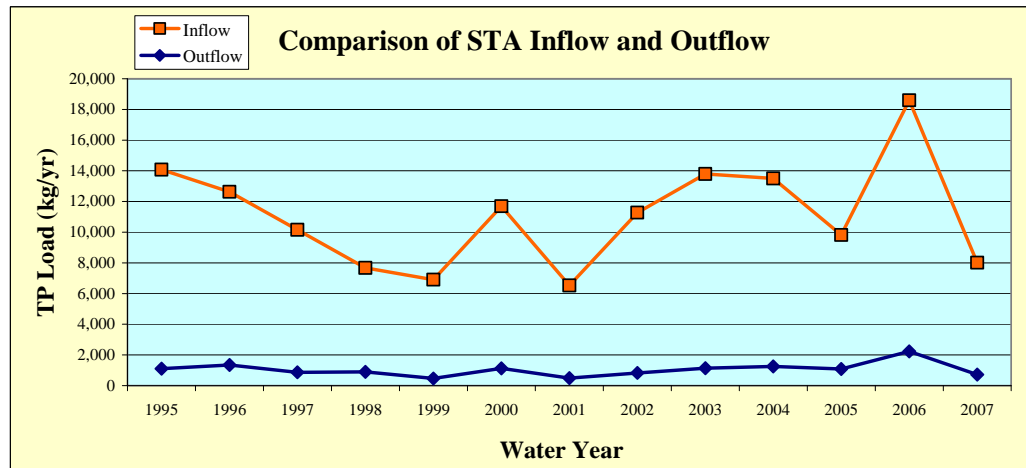
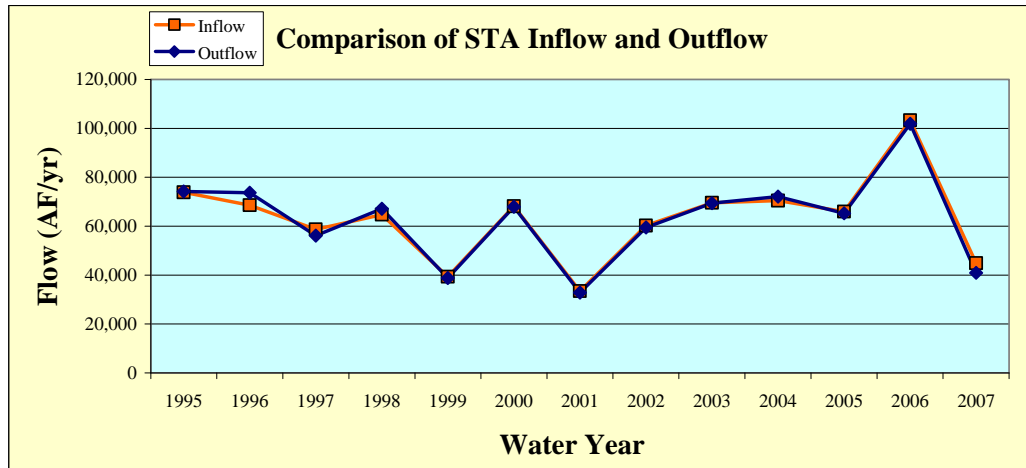
| Parameter | Unit | STA-6 |
|--|-------|---------|
| Effective Treatment Area | acres | 2,754 |
| Average Annual Inflow | | |
| Volume | AF/yr | 61,542 |
| TP Load | kg/yr | 10,644 |
| TP Concentration | ppb | 140 |
| Average Annual Outflow | | |
| Volume | AF/yr | 61,494 |
| Flow-weighted Mean TP Concentration | | |
| Upper Conf. Limit for Settling Rate | ppb | 13 (11) |
| Mean Estimate of Settling Rate | ppb | 13 (7) |
| Lower Conf. Limit for Settling Rate | ppb | 17 (1) |
| Geometric Mean TP Concentration | | |
| Upper Conf. Limit for Settling Rate | ppb | 15 |
| Mean Estimate of Settling Rate | ppb | 15 |
| Lower Conf. Limit for Settling Rate | ppb | 16 |
| TP Load (Using Mean TP Conc.) | kg/yr | 986 |
| Diversion Volumes and Loads | | |
| Volume | AF/yr | 0 |
| TP Load | kg/yr | 0 |
| TP Concentration | ppb | - |

Notes: 1. The DMSTA forecast error for flow-weighted mean concentrations is approximately +/-23% of the expected value. There is associated uncertainty in these predictions and actual performance will vary.

2. Outflow values highlighted in yellow had one or more years of the 13 years simulated below the low end of the calibration data set (enumerated by the number in parentheses). The lowest sustainable STA outflow phosphorus concentration is 19 ppb (STA-3/4), with a standard deviation of 5 ppb.



Figure 9-1: Comparison of Inflows and Outflows for STA-6.



Note: TP concentrations below 15 ppb have not been sustained.



10. Summary

A summary of the simulated phosphorus removal performance in the STAs, the EAASR A-1 and the additional treatment areas in Compartments B and C is presented in **Table 10-1** below². For Compartment B, DMSTA forecast long-term average annual phosphorus concentrations of 9.1-14.6 ppb, which are below the minimum of the calibration data sets for SAV (15 ppb), and those forecasts were replaced in Table 10-1 by 15 ppb. Similarly for STA-6, DMSTA forecast a best case scenario long-term flow-weighted mean phosphorus concentration of 10 ppb which is below the minimum of the calibration data sets for the vegetation in the treatment cells, and that forecast was replaced in Table 10-1 by 13 ppb³. The adjusted outflow phosphorus levels in Table 10-1 may still portray optimistic results in that the best performing STA (STA-3/4) is presently averaging about 19 ppb, with a 5 ppb standard deviation on annual values. Simulated long-term average annual outflow concentrations from the individual STAs and additional treatment areas in Compartments B and C ranged from 13-47 ppb. On a cumulative basis, the simulated long-term average annual concentration ranged from 18-28 ppb. In consideration of the forecast error of $\pm 23\%$, this suggests a potential long-term range of 15-34 ppb for the cumulative long-term average annual outflow concentration. The simulated excellent performance of Compartment B (long-term outflow concentrations of 15 ppb) suggests that excess treatment capacity is available in the treatment areas of Compartment B.

In addition, two sensitivity analyses were evaluated, and the results are summarized below.

STA-2 Inflow Re-direction. Table 10-1 presents results for re-direction of a portion of STA-2 inflows to Compartment B North Build-out sufficient to achieve a balanced long-term phosphorus loading rate (PLR) to STA-2 and the North Build-out of $1.0 \text{ g/m}^2/\text{yr}$. An alternative simulation was conducted with an STA-2 PLR of $1.3 \text{ g/m}^2/\text{yr}$, yielding a predicted long-term outflow concentration range for STA-2 of 19-29 ppb compared to 19-27 ppb for a PLR of $1.0 \text{ g/m}^2/\text{yr}$. The simulated results for Compartment B were essentially the same under both loading scenarios – DMSTA forecast phosphorus concentrations below the lower end of the calibration data sets (15 ppb). **The simulation of PLRs of $1.0 \text{ g/m}^2/\text{yr}$ and $1.3 \text{ g/m}^2/\text{yr}$ was conducted for the purpose of comparing alternatives and does not represent ultimate PLR goals for STAs.** The long-term PLR is just one of many factors that influence the phosphorus removal performance of an STA; others include vegetation type, soil type, antecedent land use, phosphorus loading history, inflow concentrations, hurricanes, droughts and other disturbances.

² These simulated forecasts of STA performance are made for the comparison of alternatives and not for the development of effluent limits. Effluent limits are determined through the State of Florida's issuance of permits for these facilities.

³ For STA-6, two flow-ways have SAV in the downstream cell which has a minimum calibration concentration of 15 ppb, and one has Prior Existing Wetlands (PEW) which has a minimum calibration concentration of 15 ppb. The flow-weighted minimum concentration for the three flow-ways is 13 ppb.



Table 10-1: Summary of DMSTA Modeling Results.

| Parameter | Unit | STA-1E | STA-1W | STA-2 | Comp. B North | Comp. B South | Comp. B Combined | EAAASR A-1 Note 3 | STA-3/4 | STA-5 - Emergent | STA-6 | All |
|---|-------|---------|---------|---------|---------------|---------------|------------------|-------------------|---------|------------------|---------|-----------|
| Effective Treatment Area | acres | 5,132 | 6,670 | 6,338 | 5,824 | 2,796 | 8,620 | 15,200 | 16,543 | 10,940 | 2,754 | 56,997 |
| Average Annual Inflow | | | | | | | | | | | | |
| Volume | AF/yr | 193,818 | 243,172 | 182,697 | 177,228 | 47,651 | 224,879 | 540,013 | 219,918 | 150,001 | 61,542 | 1,639,469 |
| TP Load | kg/yr | 41,864 | 54,409 | 25,662 | 22,932 | 5,553 | 28,833 | 63,885 | 26,013 | 42,300 | 10,644 | 265,107 |
| TP Concentration | ppb | 175 | 181 | 114 | 105 | 94 | 104 | 96 | 96 | 229 | 140 | 131 |
| Average Annual Outflow | | | | | | | | | | | | |
| Volume | AF/yr | 190,599 | 244,928 | 186,047 | 180,541 | 53,378 | 233,919 | 363,442 | 568,441 | 148,717 | 61,494 | 1,634,145 |
| Flow-weighted Mean TP Concentration | | | | | | | | | | | | |
| Upper Conf. Limit for Settling Rate | ppb | 20 (10) | 21 (4) | 19 (2) | 15 (35) | 15 (35) | 15 (35) | - | 16 (20) | 22 (6) | 13 (11) | 18 |
| Mean Estimate of Settling Rate | ppb | 27 (1) | 27 | 22 (1) | 15 (35) | 15 (34) | 15 (35) | 80 | 20 (16) | 32 | 13 (7) | 22 |
| Lower Conf. Limit for Settling Rate | ppb | 35 | 35 | 27 | 15 (27) | 15 (25) | 15 (27) | - | 24 (9) | 47 | 17 (1) | 28 |
| Geometric Mean TP Concentration | | | | | | | | | | | | |
| Upper Conf. Limit for Settling Rate | ppb | 18 | 20 | 17 | 15 | 15 | 15 | - | 14 | 20 | 15 | - |
| Mean Estimate of Settling Rate | ppb | 24 | 26 | 21 | 15 | 15 | 15 | 93 | 16 | 30 | 15 | - |
| Lower Conf. Limit for Settling Rate | ppb | 32 | 33 | 25 | 15 | 15 | 15 | - | 20 | 44 | 16 | - |
| TP Load (Using Mean TP Conc.) | kg/yr | 6,240 | 8,222 | 5,145 | 3,340 | 988 | 4,328 | 35,730 | 13,853 | 4,677 | 986 | 43,451 |
| Diversion Volumes and Loads (See Note 4) | | | | | | | | | | | | |
| Volume | AF/yr | 26,186 | 0 | 0 | 0 | 0 | 0 | 0 | 64,699 | 0 | 0 | 90,885 |
| TP Load | kg/yr | 4,027 | 0 | 0 | 0 | 0 | 0 | 0 | 7,897 | 0 | 0 | 11,924 |
| TP Concentration | ppb | 125 | - | - | - | - | - | - | 99 | - | - | 106 |

Notes: 1. The DMSTA forecast error for flow-weighted mean concentrations is approximately +/-23% of the expected value. There is associated uncertainty in these predictions and actual performance will vary.

2. Outflow values highlighted in yellow had one or more years below the low end of the calibration data set (enumerated by the number in parentheses). For all areas except STA-5 and STA-6, a total of 35 water years (WY66-00) were simulated; for STA-5 and STA-6 a total of 13 water years (WY95-07) were simulated. The lowest sustainable STA outflow phosphorus concentration is 19 ppb (STA-3/4), with a standard deviation of 5 ppb.

3. Outflows shown from EAAASR are outflows to STA-3/4; total outflows, including irrigation releases, were 514,580 AF/yr and 48,699 kg/yr at 77 ppb.

4. Diversion around STA-1E is directed to C-5IE. Diversion around STA-3/4 is for downstream water supply users. While the SFWMM simulated small diversion volumes of EAA runoff, for the purpose of estimating phosphorus removal performance, these were simulated by DMSTA as being captured in the STAs. Diversions may occur based on the daily hydraulic and treatment capacity of the treatment areas.



As a reference, STA-2 is presently receiving a PLR of 1.34 g/m²/yr and is one of the highest performing STAs, with a long-term average flow-weighted mean of 21 ppb (Pietro et al. 2007). In addition, STA-3/4 has been receiving phosphorus at a PLR averaging 1.5 g/m²/yr and is producing the lowest outflow phosphorus concentration of all the STAs, with a 3-year average flow-weighted mean of 19 ppb (Pietro et al. 2007). These STA performance results, in combination with the results of this sensitivity analysis, suggest that STAs loaded above a PLR 1.3 g/m²/yr may still achieve optimal performance, which could allow a greater loading to STA-2 than presently modeled, thus increasing the available treatment capacity in Compartment B.

Phosphorus Concentrations for Lake Okeechobee Releases. Table 10-1 presents DMSTA modeling results for the EAASR A-1 and STA-3/4 using an average phosphorus concentration for Lake Okeechobee releases of 100 ppb, which corresponds to recent levels. An alternative simulation was conducted assuming an average phosphorus concentration for Lake Okeechobee releases of 150 ppb. While this assumed 50% increase in concentration increased the simulated phosphorus load to the EAASR A-1 by a long-term average of 20 metric tons/yr, the simulated net increase to STA-3/4 was just over 12 metric tons/yr. The 8 metric ton/yr balance either accumulated in the reservoir storage or was discharged to the EAA to satisfy irrigation demand. DMSTA simulated an increase in the long-term STA-3/4 outflow concentration as about 3 ppb, resulting in a predicted long-term average annual increase of about 2.2 metric tons/yr of phosphorus to the Everglades compared to the simulations that used an average Lake release concentration of 100 ppb.

Relationship to EAA Regional Feasibility Study. A direct comparison between the STA performance estimates of this present analysis and the performance estimates presented in the 2005 EAA Regional Feasibility Study is not recommended for the reasons described in the following sections.

EAA Basin. Unlike the 2005 Study, the SFWMM simulation used in this analysis did not include the completion of the EAA Conveyance and Regional Treatment project (ECART), resulting in approximately 93,000 AF/yr more flow to STA-1W and a similar reduction in flow to Compartment B. In addition, the simulated long-term average annual runoff volume to the STAs and EAASR from the entire EAA increased approximately 40,800 AF/yr compared to the 2005 Study as a result of using the latest version of the SFWMM for the current analyses. Additionally, there is marked variation in the EAA sub-basin runoff volumes. For the S-5A sub-basin, the difference (a long-term increase of approximately 17,000 AF/yr) is attributable to variations in the SFWMM results. Some of the EAA-wide variations are attributable to differences in the assumptions used in the SFWMM, e.g.,

1. Land use:
 - a. 2005: All land use has been updated using most recent FLUCCS data (1995), modified in the Lower East Coast urban areas using 2000 aerial photography (2x2 scale).
 - b. 2007: The land use coverage is intermediate between 2000B3 and 2050B3
2. Miami Canal Basin



- a. 2005: EAA cells in the Miami Canal Basin between STA5 and STA6 are not production cells (shrub Land Use). Then, no irrigation demands are required in this area. Runoff from this area is part of the Miami Canal Basin.
 - b. 2007: no such assumption
3. STA Sizes
- a. 2005: Compartment B = 7,575 ac; Compartment C = 7,571 ac
 - b. 2007: Compartment B = 6,722 ac; Compartment C = 6,230 ac
4. CERP
- a. 2005: L-8 reservoir (rock pit located in S-5A Basin) 870 ac 2 ft deep
 - b. 2007: L-8 reservoir 870 ac 44 ft deep

For the S-7/S-2 and S-6/S-2 sub-basins, in addition to approximately 10,300 AF/yr less flows to the STAs and EAASR compared to this analysis as a result in variations in the SFWMM results, the 2005 Study adjusted the simulated runoff from the S-6/S-2 and S-7/S-2 basins to create a uniform average runoff depth for both basins (ADA/B&M 2005). However, for this current analysis, District modeling staff recommended no adjustment of the SFWMM results.

Western Basins. The estimated runoff volume from the C-139 Basin and C-139 Annex increased approximately 12,400 AF/yr compared to the 2005 Study estimates, due primarily to the apparent omission of a portion of the L-2/L-3 Canal flows in the 2005 Study. In addition, during the 2005 Study, a 10% reduction in phosphorus concentration was incorporated to reflect a then-promising trend in basin BMP effectiveness; for the current analysis, District staff recommended no adjustment to the period of record data for BMP implementation. A final difference is that the 2005 Study included STA-6 Section 2 and the upstream cell as part of STA-5, whereas this analysis modeled these cells as part of STA-6.

Eastern Basins. The 2005 Study assumed 100% of the simulated L-8 Basin runoff that was re-directed to the C-51 Canal through S-5AE was not captured in STA-1E; the present analysis assumed only 75% diversion of this L-8 Basin runoff, based on the best professional judgment of actual operating experience, resulting in an increase of approximately 8,600 AF/yr compared to the 2005 Study. The difference in SFWMM version accounted for approximately 6,000 AF/yr less C-51W Basin runoff to STA-1E in this analysis, however, the increase in S-5A Basin runoff to the STA-1 Inflow Basin due to the absence of ECART resulted in a net increase of approximately 22,000 AF/yr to STA-1E compared to the estimate in the 2005 Study.

Lake Okeechobee. The SFWMM simulated a considerably different magnitude and distribution of Lake Okeechobee releases for the current analyses than were simulated for the 2005 Study. Overall, a long-term average of approximately 39,000 AF/yr less Lake releases were simulated in the 2007 SFWMM Alt1 than in the 2005 simulation. The simulated quantity of water supply releases that bypasses the STAs decreased by almost 50,000 AF/yr to a long-term average annual volume of 85,753 AF/yr. Conversely, the quantity of Lake releases that were captured in the EAA Storage Reservoir increased by a long-term average of almost 40,000 AF/yr compared to the 2005 Study.



11. References

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Appendix A. SFWMM Model Assumptions

2010BCalt1_WMM5.5.2.1_082307v2_out

| Feature | 2010 Base Condition Assumptions | 2010BCalt1 Proposed Action |
|-------------------|---|----------------------------|
| Climate | <ul style="list-style-type: none"> • The climatic period of record is from 1965 to 2000. • Rainfall estimates have been revised and updated for 1965-2000. • Revised evapotranspiration methods have been used for 1965-2000. | Same as 2010BS |
| Topography | <p>Updated November 2001 and September 2003 using latest available information (in NGVD 29 datum).</p> <p>Nov 2001 update (Documented in November 2001 SFWMD memorandum from M. Hinton to K. Tarboton) includes:</p> <ul style="list-style-type: none"> • USGS High Accuracy Elevation data from helicopter surveys collected 1999-2000 for Everglades National Park and Water Conservation Area (WCA) 3 south of Alligator Alley • USGS Lidar data (May 1999) for WCA-3A north of Alligator Alley • Lindahl, Browning, Ferrari & Helstrom 1999 survey for Rotenberger Wildlife Management Area. • Stormwater Treatment Area surveys from 1990s • Aerometric Corp. 1986 survey of the 8-1/2 square mile area • Includes estimate of Everglades Agricultural Area subsidence • Other data as in SFWMM v3.7 • FWC survey 1992 for the Holey Land Wildlife Management Area. <p>September 2003 update includes:</p> <ul style="list-style-type: none"> • Reverting to FWC 1992 survey data for Rotenberger Wildlife Management Area. | Same as 2010BS |



| Feature | 2010 Base Condition Assumptions | 2010BCAlt1 Proposed Action |
|---|---|----------------------------|
| | <ul style="list-style-type: none"> DHI gridded data from Kimley –Horn contracted survey of EAA, 2002-2003. Regridded to 2x2 scale for EAA outside of STAs and WMAs. | |
| Sea Level | Sea level data from six long-term NOAA stations were used to generate a historic record to use as sea level boundary conditions for the 1965 to 2000 evaluation period. | Same as 2010BS |
| Land Use | <ul style="list-style-type: none"> The land use coverage is intermediate between 2000B3 and 2050B3 | Same as 2010BS |
| Natural Area Land Cover (Vegetation) | Vegetation classes and their spatial distribution in the natural areas comes from the following data: <ul style="list-style-type: none"> Walsh 1995 aerial photography in Everglades National Park Rutchey 1995 classification in WCA-3B, WCA-3A north of Alligator Alley and the Miami Canal, WCA-2A & 2B Richardson 1990 data for Loxahatchee National Wildlife Refuge FLUCCS 1995 for Big Cypress National Preserve, Holey Land & Rotenberger Wildlife Management Areas & WCA-3A south of Alligator Alley and Miami Canal. (Documented in August 2003 SFWMD memorandum from J. Barnes and K. Tarboton to J. Obeysekera). | Same as 2010BS |
| Lake Okeechobee Service Area | | |
| LOSA Basins | <ul style="list-style-type: none"> Southern Indian Prairie Basin, S-4, North Lake Shore and Northeast Lake Shore demands and runoff based on AFSIRS (Agricultural Field-Scale Irrigation Requirement Simulation) modeling using 2010 LU projections. | Same as 2010BS |
| Lake Okeechobee | <ul style="list-style-type: none"> Lake Okeechobee Regulation Schedule WSE according to WSE decision trees, with pulse releases in Zone D modeled as Level III pulse | Same as 2010BS |



| Feature | 2010 Base Condition Assumptions | 2010BCAlt1 Proposed Action |
|---------------------------------|---|----------------------------|
| | <p>in upper third of the zone, Level II pulse in middle third of the zone, and Level I pulse in the lower third of the zone, when the decision tree calls for regulatory releases to the estuaries in that zone.</p> <ul style="list-style-type: none"> • WSE thresholds derived from the Class Limit Adjustment (CLA) WSE modification: Increase the frequency of Pulse Releases in Zone D of WSE. • Modified WSE thresholds for zone D1 to improve utilization of EAA reservoir. • Lake Okeechobee Supply Side management policy for Lake Okeechobee Service Area water restriction cutbacks as per rule 40E-21 and 40E-22 in September, 2001 (13.0-10.5 ft. SSM trigger line). • Emergency flood control backpumping to Lake Okeechobee from the Everglades Agricultural Area. • Kissimmee River Restoration and Headwaters Revitalization Project are complete. • Lake Okeechobee environmental releases to supplement reservoir deliveries to Caloosahatchee and St. Lucie Estuaries. • Environmental deliveries to WCA-3A according to Rainfall Driven Operations as means of operating the EAA Reservoirs. • Lake Okeechobee BMP makeup water deliveries to WCAs are not made. • Adaptive protocols are included. | |
| <p>Acceler8 Projects</p> | <p>Acceler8 Projects On Line by 2010 – See A8 Website.</p> <ul style="list-style-type: none"> • C44 Reservoirs: 9315 acres, depth 5 .ft. • C43 Reservoirs: 11000 acres, depth 15 ft. • EAA Reservoirs- <p>A-1 Reservoir simulated as two interconnected compartments. Compartment 1: irrigation, 9600 acres, depth 12 ft. Compartment 2: environment 6400 acres, depth</p> | <p>Same as 2010BS</p> |



| Feature | 2010 Base Condition Assumptions | 2010BCAlt1 Proposed Action |
|--------------------------------------|---|---|
| | 12 ft. <ul style="list-style-type: none"> • WPA's <ul style="list-style-type: none"> • Site 1 Impoundment: 1660 acres; depth 8 ft. • C-9 Impoundment: 1739 acres; depth 4 ft. • C-11 Impoundment: 1730 acres; depth 4 ft. • Acme Basin B discharge to C51W and then to STA1E • WCA-3A/3B Seepage Management Area | |
| Caloosahatchee River Basin | <ul style="list-style-type: none"> • Caloosahatchee River Basin irrigation demands and runoff were estimated using the AFSIRS method based on 2010 land use. • Public water supply daily intake from the river is included in the analysis. • C43 reservoir supplements basin irrigation needs and estuarine environmental needs. | Same as 2010BS <ul style="list-style-type: none"> • |
| St. Lucie Canal Basin | <ul style="list-style-type: none"> • St. Lucie Canal Basin demands estimated using the AFSIRS method based on 2010 land use. • Basin demands include the Florida Power & Light reservoir at Indiantown. • C44 reservoir supplements basin irrigation needs and estuarine environmental needs. | <ul style="list-style-type: none"> • Same as 2010BS |
| Seminole Brighton Reservation | <ul style="list-style-type: none"> • Brighton reservation demands were estimated using AFSIRS method based on existing planted acreage in a manner consistent with that applied to other basins not in the distributed mesh of the SFWMM. • The 2 in 10 demand set forth in the Seminole Compact Work plan equals 2,262 MGM (million gallons/month). AFSIRS modeled 2 in 10 demands equaled 2,383 MGM. • While estimated demands, and therefore deliveries, for every month of simulation do not equate to monthly entitlement quantities as per Table 7, Agreement 41-21 (Nov. 1992), tribal rights to these quantities are | Same as 2010BS |



| Feature | 2010 Base Condition Assumptions | 2010BCAlt1 Proposed Action |
|--|---|----------------------------|
| | <p>preserved.</p> <ul style="list-style-type: none"> Supply-side Management applies to this agreement. | |
| <p>Seminole Big Cypress Reservation</p> | <ul style="list-style-type: none"> Big Cypress Reservation irrigation demands and runoff were estimated using the AFSIRS method based on existing planted acreage in a manner consistent with that applied to other basins not in the distributed mesh of the SFWMM. The 2 in 10 demand set forth in the Seminole Compact Work Plan equals 2,606 MGM. AFSIRS modeled 2 in 10 demands equaled 2,659 MGM. While estimated demands, and therefore deliveries, for every month of simulation do not equate to monthly entitlement quantities as per the District's Final Order and Tribe's Resolution establishing the Big Cypress Reservation entitlement, tribal rights to these quantities are preserved. Supply-side Management applies to this agreement | <p>Same as 2010BS</p> |
| <p>Seminole Hollywood Reservation</p> | <ul style="list-style-type: none"> Hollywood Reservation demands are set forth under VI. C of the Tribal Rights Compact. Tribal sources of water supply include various bulk sale agreements with municipal service suppliers. | <p>Same as 2010BS</p> |



| Feature | 2010 Base Condition Assumptions | 2010BCAlt1 Proposed Action |
|---|--|---|
| Everglades Agricultural Area | <ul style="list-style-type: none"> Everglades Agricultural Area irrigation demands are simulated using climatic data for the 36 year period of record and a soil moisture accounting algorithm, with parameters calibrated to match historical regional supplemental deliveries from Lake Okeechobee. SFWMM EAA runoff and irrigation demand response to rainfall was calibrated for 1984-95 and verified for 1979-1983/1996-2000. No runoff reduction adjustment was necessary to account for Best Management Practices (BMPs). | Same as 2010BS |
| Everglades Construction Project Stormwater Treatment Areas | <ul style="list-style-type: none"> STA-1E: 5132 acres total treatment area STA-1W: 6670 acres total treatment area STA-2: 6430 acres total treatment area STA 2 Cell 4: 1,902 acres total treatment area STA-3/4: 16543 acres total treatment area STA-5: 4110 acres total treatment area STA 5 Flowway 3: 1,985 acres total treatment area STA-6: 870 acres total treatment area STA 6 Section 2: 1,387 acres total treatment area Operation of STAs assumes maintenance of a 6" minimum depth. | Same as 2010BS, plus: <ul style="list-style-type: none"> Buildout STA B: 6,722 acres total treatment area. Source 100% EAA runoff Buildout STA C: 6,230 acres total treatment areas. Source 139 Basin and Annex |
| Holey Land Wildlife WMA | <ul style="list-style-type: none"> As per Memorandum of Agreement between the FWC and the District. | Same as 2010BS |
| Rotenberger Wildlife WMA | <ul style="list-style-type: none"> Interim Operational Schedule as defined in the Operation Plan for Rotenberger (SFWMD Jan 2002). | Same as 2010BS |
| Water Conservation Areas | | |
| Water Conservation Area 1 (ARM Loxahatchee) | <ul style="list-style-type: none"> Current C&SF Regulation Schedule. Includes regulatory releases to tide through LEC canals. | Same as 2010BS |



| Feature | 2010 Base Condition Assumptions | 2010BCAlt1 Proposed Action |
|--|---|----------------------------|
| National Wildlife Refuge) | <ul style="list-style-type: none"> No net outflow to maintain minimum stages in the LEC Service Area canals (salinity control), if water levels are less than minimum operating criteria of 14 ft. The bottom floor of the schedule (Zone C) is the area below 14 ft. Any water supply releases below the floor will be matched by an equivalent volume of inflow from Lake Okeechobee. | |
| Water Conservation Area 2 A&B | <ul style="list-style-type: none"> Current C&SF regulation schedule. Includes regulatory releases to tide through LEC canals. No net outflow to maintain minimum stages in the LEC Service Area canals (salinity control), if water levels in WCA-2A are less than minimum operating criteria of 10.5 ft. Any water supply releases below the floor will be matched by an equivalent volume of inflow from Lake Okeechobee. | Same as 2010BS |
| Water Conservation Area 3 A&B | <ul style="list-style-type: none"> Current C&SF regulation schedule. Includes regulatory releases to tide through LEC canals. No net outflow to maintain minimum stages in the LEC Service Area canals (salinity control), if water levels are less than minimum operating criteria of 7.5 ft in WCA-3A. Any water supply releases below the floor will be matched by an equivalent volume of inflow from Lake Okeechobee. Structural and operational modifications for L-67 canal conveyance and S-355 structures as in the federally authorized Modified Water Deliver Project. Rainfall driven operational criteria for determining timing of deliveries to and discharges from WCA-3A and WCA-3B. | Same as 2010BS |
| <i>Lower East Coast Service Areas</i> | | |
| Public Water | <ul style="list-style-type: none"> 2010 projections based upon permitted | Same as 2010BS |



| Feature | 2010 Base Condition Assumptions | 2010BCAlt1 Proposed Action |
|--|---|----------------------------|
| Supply and Irrigation | allocation to utilities by 2005, with 2010 well field distribution and inclusion of utility ASR. <ul style="list-style-type: none"> Irrigation demands are based upon existing land use (updated through 2010) and calculated using AFSIRS, reduced to account for landscape and golf course areas irrigated using reuse water and landscape areas irrigated using public water supply. | |
| Other Natural Areas | <ul style="list-style-type: none"> For the Northwest Fork of the Loxahatchee River, the District operates the G-92 structure and associated structures to provide approximately 50 cfs over Lainhart Dam to the Northwest Fork, when sufficient water is available in C-18 Canal. Flows to Pond Apple Slough through S-13A are adjusted in the model to approximate measured flows at the structure. Flows to Biscayne Bay are simulated through Snake Creek, North Bay, the Miami River, Central Bay and South Bay. | Same as 2010BS |
| Features | <ul style="list-style-type: none"> C-4 Impoundment – 843.5 acres | Same as 2010BS |
| Upper East Coast Operational CERP | <ul style="list-style-type: none"> L-8 Reservoir: 870 acres, depth 44 ft. | Same as 2010BS |
| <i>Western Basins and Big Cypress National Preserve</i> | | |
| Western Basins | <ul style="list-style-type: none"> Estimated and updated historical inflows from western basins at two locations: G-136 and G-406. The G-406 location represents potential inflow from the C-139 Basin into STA 5. Data for the period 1978 - 2000 is the same as the data used for the C-139 Basin Rule development. (Documented in June 2002 SFWMD memorandum from L. Cadavid and L. Brion to J. Obeysekera). | Same as 2010BS |



| Feature | 2010 Base Condition Assumptions | 2010BCAlt1 Proposed Action |
|---|--|----------------------------|
| Big Cypress National Preserve | <ul style="list-style-type: none"> Tamiami Trail culverts are not modeled in SFWMM due to the coarse (2x2 mile) model resolution. | Same as 2010BS |
| <i>Everglades National Park and Florida Bay</i> | | |
| Everglades National Park | <ul style="list-style-type: none"> Water deliveries to Everglades National Park are based upon Everglades Rain-driven operations. 8.5 SMA as per the federally authorized Alternative 6D of the 8.5 SMA project. Northern C111 project (2002 IOP EIS) Southern C111 project modeled per C-111 Project 1994 GRR | Same as 2010BS |
| <i>Region-wide Water Management and Related Operations</i> | | |
| Water Shortage Rules | <ul style="list-style-type: none"> The existing condition reflects the existing water shortage policies in 2005 as reflected in South Florida Water Management District Chapters 40E-21 and 40E-22, FAC | Same as 2010BS |



Appendix B. Revised C-139 Basin Data Sets and Inflow Data Sets for STA-5 and STA-6

Revisions to *Updated Flow and Phosphorus Data Sets for the ECP Basins Covering the Period May 1, 1994 – April 30, 2007*

Table 10-4: Discharge Summary for Runoff South of STA-5.

| Water Year | Annual Data | | | | Month | Monthly Data | | | |
|------------|-------------|-----------------|---------|---------|--------|--------------|-----------------|---------|---------|
| | Volume | | TP Load | TP Conc | | Volume | | TP Load | TP Conc |
| | ac-ft | hm ³ | kg | ppb | | ac-ft | hm ³ | kg | ppb |
| 1995 | 236,266 | 291.429 | 56,337 | 193 | Jan | 3,845 | 4.742 | 633 | 133 |
| 1996 | 214,503 | 264.585 | 45,070 | 170 | Feb | 3,268 | 4.031 | 389 | 96 |
| 1997 | 151,440 | 186.798 | 42,427 | 227 | Mar | 3,172 | 3.913 | 467 | 119 |
| 1998 | 149,152 | 183.976 | 30,171 | 164 | Apr | 1,280 | 1.579 | 177 | 112 |
| 1999 | 122,058 | 150.556 | 31,376 | 208 | May | 2,028 | 2.502 | 399 | 159 |
| 2000 | 168,779 | 208.186 | 41,889 | 201 | Jun | 7,874 | 9.712 | 3,093 | 318 |
| 2001 | 2,738 | 3.377 | 973 | 288 | Jul | 14,733 | 18.173 | 4,592 | 253 |
| 2002 | 23,349 | 28.800 | 12,450 | 432 | Aug | 13,787 | 17.006 | 4,036 | 237 |
| 2003 | 39,061 | 48.181 | 13,824 | 287 | Sep | 18,917 | 23.333 | 5,916 | 254 |
| 2004 | 37,633 | 46.420 | 16,457 | 355 | Oct | 15,740 | 19.415 | 4,323 | 223 |
| 2005 | 30,165 | 37.208 | 10,475 | 282 | Nov | 8,364 | 10.317 | 2,013 | 195 |
| 2006 | 86,124 | 106.232 | 44,042 | 415 | Dec | 4,900 | 6.044 | 1,020 | 169 |
| 2007 | 11,530 | 14.222 | 6,274 | 441 | Annual | 97,907 | 120.767 | 27,059 | 224 |
| Min. | 2,738 | 3.377 | 973 | - | | | | | |
| Max. | 236,266 | 291.429 | 56,337 | - | | | | | |
| Ave. | 97,907 | 120.767 | 27,059 | 224 | | | | | |

Table 10-5: Discharge Summary for C-139 Basin.

| Water Year | Annual Data | | | | Month | Monthly Data | | | |
|------------|-------------|-----------------|---------|---------|--------|--------------|-----------------|---------|---------|
| | Volume | | TP Load | TP Conc | | Volume | | TP Load | TP Conc |
| | ac-ft | hm ³ | kg | ppb | | ac-ft | hm ³ | kg | ppb |
| 1995 | 274,099 | 338.095 | 62,190 | 184 | Jan | 6,599 | 8.139 | 861 | 106 |
| 1996 | 236,150 | 291.286 | 48,600 | 167 | Feb | 5,790 | 7.142 | 778 | 109 |
| 1997 | 169,113 | 208.598 | 45,933 | 220 | Mar | 5,669 | 6.992 | 785 | 112 |
| 1998 | 174,689 | 215.475 | 36,375 | 169 | Apr | 2,055 | 2.535 | 284 | 112 |
| 1999 | 141,167 | 174.127 | 36,900 | 212 | May | 3,495 | 4.311 | 603 | 140 |
| 2000 | 212,472 | 262.080 | 53,978 | 206 | Jun | 17,386 | 21.446 | 6,134 | 286 |
| 2001 | 56,877 | 70.156 | 16,908 | 241 | Jul | 28,144 | 34.715 | 9,006 | 259 |
| 2002 | 204,663 | 252.448 | 67,405 | 267 | Aug | 32,235 | 39.762 | 9,282 | 233 |
| 2003 | 229,260 | 282.788 | 77,697 | 275 | Sep | 39,752 | 49.034 | 12,861 | 262 |
| 2004 | 209,423 | 258.319 | 69,154 | 268 | Oct | 30,010 | 37.017 | 7,978 | 216 |
| 2005 | 195,834 | 241.557 | 44,976 | 186 | Nov | 14,335 | 17.682 | 3,456 | 195 |
| 2006 | 353,264 | 435.745 | 109,000 | 250 | Dec | 10,136 | 12.503 | 1,774 | 142 |
| 2007 | 85,883 | 105.935 | 30,303 | 286 | Annual | 195,607 | 241.278 | 53,801 | 223 |
| Min. | 56,877 | 70.156 | 16,908 | - | | | | | |
| Max. | 353,264 | 435.745 | 109,000 | - | | | | | |
| Ave. | 195,607 | 241.278 | 53,801 | 223 | | | | | |



Figure 10-2: Average Monthly Flows and Phosphorus Levels for C-139 Basin Runoff.

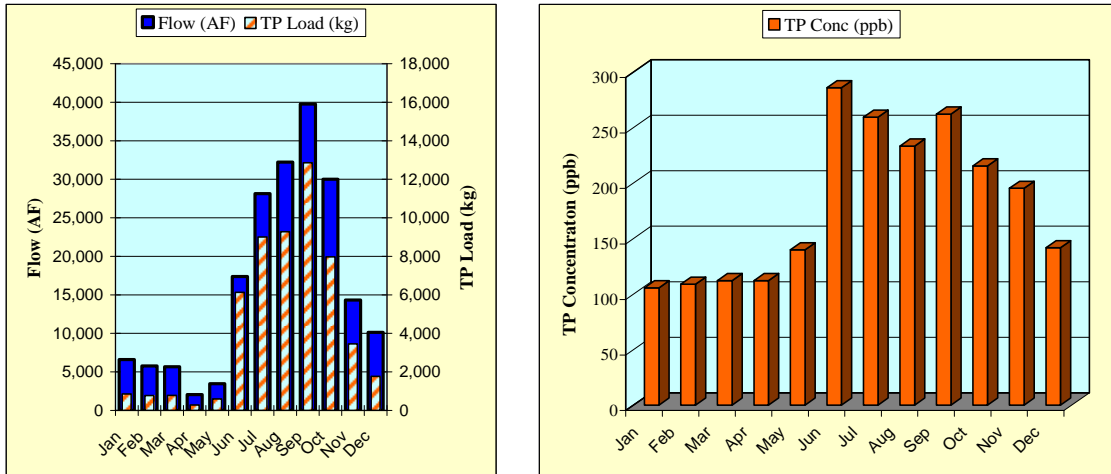


Figure 10-3: WY1995-2007 Flows and Phosphorus Levels for C-139 Basin Runoff.

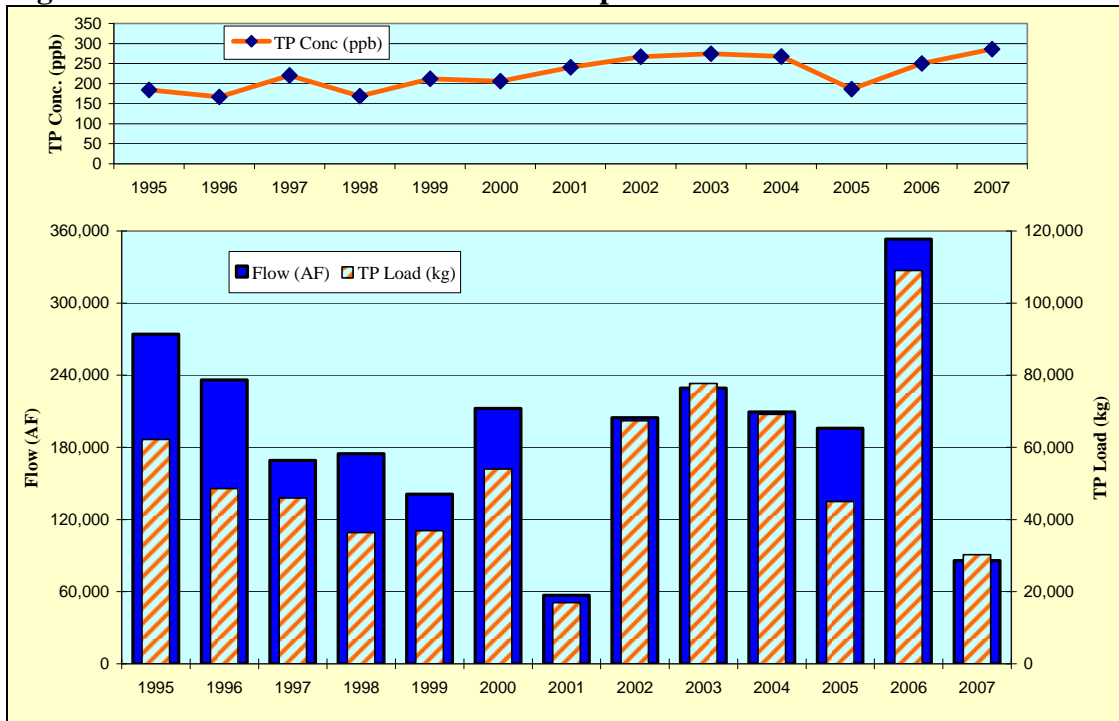


Table 11-1: Comparison of Annual Average Flow and Phosphorus Data

| Basin | This Update | | | 2005 Update | | | Difference | | |
|---|------------------|------------------|----------------|------------------|------------------|----------------|------------------------------|---------------------------|------------------------|
| | Flow AF/yr | TP Load kg/yr | TP Conc ppb | Flow AF/yr | TP Load kg/yr | TP Conc ppb | Flow AF/yr | TP Load kg/yr | TP Conc ppb |
| S-5A | 254,957 | 51,729 | 164 | 277,225 | 53,171 | 155 | -22,268 | -1,442 | 9 |
| S-6/S-2 | 284,089 | 38,023 | 109 | 297,002 | 36,210 | 99 | -12,913 | 1,813 | 10 |
| S-7/S-2 | 237,451 | 28,179 | 96 | 235,812 | 22,976 | 79 | 1,639 | 5,203 | 17 |
| S-8/S-3 | 294,685 | 29,909 | 82 | 309,172 | 31,199 | 82 | -14,487 | -1,290 | 0 |
| L-8 | 182,520 | 22,050 | 98 | 171,673 | 19,743 | 93 | 10,847 | 2,307 | 5 |
| C-51W | 125,326 | 30,222 | 196 | 125,543 | 19,599 | 127 | -217 | 10,623 | 69 |
| Acme Basin B | 31,813 | 4,703 | 120 | 33,519 | 4,892 | 118 | -1,706 | -189 | 2 |
| East Beach Water Control District | 16,471 | 9,938 | 489 | 14,490 | 9,347 | 523 | 1,981 | 591 | -34 |
| East Shore Water Control District and 715 Farms | 28,608 | 4,748 | 135 | 27,933 | 4,279 | 124 | 675 | 469 | 10 |
| South Shore Drainage District | 12,785 | 1,644 | 104 | 9,358 | 1,235 | 107 | 3,427 | 409 | -3 |
| South Florida Conservancy District | 29,314 | 4,255 | 118 | 24,047 | 3,658 | 123 | 5,267 | 597 | -6 |
| C-139 | 195,607 | 53,801 | 223 | 265,072 | 64,101 | 196 | -69,465 | -10,300 | 27 |
| C-139 Annex | 41,486 | 4,987 | 97 | 39,627 | 4,830 | 99 | 1,859 | 157 | -1 |
| Total | 1,735,112 | 284,188 | 133 | 1,830,473 | 275,240 | 122 | -95,361 -5% | 8,948 3% | 11 9% |



Revisions to Updated STA Inflow Data Sets for the 2010 Period

Table 5-1: Annual Runoff from the C-139 Basin to STA-5 and STA-6.

| Water Year | Volume ac-ft | TP Load kg | TP Conc. ppb |
|-------------|-----------------|---------------|-----------------|
| 1995 | 236,529 | 56,368 | 193 |
| 1996 | 214,503 | 45,070 | 170 |
| 1997 | 151,440 | 42,427 | 227 |
| 1998 | 149,152 | 30,171 | 164 |
| 1999 | 122,058 | 31,376 | 208 |
| 2000 | 176,867 | 44,149 | 202 |
| 2001 | 53,197 | 16,642 | 254 |
| 2002 | 182,608 | 61,521 | 273 |
| 2003 | 209,265 | 71,031 | 275 |
| 2004 | 190,713 | 64,536 | 274 |
| 2005 | 150,075 | 34,933 | 189 |
| 2006 | 302,638 | 97,068 | 260 |
| 2007 | 71,783 | 28,149 | 318 |
| Min. Annual | 53,197 | 16,642 | - |
| Max. Annual | 302,638 | 97,068 | - |
| Ave. Annual | 170,064 | 47,957 | 229 |

Table 7-22: Estimate of Inflow Distribution to Balance PLR.

| Flow-way | Area | TP inflow | Flow at PLR | Load | PLR |
|----------|--------|-----------|-------------|--------|------|
| STA-5 1 | 2,055 | 229 | 28,177 | 7,945 | 0.96 |
| STA-5 2 | 2,055 | 229 | 28,177 | 7,945 | 0.96 |
| STA-5 3 | 1,985 | 229 | 27,217 | 7,674 | 0.96 |
| STA-5 4 | 2,176 | 229 | 29,836 | 8,413 | 0.96 |
| STA-5 5 | 2,669 | 229 | 36,595 | 10,319 | 0.96 |
| STA-6 3 | 1,857 | 178 | 32,692 | 7,180 | 0.96 |
| STA-6 5 | 652 | 97 | 20,970 | 2,521 | 0.96 |
| STA-6 2 | 245 | 97 | 7,880 | 947 | 0.96 |
| Total | 13,694 | 203 | 211,544 | 52,944 | 0.96 |

Table 7-23: Summary of Long-term Average Annual Inflow to STA-5.

| Source | Flow (AF/yr) | TP Load (kg/yr) | TP Conc (ppb) |
|-------------|--------------|-----------------|---------------|
| C-139 Basin | 150,001 | 42,300 | 229 |



Table 7-24: Annual Runoff to STA-5 from All Sources.

| Water Year | Volume (acre-feet) | TP Load Load (kg) | TP Conc. (ppb) | Phosphorus Loading Rate (g/m2/yr) |
|-------------|--------------------|-------------------|----------------|-----------------------------------|
| 1995 | 208,626 | 49,719 | 193 | 1.12 |
| 1996 | 189,198 | 39,753 | 170 | 0.90 |
| 1997 | 133,575 | 37,422 | 227 | 0.85 |
| 1998 | 131,557 | 26,612 | 164 | 0.60 |
| 1999 | 107,659 | 27,675 | 208 | 0.63 |
| 2000 | 156,002 | 38,940 | 202 | 0.88 |
| 2001 | 46,922 | 14,679 | 254 | 0.33 |
| 2002 | 161,066 | 54,263 | 273 | 1.23 |
| 2003 | 184,578 | 62,652 | 275 | 1.42 |
| 2004 | 168,215 | 56,922 | 274 | 1.29 |
| 2005 | 132,370 | 30,812 | 189 | 0.70 |
| 2006 | 266,936 | 85,617 | 260 | 1.93 |
| 2007 | 63,315 | 24,828 | 318 | 0.56 |
| Min. Annual | 46,922 | 14,679 | - | 0.33 |
| Max. Annual | 266,936 | 85,617 | - | 1.93 |
| Ave. Annual | 150,001 | 42,300 | 229 | 0.96 |

Table 7-25: Summary of Long-term Average Annual Inflow to STA-6.

| Source | Flow (AF/yr) | TP Load (kg/yr) | TP Conc (ppb) |
|--------------|---------------|-----------------|---------------|
| C-139 Basin | 20,062 | 5,657 | 229 |
| C-139 Annex | 41,480 | 4,987 | 97 |
| Total | 61,542 | 10,644 | 140 |

Table 7-26: Annual Runoff to STA-6 from All Sources.

| Water Year | Volume (acre-feet) | TP Load Load (kg) | TP Conc. (ppb) | Phosphorus Loading Rate (g/m2/yr) |
|-------------|--------------------|-------------------|----------------|-----------------------------------|
| 1995 | 69,389 | 11,637 | 136 | 1.04 |
| 1996 | 61,742 | 9,876 | 130 | 0.89 |
| 1997 | 58,061 | 10,112 | 141 | 0.91 |
| 1998 | 63,676 | 7,581 | 97 | 0.68 |
| 1999 | 38,669 | 6,833 | 143 | 0.61 |
| 2000 | 67,230 | 11,624 | 140 | 1.04 |
| 2001 | 33,106 | 6,528 | 160 | 0.59 |
| 2002 | 59,264 | 11,103 | 152 | 1.00 |
| 2003 | 68,608 | 13,640 | 161 | 1.22 |
| 2004 | 69,357 | 13,344 | 156 | 1.20 |
| 2005 | 65,222 | 9,772 | 121 | 0.88 |
| 2006 | 101,432 | 18,352 | 147 | 1.65 |
| 2007 | 44,295 | 7,970 | 146 | 0.72 |
| Min. Annual | 33,106 | 6,528 | - | 0.59 |
| Max. Annual | 101,432 | 18,352 | - | 1.65 |
| Ave. Annual | 61,542 | 10,644 | 140 | 0.96 |



Table 9-1: Comparison of Current Data Sets to 2005 EAA RFS Values.

| Basin or Source | Water Years | 2005 EAA RFS ¹ | | | This Analysis | | |
|--|---------------|---------------------------|----------------|-------------|------------------|----------------|-------------|
| | | Flow AF/yr | TP Load kg/yr | TP Conc ppb | Flow AF/yr | TP Load kg/yr | TP Conc ppb |
| EAA Basin Runoff | | | | | | | |
| Discharge to STAs | | | | | | | |
| S-5A Basin | 1966-2000 | 291,096 | 55,256 | 154 | 308,058 | 61,844 | 163 |
| S-6/S-2 Basin | 1966-2000 | 236,624 | 28,327 | 97 | 181,700 | 23,661 | 106 |
| S-7/S-2 Basin | 1966-2000 | 109,310 | 10,747 | 80 | 141,171 | 16,640 | 96 |
| S-8/S-3 Basin | 1966-2000 | 170,624 | 17,460 | 83 | 132,584 | 13,383 | 82 |
| Subtotal | 1966-2000 | 807,654 | 111,790 | 112 | 763,513 | 115,528 | 123 |
| Discharge to EAASR | | | | | | | |
| S-7/S-2 Basin | 1966-2000 | 72,078 | 7,235 | 81 | 119,549 | 14,011 | 95 |
| S-8/S-3 Basin | 1966-2000 | 59,784 | 5,910 | 80 | 97,242 | 10,012 | 83 |
| Subtotal | 1966-2000 | 131,862 | 13,145 | 81 | 216,791 | 24,023 | 90 |
| Discharge to Lake Okeechobee | | | | | | | |
| S-5A Basin | 1966-2000 | 0 | 0 | - | 1,697 | 341 | 163 |
| S-2/S-6/S-7 Basin | 1966-2000 | 24,946 | 2,822 | 92 | 17,826 | 2,197 | 100 |
| S-8/S-3 Basin | 1966-2000 | 4,091 | 445 | 88 | 5,129 | 518 | 82 |
| Subtotal | 1966-2000 | 29,037 | 3,267 | 91 | 24,652 | 3,056 | 100 |
| Total from EAA Basins | | | | | | | |
| S-5A Basin | 1966-2000 | 291,096 | 55,256 | 154 | 309,754 | 62,185 | 163 |
| S-2/S-6/S-7 Basin | 1966-2000 | 442,958 | 49,131 | 90 | 460,246 | 56,509 | 100 |
| S-8/S-3 Basin | 1966-2000 | 234,499 | 23,815 | 82 | 234,955 | 23,913 | 83 |
| Total | 1966-2000 | 968,553 | 128,202 | 107 | 1,004,956 | 142,607 | 115 |
| Chapter 298 districts and 715 Farms | | | | | | | |
| Discharge to STAs | | | | | | | |
| EBWCD | 1966-2000 | 15,212 | 9,386 | 500 | 16,033 | 9,244 | 467 |
| ESWCD/715 Farms | 1966-2000 | 29,818 | 4,588 | 125 | 31,129 | 5,215 | 136 |
| SSDD | 1966-2000 | 10,559 | 1,390 | 107 | 10,539 | 1,324 | 102 |
| SFCD | 1966-2000 | 21,145 | 3,183 | 122 | 24,110 | 3,363 | 113 |
| Subtotal | 1966-2000 | 76,734 | 18,547 | 196 | 81,810 | 19,146 | 190 |
| Discharge to Lake Okeechobee | | | | | | | |
| EBWCD | 1966-2000 | 0 | 0 | - | 487 | 278 | 462 |
| ESWCD/715 Farms | 1966-2000 | 0 | 0 | - | 3,801 | 479 | 102 |
| SSDD | 1966-2000 | 0 | 0 | - | 344 | 43 | 102 |
| SFCD | 1966-2000 | 0 | 0 | - | 1,363 | 190 | 113 |
| Subtotal | 1966-2000 | 0 | 0 | - | 5,996 | 990 | 134 |
| Total from Ch. 298 Districts and 715 Farms | | | | | | | |
| EBWCD | 1966-2000 | 15,212 | 9,386 | 500 | 16,520 | 9,522 | 467 |
| ESWCD/715 Farms | 1966-2000 | 29,818 | 4,588 | 125 | 34,931 | 5,694 | 132 |
| SSDD | 1966-2000 | 10,559 | 1,390 | 107 | 10,883 | 1,367 | 102 |
| SFCD | 1966-2000 | 21,145 | 3,183 | 122 | 25,473 | 3,553 | 113 |
| Total | 1966-2000 | 76,734 | 18,547 | 196 | 87,806 | 20,136 | 186 |
| Western Basins | | | | | | | |
| C-139 to L2/L-3 | Varies | 159,030 | 39,111 | 199 | 170,064 | 47,957 | 229 |
| C-139 to STA-3/4 | 1966-2000 | 13,204 | 2,958 | 182 | 13,201 | 3,401 | 209 |
| C-139 to EAA Irrig. | 1966-2000 | 4,383 | 969 | 179 | 4,385 | 1,130 | 209 |
| USSC SDR Unit 2 | 1998-2005 | 0 | 0 | - | 0 | 0 | - |
| C-139 Annex | Varies | 40,176 | 4,873 | 98 | 41,480 | 4,987 | 97 |
| Total | Varies | 216,793 | 47,911 | 179 | 229,130 | 57,475 | 203 |
| Eastern Basins | | | | | | | |
| C-51 West to STA | 1966-2000 | 136,812 | 23,307 | 138 | 130,375 | 31,529 | 196 |
| C-51 West to East | 1966-2000 | 3,610 | 615 | 138 | 6,902 | 1,669 | 196 |
| L-8 Basin to STA | 1966-2000 | 0 | 0 | - | 8,571 | 1,019 | 96 |
| L-8 Basin to Lake | 1966-2000 | 71,931 | 9,157 | 103 | 57,008 | 6,971 | 99 |
| L-8 to WPB WCA | 1966-2000 | 0 | 0 | - | 85,022 | 10,397 | 99 |
| L-8 to L-8 Rock Pit | 1966-2000 | 0 | 0 | - | 17,461 | 2,135 | 99 |
| L-8 to C-51 East | 1966-2000 | 36,256 | 3,548 | 79 | 25,712 | 3,173 | 100 |
| Acme Basin B | 1966-2000 | 34,887 | 4,850 | 113 | 35,066 | 4,915 | 114 |
| Total | 1966-2000 | 283,496 | 41,477 | 119 | 366,117 | 61,809 | 137 |
| Total Runoff | Varies | 1,545,576 | 236,137 | 124 | 1,688,009 | 282,026 | 135 |

¹ 2005 EAA RFS values came from Appendix D "Inflow Data Sets for the Period 2010-2014"



Table 9-1: Comparison of Current Data Sets to 2005 EAA RFS Values (Cont'd).

| Basin or Source | Water Years | 2005 EAA RFS ¹ | | | This Analysis | | |
|---|-------------|---------------------------|---------------|-------------|---------------|---------------|-----------------|
| | | Flow AF/yr | TP Load kg/yr | TP Conc ppb | Flow AF/yr | TP Load kg/yr | TP Conc ppb |
| Lake Okeechobee Releases | | | | | | | |
| Flow-Through Releases in STA Inflows | | | | | | | |
| S-351 | 1966-2000 | 1,551 | 132 | 69 | 16 | 2 | 100 |
| S-352 | 1966-2000 | 19 | 2 | 104 | 0 | 0 | - |
| S-354 | 1966-2000 | 26,581 | 2,115 | 65 | 0 | 0 | - |
| Subtotal | 1966-2000 | 28,150 | 2,250 | 65 | 16 | 2 | 100 |
| Water Supply Bypasses | | | | | | | |
| S-351 | 1966-2000 | 11,484 | 1,189 | 84 | 18,559 | 2,527 | 110 |
| S-352 | 1966-2000 | 14,184 | 2,227 | 127 | 21,054 | 3,439 | 132 |
| S-354 | 1966-2000 | 109,279 | 9,391 | 70 | 46,140 | 5,271 | 93 |
| Subtotal | 1966-2000 | 134,947 | 12,807 | 77 | 85,753 | 11,237 | 106 |
| Total Flow-Through Releases | | | | | | | |
| S-351 | 1966-2000 | 13,035 | 1,321 | 82 | 18,575 | 2,529 | 110 |
| S-352 | 1966-2000 | 14,203 | 2,229 | 127 | 21,054 | 3,439 | 132 |
| S-354 | 1966-2000 | 135,860 | 11,506 | 69 | 46,140 | 5,271 | 93 |
| Total | 1966-2000 | 163,097 | 15,056 | 75 | 85,769 | 11,239 | 106 |
| Lake Okeechobee Releases to EAA Storage Reservoir | | | | | | | |
| S-351 | 1966-2000 | 131,928 | 16,689 | 103 | 139,761 | 17,233 | 100 |
| S-354 | 1966-2000 | 152,793 | 16,958 | 90 | 183,461 | 22,629 | 100 |
| Total | 1966-2000 | 284,721 | 33,647 | 96 | 323,222 | 39,862 | 100 |
| Total Lake Okeechobee Releases | | | | | | | |
| S-351 | 1966-2000 | 144,963 | 18,010 | 101 | 158,336 | 19,762 | 101 |
| S-352 | 1966-2000 | 14,203 | 2,229 | 127 | 21,054 | 3,439 | 132 |
| S-354 | 1966-2000 | 288,653 | 28,464 | 80 | 229,601 | 27,900 | 99 |
| Total | 1966-2000 | 447,819 | 48,703 | 88 | 408,991 | 51,101 | 101 |
| EAA Storage Reservoir Releases | | | | | | | |
| S-2/S-6/S-7 Irrig. | 1966-2000 | 105,115 | --- | --- | 95,323 | 9,007 | 77 ² |
| S-3/S-8 Irrig. | 1966-2000 | 74,911 | --- | --- | 72,835 | 6,882 | 77 |
| STA-3/4 | 1966-2000 | 233,685 | --- | --- | 363,442 | 34,340 | 77 |
| Total | 1966-2000 | 413,711 | --- | --- | 531,600 | 50,229 | 77 |
| Seepage from WCA-2A | | | | | | | |
| WCA-2A seepage | 1966-2000 | 27,530 | 509 | 15 | 27,530 | 509 | 15 |
| Total Volumes and TP Loads | | | | | | | |
| Direct STA Inflow | Varies | 1,562,245 | 209,164 | 109 | 1,639,453 | 264,464 | 131 |
| To EAA SR | 1966-2000 | 416,583 | 46,792 | 91 | 540,013 | 63,885 | 96 |
| To Lake Okeechobee | 1966-2000 | 100,968 | 12,424 | 100 | 87,656 | 11,017 | 102 |
| Other Destinations | 1966-2000 | 44,249 | 5,132 | 94 | 139,482 | 18,504 | 108 |
| Total (w/ L-8 Basin) | Varies | 2,124,045 | 273,512 | 104 | 2,406,604 | 357,870 | 121 |
| Total (w/o L-8 Basin) | Varies | 2,015,858 | 260,807 | 105 | 2,212,830 | 334,174 | 122 |

¹ 2005 EAA RFS values came from Appendix D "Inflow Data Sets for the Period 2010-2014"

² Assumes 76.6 ppb TP concentration from EAA Storage Reservoir; Lake O=100 ppb



Table 9-2: Comparison of Basin Runoff to the STAs, Compartments B and C, and the EAA Storage Reservoir.

| Source Basin | 1994 Conceptual Design ¹ | | | This Analysis | | |
|---------------------------------|-------------------------------------|----------------|-------------|------------------|----------------|-------------|
| | Flow AF/yr | TP Load kg/yr | TP Conc ppb | Flow AF/yr | TP Load kg/yr | TP Conc ppb |
| S-5A | 195,342 | 50,631 | 210 | 308,058 | 61,844 | 163 |
| S-6/S-2 | 131,676 | 23,166 | 143 | 181,700 | 23,661 | 106 |
| S-7/S-2 | 155,734 | 19,211 | 100 | 223,454 | 24,415 | 89 |
| S-8/S-3 | 187,020 | 47,290 | 205 | 200,833 | 19,831 | 80 |
| Total EAA | 669,772 | 140,298 | 170 | 914,044 | 129,751 | 115 |
| Ch. 298 Districts/715 Farms | 24,857 | 6,156 | 201 | 81,810 | 19,146 | 190 |
| C-51W | 105,376 | 24,042 | 185 | 130,375 | 31,529 | 196 |
| L-8 | 0 | 0 | - | 8,571 | 1,019 | 96 |
| C-139 | 97,605 | 28,693 | 238 | 183,265 | 51,358 | 227 |
| C-139 Annex | 0 | 0 | - | 41,480 | 4,987 | 97 |
| Acme Basin B | 0 | 0 | - | 35,066 | 4,915 | 114 |
| WCA-2A | 27530.11 | 509.37586 | 15 | 27,530 | 509 | 15 |
| Lake Okeechobee ² | 254,571 | 21,364 | 68 | 217,536 | 20,554 | 77 |
| Effective Treatment Area | 39,690 | | | 56,997 | | |
| Total Inflows | 1,179,711 | 221,062 | 152 | 1,639,677 | 263,769 | 130 |

¹ Assumed 20% reduction in EAA runoff.

² Calculated as 60% of EAA SR discharge to STA-3/4, equivalent to percentage of Lake inflows.

Table 9-3: Summary of Inflows to the STAs and EAA Storage Reservoir for STA-2 PLR of 1.0 g/m²/yr.

| STA | Effective Treatment Area acres | Inflow Volume (acre-feet) | Inflow TP Load Load (kg) | Inflow TP Conc. (ppb) | Phosphorus Loading Rate (g/m ² /yr) |
|-----------------------------|--------------------------------|---------------------------|--------------------------|-----------------------|--|
| STA-1E | 5,132 | 193,818 | 41,864 | 175 | 2.02 |
| STA-1W | 6,670 | 243,172 | 54,409 | 181 | 2.02 |
| STA-2 | 6,338 | 182,713 | 25,664 | 114 | 1.00 |
| Compartment B | 8,620 | 225,073 | 28,509 | 103 | 0.82 |
| STA-3/4 ¹ | 16,543 | 583,360 | 60,353 | 84 | 0.90 |
| STA-5 (incl. Comp. C) | 10,940 | 150,001 | 42,300 | 229 | 0.96 |
| STA-6 (incl. Comp. C) | 2,754 | 61,542 | 10,644 | 140 | 0.96 |
| Total Inflow to STAs | 56,997 | 1,639,679 | 263,743 | 130 | 1.14 |
| EAA SR A-1 | 15,200 | 540,013 | 63,885 | 96 | 1.04 |
| Total | 72,197 | 2,179,692 | 327,628 | 122 | 1.12 |

¹ Assumes 76.6 ppb TP concentration from EAA Storage Reservoir; Lake O=100 ppb



Table 9-4: Comparison of Inflows to 2005 EAARFS Values (STA-2 PLR of 1.0 g/m²/yr).

| Receiving Water Body | 2005 EAA RFS ¹ | | | This Analysis | | |
|-----------------------|---------------------------|----------------|-------------|------------------|----------------|-------------|
| | Flow AF/yr | TP Load kg/yr | TP Conc ppb | Flow AF/yr | TP Load kg/yr | TP Conc ppb |
| STA-1E | 171,800 | 27,030 | 128 | 193,818 | 41,864 | 175 |
| STA-1W | 131,400 | 25,800 | 160 | 243,172 | 54,409 | 181 |
| STA-2 | 180,700 | 20,300 | 91 | 182,713 | 25,664 | 114 |
| Compartment B | 291,100 | 44,100 | 123 | 225,073 | 28,509 | 103 |
| STA-3/4 | 585,500 | 65,920 | 91 | 583,360 | 60,353 | 84 |
| STA-5 (incl. Comp. C) | 159,030 | 39,111 | 199 | 150,001 | 42,300 | 229 |
| STA-6 (incl. Comp. C) | 40,176 | 4,873 | 98 | 61,542 | 10,644 | 140 |
| EAA SR | 416,900 | 50,000 | 97 | 540,013 | 63,885 | 96 |
| Total Inflows | 1,976,606 | 277,134 | 114 | 2,179,692 | 327,628 | 122 |

¹For comparison, inflows from the EAA RFS Alternative 1 for the 2010-2014 Period are presented. Alternative 1 included additional facilities to transfer S-5A Basin runoff to the west.

Table 9-5: Comparison of STA Inflows to 1994 Conceptual Design Values, Excluding Compartments B and C for STA-2 PLR of 1.0 g/m²/yr.

| Receiving Water Body | 1994 Conceptual Design | | | This Analysis | | |
|-----------------------|------------------------|----------------|-------------|------------------|----------------|-------------|
| | Flow AF/yr | TP Load kg/yr | TP Conc ppb | Flow AF/yr | TP Load kg/yr | TP Conc ppb |
| STA-1E | 124,900 | 29,500 | 191 | 193,818 | 41,864 | 175 |
| STA-1W | 142,853 | 37,701 | 214 | 243,172 | 54,409 | 181 |
| STA-2 (excl. Comp. B) | 174,641 | 33,764 | 157 | 182,713 | 25,664 | 114 |
| STA-3/4 | 604,753 | 87,200 | 117 | 583,360 | 60,353 | 84 |
| STA-5 (excl. Comp. C) | 87,000 | 28,000 | 261 | 56,353 | 15,890 | 229 |
| STA-6 (excl. Comp. C) | 18,034 | 4,388 | 197 | 28,850 | 3,468 | 96 |
| Total Inflows | 1,152,181 | 220,553 | 155 | 1,288,266 | 201,649 | 127 |

Table 9-6: Comparison of STA Inflows to 1994 Conceptual Design Values, Including Compartments B and C for STA-2 PLR of 1.0 g/m²/yr.

| Receiving Water Body | 1994 Conceptual Design | | | | This Analysis | | | |
|-----------------------|------------------------|------------------|----------------|-------------|--------------------|------------------|----------------|-------------|
| | Eff. Tr. Area (ac) | Flow AF/yr | TP Load kg/yr | TP Conc ppb | Eff. Tr. Area (ac) | Flow AF/yr | TP Load kg/yr | TP Conc ppb |
| STA-1E | 5,132 | 124,900 | 29,500 | 191 | 5,132 | 193,818 | 41,864 | 175 |
| STA-1W | 6,670 | 142,853 | 37,701 | 214 | 6,670 | 243,172 | 54,409 | 181 |
| STA-2 (incl. Comp. B) | 6,338 | 174,641 | 33,764 | 157 | 14,958 | 407,786 | 54,173 | 108 |
| STA-3/4 | 16,543 | 604,753 | 87,200 | 117 | 16,543 | 583,360 | 60,353 | 84 |
| STA-5 (incl. Comp. C) | 4,110 | 87,000 | 28,000 | 261 | 10,940 | 150,001 | 42,300 | 229 |
| STA-6 (incl. Comp. C) | 897 | 18,034 | 4,388 | 197 | 2,754 | 61,542 | 10,644 | 227 |
| Total Inflows | 39,690 | 1,152,181 | 220,553 | 155 | 56,997 | 1,639,679 | 263,743 | 130 |



Appendix C. DMSTA Output

STA-1E

| DMSTA2- Inputs & Outputs | | Project: STA 1E EIS | | | | | | | | | | | | Model Release: | 07/05/07 | | |
|---------------------------------------|--|---------------------|---|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|----------------|-------------------------------|--|------------|
| Input Variable | | Units | Value | | | | | | | | | | | | Current Date: | | 10/19/2007 |
| Design Case Name | | All EIS Alt | All cells | | | | | | | | | | | | | | |
| Input Series Name | | TS_Alt_EIS_Alt | Inflows to cells 1-4 increased by 4% (approx. 6,400 ac-ft/yr) for seepage recycle from west flow path | | | | | | | | | | | | | | |
| Starting Date for Simulation | | 05/01/65 | East Distribution Cell modeled as two cells in parallel; minimal treatment in EDC (K=0.01 m/yr) | | | | | | | | | | | | | | |
| Ending Date for Simulation | | 04/30/00 | West Distribution Cell modeled as two cells in parallel; minimal treatment in WDC (K=0.01 m/yr) | | | | | | | | | | | | | | |
| Starting Date for Output | | 05/01/65 | | | | | | | | | | | | | | | |
| Number of Iterations | | 4 | | | | | | | | | | | | | | | |
| Integration Steps Per Day | | 4 | | | | | | | | | | | | | | | |
| Output Averaging Interval | | days | Simulation Type: Uncertainty Analysis | | | | | | | | | | | | Diagnostics | | |
| Inflow Conc Scale Factor | | 1 | Output Variable | | | | | | | | | | | | H2O Balance Error Mean & Max | | 0.0% |
| Raintail P Conc | | ppb | FWM Outflow C (ppb) | | | | | | | | | | | | Mass Balance Error Mean & Max | | 0.0% |
| Atmospheric P Load (Dry) | | mg/m2-yr | GM Outflow C (ppb) | | | | | | | | | | | | Iteration % Convergence | | 3 |
| | | | Load Reduction | | | | | | | | | | | | Warning/Error Messages | | 4 |
| | | | Bypass Load (%) | | | | | | | | | | | | | | |
| Cell Number -> | | | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | | | |
| Cell Label | | | EDCE | 1 | 2 | EDCW | 3 | 4N | 4S | WDCW | 7 | WDCE | 5 | 6 | | | |
| Vegetation Type | | | EMG_3 | SAV_3 | SAV_3 | EMG_3 | SAV_3 | SAV_3 | SAV_3 | EMG_3 | SAV_3 | EMG_3 | SAV_3 | SAV_3 | | | |
| Inflow Fraction | | | 0.21993431 | | | | | | | | | | | | 0.2292878 | | |
| Downstream Cell Number | | | 2 | | | | | | | | | | | | 3 | | |
| Surface Area | | km2 | 0.85 | | | | | | | | | | | | 2.25 | | 4.25 |
| Mean Width of Flow Path | | km | 0.66 | | | | | | | | | | | | 1.55 | | 0.75 |
| Number of Tanks in Series | | | 0.5 | | | | | | | | | | | | 3.0 | | 3.0 |
| Minimum Depth for Releases | | cm | | | | | | | | | | | | | | | |
| Release 1 Series Name | | | | | | | | | | | | | | | | | |
| Release 2 Series Name | | | | | | | | | | | | | | | | | |
| Outflow Series Name | | | | | | | | | | | | | | | | | |
| Depth Series Name | | | | | | | | | | | | | | | | | |
| Outflow Control Depth | | cm | 38.1 | | | | | | | | | | | | 38.1 | | 38.1 |
| Outflow Weir Depth | | cm | 4 | | | | | | | | | | | | 4 | | 4 |
| Outflow Coefficient - Exponent | | | 1 | | | | | | | | | | | | 1 | | 1 |
| Outflow Coefficient - Intercept | | cm | | | | | | | | | | | | | | | |
| Bypass Depth | | cm | | | | | | | | | | | | | | | |
| Maximum Inflow | | hm3/day | | | | | | | | | | | | | | | |
| Maximum Outflow | | hm3/day | | | | | | | | | | | | | | | |
| Inflow Seepage Rate | | (cm/d) / cm | | | | | | | | | | | | | 0.0054 | | 0.0057 |
| Inflow Seepage Control Elev | | ppb | | | | | | | | | | | | | 69 | | 94 |
| Inflow Seepage Conc | | (cm/d) / cm | 0.0095 | | | | | | | | | | | | 0.0042 | | 0.0095 |
| Outflow Seepage Rate | | (cm/d) / cm | | | | | | | | | | | | | 0.0054 | | 0.01 |
| Outflow Seepage Control Elev | | ppb | | | | | | | | | | | | | -137 | | -99 |
| Max Outflow Seepage Conc | | ppb | 20 | | | | | | | | | | | | 20 | | 20 |
| Seepage Recycle to Cell Number | | | 1 | | | | | | | | | | | | 1 | | 1 |
| Seepage Recycle Fraction | | | 1 | | | | | | | | | | | | 1 | | 1 |
| Seepage Discharge Fraction | | | | | | | | | | | | | | | | | |
| Initial Water Column Conc | | ppb | 30 | | | | | | | | | | | | 30 | | 30 |
| Initial P Storage Per Unit Area | | mg/m2 | 500 | | | | | | | | | | | | 500 | | 500 |
| Initial Water Column Depth | | cm | 50 | | | | | | | | | | | | 50 | | 50 |
| C0 = Conc at 0 gm2 P Storage | | ppb | 3 | | | | | | | | | | | | 3 | | 3 |
| C1 = Conc at 1 gm2 P storage | | ppb | 22 | | | | | | | | | | | | 22 | | 22 |
| C2 = Conc at Half-Max Uptake | | ppb | 300 | | | | | | | | | | | | 300 | | 300 |
| K - Net Settling Rate at Steady State | | m/yr | 0.0 | | | | | | | | | | | | 52.5 | | 0.0 |
| Z1 = Saturated Uptake Depth | | cm | 40 | | | | | | | | | | | | 40 | | 40 |
| Z2 = Lower Penalty Depth | | cm | 100 | | | | | | | | | | | | 100 | | 100 |
| Z3 = Upper Penalty Depth | | cm | 200 | | | | | | | | | | | | 200 | | 200 |

| Output Variables | | Units | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | Overall | | |
|----------------------------------|--|----------|----------|---|---|---|---|---|---|---|---|----|----|----|----------|--|----------|
| Execution Time | | sec/yr | 53.97 | | | | | | | | | | | | 55.57 | | 57.37 |
| Run Date | | | 10/19/07 | | | | | | | | | | | | 10/19/07 | | 10/19/07 |
| Starting Date for Simulation | | | 05/01/65 | | | | | | | | | | | | 05/01/65 | | 05/01/65 |
| Starting Date for Output | | | 05/01/65 | | | | | | | | | | | | 05/01/65 | | 05/01/65 |
| Ending Date | | | 04/30/00 | | | | | | | | | | | | 04/30/00 | | 04/30/00 |
| Output Duration | | days | 12784 | | | | | | | | | | | | 12784 | | 12784 |
| Cell Label | | | EDCE | | | | | | | | | | | | 1 | | 2 |
| Downstream Cell Label | | | 1 | | | | | | | | | | | | 2 | | EDCW |
| Network Simulation Name | | | none | | | | | | | | | | | | none | | 4N |
| Simulation Type | | | Uncerta | | | | | | | | | | | | Uncerta | | 4S |
| Surface Area | | km2 | 0.95 | | | | | | | | | | | | 2.25 | | 2.81 |
| Mean Rainfall | | cm/yr | 142.94 | | | | | | | | | | | | 142.94 | | 142.94 |
| Mean ET | | cm/yr | 129.66 | | | | | | | | | | | | 129.66 | | 129.66 |
| Cell Inflow Volume | | hm3/yr | 51.7 | | | | | | | | | | | | 63.4 | | 57.1 |
| Cell Inflow Load | | kg/yr | 8665.47 | | | | | | | | | | | | 5367.2 | | 15484.8 |
| Cell Inflow Conc | | ppb | 175 | | | | | | | | | | | | 137 | | 94 |
| Treated Outflow Volume | | hm3/yr | 63.4 | | | | | | | | | | | | 57.1 | | 52.3 |
| Treated Outflow Load | | kg/yr | 8665.7 | | | | | | | | | | | | 5367.2 | | 15484.8 |
| Treated FWM Outflow Conc | | ppb | 137 | | | | | | | | | | | | 94 | | 30 |
| Upper Confidence Limit | | ppb | 136.7 | | | | | | | | | | | | 102.4 | | 168.8 |
| Lower Confidence Limit | | ppb | 136.6 | | | | | | | | | | | | 84.3 | | 168.8 |
| Total Outflow Volume + Bypass | | hm3/yr | 63.4 | | | | | | | | | | | | 57.1 | | 52.3 |
| Total Outflow Load | | kg/yr | 8665.7 | | | | | | | | | | | | 5367.2 | | 15484.8 |
| Total FWM Outflow Conc | | ppb | 136.7 | | | | | | | | | | | | 93.9 | | 129.0 |
| Bypass Load | | kg/yr | 0 | | | | | | | | | | | | 0 | | 0 |
| Bypass Load | | % | 0% | | | | | | | | | | | | 0% | | 0% |
| Maximum Inflow | | hm3/d | 0.25 | | | | | | | | | | | | 0.28 | | 0.44 |
| Maximum Inflow | | hm3/d | 0.28 | | | | | | | | | | | | 0.27 | | 0.44 |
| Surface Load Reduction | | kg/yr | 744 | | | | | | | | | | | | 3299 | | 3821 |
| Load Trapped in Sediments | | kg/yr | 0 | | | | | | | | | | | | 2889 | | 3640 |
| Overall Load Reduction | | % | 4% | | | | | | | | | | | | 38% | | 71% |
| Lower Confidence Limit | | % | 0.0 | | | | | | | | | | | | 0.0 | | 0.2 |
| Upper Confidence Limit | | % | 0.0 | | | | | | | | | | | | 0.4 | | 0.3 |
| Daily Geometric Mean | | ppb | 102.8 | | | | | | | | | | | | 88.3 | | 23.9 |
| Outflow Geo Mean - Composites | | ppb | 134.8 | | | | | | | | | | | | 91.5 | | 27.1 |
| Upper Confidence Limit | | ppb | 134.8 | | | | | | | | | | | | 100.2 | | 35.1 |
| Lower Confidence Limit | | ppb | 134.7 | | | | | | | | | | | | 81.7 | | 20.5 |
| Frequency Outflow Conc > 10 ppb | | % | 100% | | | | | | | | | | | | 100% | | 100% |
| Frequency Outflow Conc > 20 ppb | | % | 100% | | | | | | | | | | | | 100% | | 83% |
| Frequency Outflow Conc > 50 ppb | | % | 100% | | | | | | | | | | | | 100% | | 100% |
| 95th Percentile Outflow Conc | | ppb | 150.74 | | | | | | | | | | | | 108.53 | | 39.96 |
| Mean Biomass P Storage | | mg/m2 | 737 | | | | | | | | | | | | 3758 | | 1639 |
| Storage Increase / Net Removal | | % | 2069% | | | | | | | | | | | | 0% | | 2686% |
| Net Storage Turnover Rate | | 1/yr | 0.0 | | | | | | | | | | | | 11.1 | | 34.8 |
| Unit Area P Load | | mg/m2-yr | 9511 | | | | | | | | | | | | 3850 | | 2402 |
| Unit Area P Removal | | mg/m2-yr | 0 | | | | | | | | | | | | 1194 | | 1629 |
| Mean Water Load | | cm/d | 14.9 | | | | | | | | | | | | 7.7 | | 7.0 |
| Max Water Load | | cm/d | 25.9 | | | | | | | | | | | | 12.5 | | 46.5 |
| Mean Depth | | cm | 68 | | | | | | | | | | | | 53 | | 51 |
| Minimum Depth | | cm | 58.8 | | | | | | | | | | | | 46.2 | | 92.8 |
| Maximum Depth | | cm | 76.1 | | | | | | | | | | | | 60.2 | | 97.8 |
| Frequency Depth < 10 cm | | % | 0% | | | | | | | | | | | | 0% | | 0% |
| FlowWidth | | m2/day | 214 | | | | | | | | | | | | 112 | | 101 |
| HRT Days | | days | 4.6 | | | | | | | | | | | | 6.9 | | 7.3 |
| Mean Velocity | | cm/sec | 0.37 | | | | | | | | | | | | 0.24 | | 0.47 |
| Seepage Outflow / Total Outflow | | % | 0% | | | | | | | | | | | | 0% | | 0% |
| Release 1 Outflow Volume | | hm3/yr | 0.00 | | | | | | | | | | | | 0.00 | | 0.00 |
| Release 2 Outflow Volume | | hm3/yr | 0.00 | | | | | | | | | | | | 0.00 | | 0.00 |
| 95th Percentile Outflow Volume | | hm3/d | 0.2 | | | | | | | | | | | | 0.2 | | 0.4 |
| 95th Percentile Outflow Load | | kg/d | 35.8 | | | | | | | | | | | | 23.8 | | 8.2 |
| Simulated / Specified Mean Depth | | % | #N/A | | | | | | | | | | | | #N/A | | #N/A |
| Release 1 Demand Met | | % | #N/A | | | | | | | | | | | | #N/A | | #N/A |
| Release 2 Demand Met | | % | #N/A | | | | | | | | | | | | #N/A | | #N/A |
| Outflow Demand Met | | % | #N/A | | | | | | | | | | | | #N/A | | #N/A |
| Range Check - Mean Depth | | | | | | | | | | | | | | | 0.82 | | 0.94 |
| Range Check - Freq Depth < 10 cm | | | | | | | | | | | | | | | 0.62 | | 0 |
| Range Check - FlowWidth | | | | | | | | | | | | | | | | | 0 |
| Range Check - Inflow Conc | | | | | | | | | | | | | | | | | 0 |
| Range Check - Outflow Conc | | | | | | | | | | | | | | | | | 0 |
| Water Balance Error | | % | 0.00% | | | | | | | | | | | | 0.00% | | 0.00% |
| Mass Balance Error | | % | 0.00% | | | | | | | | | | | | 0.01% | | 0.02% |
| Warning or Error Messages | | | | | | | | | | | | | | | | | 4 |



STA-2 With PLR of 1.0 g/m2/yr

| DMSTA2- Inputs & Outputs | | Project: STA 2 EIS | | Model Release: 07/05/07 | | |
|---------------------------------------|--------------|--|---|---|---------------------------------|---|
| | | | | Current Date: 10/14/2007 | | |
| Input Variable | Units | Value | Case Description: | | | |
| Design Case Name | - | EIS A#1 | Analysis for WY 1066-2000 | | | |
| Input Series Name | - | TS_EIS_A#1 | Inflow time series includes allowance of 38 cfs (27,500 ac-ft/yr) seepage from WCA-2A to Supply Canal | | | |
| Starting Date for Simulation | - | 05/01/65 | PLR = 1.0 g/m2/yr | | | |
| Ending Date for Simulation | - | 04/30/00 | | | | |
| Starting Date for Output | - | 05/01/65 | | | | |
| Integration Steps Per Day | - | 4 | | | | |
| Number of Iterations | - | 0 | | | | |
| Output Averaging Interval | days | 365 | | | | |
| Inflow Conc Scale Factor | - | 1 | | | | |
| Rainfall P Conc | ppb | 10 | | | | |
| Atmospheric P Load (Dry) | mg/m2-yr | 20 | | | | |
| | | Simulation Type: | | Uncertainty Analysis | | |
| | | Output Variable | | Mean | Lower Cl Upper Cl | |
| | | FWM Outflow C (ppb) | | 22.4 | 26.9 18.9 | |
| | | GM Outflow C (ppb) | | 20.5 | 24.8 17.2 | |
| | | Load Reduction % | | 80% | 76% 83% | |
| | | Bypass Load (%) | | 0.0% | | |
| | | | | Diagnostics | | |
| | | | | H2O Balance Error Mean & Max 0.0% 0.0% | | |
| | | | | Mass Balance Error Mean & Max 0.1% 0.2% | | |
| | | | | Iterations & Convergence 2 0.2% | | |
| | | | | Warning/Error Messages 4 | | |
| Cell Number -> | | 1 | 2 | 3 | 4 5 6 7 8 9 10 11 12 | |
| Cell Label | -> | PEW_3 | PEW_3 | SAV_3 | | |
| Vegetation Type | -> | | | | | |
| Inflow Fraction | -> | 0.283710055 | 0.358144973 | 0.358144973 | | |
| Downstream Cell Number | - | | | | | |
| Surface Area | km2 | 7.28 | 9.19 | 9.19 | | |
| Mean Width of Flow Path | km | 1.58 | 2.00 | 2.00 | | |
| Number of Tanks in Series | - | 3.0 | 3.0 | 6.0 | | |
| Minimum Depth for Releases | cm | | | | | |
| Release 1 Series Name | - | | | | | |
| Release 2 Series Name | - | | | | | |
| Outflow Series Name | - | | | | | |
| Depth Series Name | - | | | | | |
| Outflow Control Depth | cm | 52.73 | 29.26 | 29.87 | | |
| Outflow Weir Depth | cm | | | | | |
| Outflow Coefficient - Exponent | - | 4 | 4 | 4 | | |
| Outflow Coefficient - Intercept | - | 1 | 1 | 1 | | |
| Bypass Depth | cm | | | | | |
| Maximum Inflow | hm3/day | | | | | |
| Maximum Outflow | hm3/day | | | | | |
| Inflow Seepage Rate | (cm/d) / cm | 0.008 | | | | |
| Inflow Seepage Control Elev | cm | 78 | | | | |
| Inflow Seepage Conc | ppb | 20 | | | | |
| Outflow Seepage Rate | (cm/d) / cm | 0.004 | 0.006 | 0.00337 | | |
| Outflow Seepage Control Elev | cm | -61 | -61 | -30 | | |
| Max Outflow Seepage Conc | ppb | 20 | 20 | 20 | | |
| Seepage Recycle to Cell Number | - | 1 | 2 | 3 | | |
| Seepage Recycle Fraction | - | 1 | 1 | 1 | | |
| Initial Water Column Conc | ppb | 30 | 30 | 30 | | |
| Initial P Storage Per Unit Area | mg/m2 | 500 | 500 | 500 | | |
| Initial Water Column Depth | cm | 200 | 200 | 200 | | |
| C0 = Conc at 0 g/m2 P Storage | ppb | 3 | 3 | 3 | | |
| C1 = Conc at 1 g/m2 P storage | ppb | 22 | 22 | 22 | | |
| C2 = Conc at Half-Max Uptake | ppb | 300 | 300 | 300 | | |
| K = Net Settling Rate at Steady State | m/yr | 34.9 | 34.9 | 52.5 | | |
| Z1 = Saturated Uptake Depth | cm | 40 | 40 | 40 | | |
| Z2 = Lower Penalty Depth | cm | 100 | 100 | 100 | | |
| Z3 = Upper Penalty Depth | cm | 200 | 200 | 200 | | |
| Output Variables | Units | 1 | 2 | 3 | Overall | |
| Execution Time | sec/yr | 3.74 | 4.46 | 5.60 | 5.60 | |
| Run Date | - | 10/14/07 | 10/14/07 | 10/14/07 | 10/14/07 | |
| Starting Date for Simulation | - | 05/01/65 | 05/01/65 | 05/01/65 | 05/01/65 | |
| Ending Date for Output | - | 04/30/00 | 04/30/00 | 04/30/00 | 04/30/00 | |
| Output Duration | days | 12784 | 12784 | 12784 | 12784 | |
| Cell Label | - | 1 | 2 | 3 | Total | |
| Downstream Cell Label | - | Outflow | Outflow | Outflow | - | |
| Network Simulation Name | - | none | none | none | none | |
| Simulation Type | - | Uncerta | Uncerta | Uncerta | Uncerta | |
| Surface Area | km2 | 7.28 | 9.19 | 9.19 | 25.66 | |
| Mean Rainfall | cm/yr | 128.37 | 128.37 | 128.37 | 129.4 | |
| Mean ET | cm/yr | 128.37 | 128.37 | 128.37 | 129.4 | |
| Cell Inflow Volume | hm3/yr | 64.0 | 80.8 | 80.8 | 225.5 | |
| Cell Inflow Load | kg/yr | 7286.0 | 9197.5 | 9197.5 | 25681 | |
| Cell Inflow Conc | ppb | 114 | 114 | 114 | 113.9 | |
| Treated Outflow Volume | hm3/yr | 67.9 | 80.8 | 80.8 | 229.5 | |
| Treated Outflow Load | kg/yr | 1732.0 | 2082.1 | 1330.8 | 5145 | |
| Treated FWM Outflow Conc | ppb | 25 | 26 | 16 | 22.4 | |
| Upper Confidence Limit | ppb | 31.1 | 31.4 | 18.9 | 26.9 | |
| Lower Confidence Limit | ppb | 21.1 | 21.4 | 14.7 | 18.9 | |
| Total Outflow Volume + Bypass | hm3/yr | 67.9 | 80.8 | 80.8 | 229.5 | |
| Total Outflow Load + Bypass | kg/yr | 1732.0 | 2082.1 | 1330.8 | 5144.9 | |
| Total FWM Outflow Conc | ppb | 25.5 | 25.8 | 16.5 | 22.4 | |
| Bypass Load | kg/yr | 0 | 0 | 0 | 0.0 | |
| Bypass Load % | % | 0% | 0% | 0% | 0.0 | |
| Maximum Inflow | hm3/d | 0.34 | 0.43 | 0.43 | 1.21 | |
| Maximum Outflow | hm3/d | 0.35 | 0.44 | 0.44 | 1.23 | |
| Surface Load Reduction | kg/yr | 5783 | 7523 | 8000 | 20536 | |
| Load Trapped in Sediments | kg/yr | 5729 | 7150 | 8066 | 20944 | |
| Overall Load Reduction | % | 76% | 77% | 86% | 80% | |
| Lower Confidence Limit | % | 0.7 | 0.7 | 0.8 | 76% | |
| Upper Confidence Limit | % | 0.8 | 0.8 | 0.9 | 83% | |
| Daily Geometric Mean | ppb | 14.6 | 15.2 | 6.6 | #N/A | |
| Outflow Geo Mean - Composites | ppb | 23.4 | 23.6 | 14.7 | 20.5 | |
| Upper Confidence Limit | ppb | 28.9 | 29.1 | 16.9 | 24.8 | |
| Lower Confidence Limit | ppb | 19.2 | 19.4 | 13.1 | 17.2 | |
| Frequency Outflow Conc > 10 ppb | % | 100% | 100% | 92% | 100% | |
| Frequency Outflow Conc > 20 ppb | % | 69% | 75% | 19% | 92% | |
| Frequency Outflow Conc > 50 ppb | % | 0% | 0% | 0% | 53% | |
| Freq Outflow Volume > 10 ppb | % | 97% | 96% | 60% | 90% | |
| 95th Percentile Outflow Conc | ppb | 31.74 | 32.52 | 21.33 | 29 | |
| Mean Biomass P Storage | mg/m2 | 1186 | 1172 | 879 | 1071 | |
| Storage Increase / Net Removal | % | 0% | 0% | 0% | 0% | |
| Net Storage Turnover Rate | 1/yr | 23.2 | 23.2 | 34.9 | 26.7 | |
| Unit Area P Load | mg/m2-yr | 1001 | 1001 | 1001 | 1001 | |
| Unit Area P Removal | mg/m2-yr | 787 | 778 | 878 | 816 | |
| Mean Water Load | cm/d | 2.4 | 2.4 | 2.4 | 2.4 | |
| Max Water Load | cm/d | 4.7 | 4.7 | 4.7 | 4.7 | |
| Mean Depth | cm | 58 | 50 | 50 | 52 | |
| Minimum Depth | cm | 55.2 | 41.4 | 41.4 | 45 | |
| Maximum Depth | cm | 64.4 | 61.0 | 61.0 | 62 | |
| Frequency Depth < 10 cm | % | 0% | 0% | 0% | 0.0% | |
| Flow/Width | m2/day | 111 | 111 | 111 | 110.7 | |
| HRT Days | days | 24.2 | 20.6 | 20.6 | 21.7 | |
| Mean Velocity | cm/sec | 0.22 | 0.26 | 0.26 | 0.25 | |
| Seepage Outflow / Total Outflow | % | 0% | 0% | 0% | 0% | |
| Release 1 Outflow Volume | hm3/yr | 0.00 | 0.00 | 0.00 | 0.0 | |
| Release 2 Outflow Volume | hm3/yr | 0.00 | 0.00 | 0.00 | 0.0 | |
| 95th Percentile Outflow Volume | hm3/d | 0.3 | 0.4 | 0.4 | 1.1 | |
| 95th Percentile Outflow Load | kg/d | 10.0 | 12.3 | 8.1 | 30.4 | |
| Simulated / Specified Mean Depth | % | #N/A | #N/A | #N/A | #N/A | |
| Release 1 Demand Met | % | #N/A | #N/A | #N/A | #N/A | |
| Release 2 Demand Met | % | #N/A | #N/A | #N/A | #N/A | |
| Outflow Demand Met | % | #N/A | #N/A | #N/A | #N/A | |
| Range Check - Mean Depth | - | | | 0.80 | 1 | |
| Range Check - Freq Depth < 10 cm | - | | | | 0 | |
| Range Check - Flow/Width | - | | | 0.68 | 1 | |
| Range Check - Inflow Conc | - | | | | 2 | |
| Range Check - Outflow Conc | - | 1.04 | 1.04 | | 0 | |
| Water Balance Error | % | 0.04% | 0.04% | 0.22% | 0.00% | |
| Mass Balance Error | % | 0.04% | 0.04% | 0.22% | 0.11% | |
| Warning or Error Messages | | Cell 1 Inflow Conc out of calib. range for PEW_3: 114 vs. 8 - 110 ppb Cell 2 Inflow Conc out of calib. range for PEW_3: 114 vs. 8 - 110 ppb Cell 3 Depth out of calib. range for SAV_3: 50 vs. 62 - 87 cm Cell 3 Flow/Width out of calib. range for SAV_3: 111 vs. 162 - 374 m2/day | | | | 4 |



Compartment B North Build-out With STA-2 PLR of 1.3 g/m2/yr

| DMSTA2- Inputs & Outputs | | Project: COMP B EIS | | | | | | | | | | | | Model Release: | 07/05/07 | | |
|---------------------------------------|--------------|---|--|----------|----------|----------|----------|----------|----------|----------|-----------|-------------------------------|-----------|----------------|-------------------------------|--|----------|
| | | | | | | | | | | | | | | Current Date: | 10/15/2007 | | |
| Input Variable | Units | Value | Class Description: | | | | | | | | | | | | | | |
| Design Case Name | | BO EIS Alt 1 | Analysis for WY 1966-2000 | | | | | | | | | | | | | | |
| Input Series Name | | TS_NBO_EIS_Alt | Receives re-directed inflow from STA-2 up to 1,000 cfs | | | | | | | | | | | | | | |
| Starting Date for Simulation | | 05/01/65 | | | | | | | | | | | | | | | |
| Ending Date for Simulation | | 04/30/00 | | | | | | | | | | | | | | | |
| Starting Date for Output | | 05/01/65 | | | | | | | | | | | | | | | |
| Integration Steps Per Day | | 4 | Simulation Type: Uncertainty Analysis | | | | | | | | | | | | | | |
| Number of Iterations | | 0 | Output Variable | | | | | | | | | | | | | | |
| Output Averaging Interval | days | 365 | FWM Outflow C (ppb) | 9.3 | 10.9 | 8.1 | | | | | | Diagnosics | | | | | |
| Inflow Conc Scale Factor | | 1 | GM Outflow C (ppb) | 8.7 | 10.1 | 7.6 | | | | | | R2D Balance Error Mean & Max | 0.0% | 0.0% | | | |
| Rainfall P Conc | ppb | 10 | Load Reduction % | 91% | 89% | 92% | | | | | | Mass Balance Error Mean & Max | 0.1% | 0.0% | | | |
| Atmospheric P Load (Dry) | mg/m2-yr | 20 | Bypass Load (%) | 0.0% | | | | | | | | Iterations & Convergence | 3 | 0.0% | | | |
| Cell Number --> | | | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | Warning/Error Messages | | |
| Cell Label | | NBO1 | NBO2 | Cell 4 | | | | | | | | | | | | | |
| Vegetation Type | --> | EMG_3 | SAV_3 | SAV_3 | | | | | | | | | | | | | |
| Inflow Fraction | | 1 | | | | | | | | | | | | | | | |
| Downstream Cell Number | | 2 | 3 | | | | | | | | | | | | | | |
| Surface Area | km2 | 4.35 | 11.53 | 7.70 | | | | | | | | | | | | | |
| Mean Width of Flow Path | km | 6.50 | 5.00 | 2.50 | | | | | | | | | | | | | |
| Number of Tanks in Series | | 3.0 | 3.0 | 3.0 | | | | | | | | | | | | | |
| Minimum Depth for Releases | cm | | | | | | | | | | | | | | | | |
| Release 1 Series Name | | | | | | | | | | | | | | | | | |
| Release 2 Series Name | | | | | | | | | | | | | | | | | |
| Outflow Series Name | | | | | | | | | | | | | | | | | |
| Depth Series Name | | | | | | | | | | | | | | | | | |
| Outflow Control Depth | cm | 38.1 | 38.1 | 42.7 | | | | | | | | | | | | | |
| Outflow Weir Depth | cm | | | | | | | | | | | | | | | | |
| Outflow Coefficient - Exponent | | 4 | 4 | 4 | | | | | | | | | | | | | |
| Outflow Coefficient - Intercept | | 1 | 1 | 1 | | | | | | | | | | | | | |
| Bypass Depth | cm | | | | | | | | | | | | | | | | |
| Maximum Inflow | hm3/day | | | | | | | | | | | | | | | | |
| Maximum Outflow | hm3/day | | | | | | | | | | | | | | | | |
| Inflow Seepage Rate | (cm/d) / cm | 0.033 | 0.0124 | 0.0064 | | | | | | | | | | | | | |
| Inflow Seepage Control Elev | cm | 48 | 48 | 60 | | | | | | | | | | | | | |
| Inflow Seepage Conc | ppb | 75 | 75 | 75 | | | | | | | | | | | | | |
| Outflow Seepage Rate | (cm/d) / cm | 0.0045 | 0.0017 | | | | | | | | | | | | | | |
| Outflow Seepage Control Elev | cm | 42 | 42 | | | | | | | | | | | | | | |
| Max Outflow Seepage Conc | ppb | 20 | 20 | | | | | | | | | | | | | | |
| Seepage Recycle to Cell Number | | 1 | 1 | | | | | | | | | | | | | | |
| Seepage Recycle Fraction | | 0.75 | 0.75 | | | | | | | | | | | | | | |
| Seepage Discharge Fraction | | | | | | | | | | | | | | | | | |
| Initial Water Column Conc | ppb | 30 | 30 | 30 | | | | | | | | | | | | | |
| Initial P Storage Per Unit Area | mg/m2 | 500 | 500 | 500 | | | | | | | | | | | | | |
| Initial Water Column Depth | cm | 200 | 200 | 200 | | | | | | | | | | | | | |
| C0 = Conc at 0 g/m2 P Storage | ppb | 3 | 3 | | | | | | | | | | | | | | |
| C1 = Conc at 1 g/m2 P storage | ppb | 22 | 22 | | | | | | | | | | | | | | |
| C2 = Conc at Half-Max Uptake | ppb | 300 | 300 | | | | | | | | | | | | | | |
| K = Net Settling Rate at Steady State | m/yr | 16.8 | 52.5 | 52.5 | | | | | | | | | | | | | |
| Z1 = Saturated Uptake Depth | cm | 40 | 40 | | | | | | | | | | | | | | |
| Z2 = Lower Penalty Depth | cm | 100 | 100 | | | | | | | | | | | | | | |
| Z3 = Upper Penalty Depth | cm | 200 | 200 | | | | | | | | | | | | | | |
| Output Variables | Units | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | Overall | | | |
| Execution Time | sec/yr | 5.31 | 6.00 | 6.71 | | | | | | | | | | | | | 6.71 |
| Run Date | | 10/15/07 | 10/15/07 | 10/15/07 | | | | | | | | | | | | | 10/15/07 |
| Starting Date for Simulation | | 05/01/65 | 05/01/65 | 05/01/65 | | | | | | | | | | | | | 05/01/65 |
| Starting Date for Output | | 05/01/65 | 05/01/65 | 05/01/65 | | | | | | | | | | | | | 05/01/65 |
| Ending Date | | 04/30/00 | 04/30/00 | 04/30/00 | | | | | | | | | | | | | 04/30/00 |
| Output Duration | days | 12784 | 12784 | 12784 | | | | | | | | | | | | | 12784 |
| Cell Label | | NBO1 | NBO2 | Cell 4 | | | | | | | | | | | | | Total |
| Downstream Cell Label | | | | Outflow | | | | | | | | | | | | | |
| Network Simulation Name | | none | none | none | | | | | | | | | | | | | none |
| Simulation Type | | Uncerta | Uncerta | Uncerta | | | | | | | | | | | | | Uncerta |
| Surface Area | km2 | 4.35 | 11.53 | 7.70 | | | | | | | | | | | | | 23.58 |
| Mean Rainfall | cm/yr | 128.59 | 128.59 | 128.59 | | | | | | | | | | | | | 128.6 |
| Mean ET | cm/yr | 133.76 | 133.76 | 133.76 | | | | | | | | | | | | | 133.8 |
| Cell Inflow Volume | hm3/yr | 147.7 | 151.1 | 152.3 | | | | | | | | | | | | | 147.7 |
| Cell Inflow Conc | kg/yr | 15252.9 | 11490.9 | 2522.9 | | | | | | | | | | | | | 15253 |
| Treated Outflow Volume | hm3/yr | 103 | 76 | 17 | | | | | | | | | | | | | 103.3 |
| Treated Outflow Load | kg/yr | 151.1 | 152.3 | 153.4 | | | | | | | | | | | | | 153.4 |
| Treated FWM Outflow Conc | ppb | 11490.9 | 2522.9 | 1419.8 | | | | | | | | | | | | | 1420 |
| Upper Confidence Limit | ppb | 81.2 | 20.1 | 10.9 | | | | | | | | | | | | | 10.9 |
| Lower Confidence Limit | ppb | 70.1 | 13.8 | 8.1 | | | | | | | | | | | | | 8.1 |
| Total Outflow Volume + Bypass | hm3/yr | 151.1 | 152.3 | 153.4 | | | | | | | | | | | | | 153.4 |
| Total Outflow Load + Bypass | kg/yr | 11490.9 | 2522.9 | 1419.8 | | | | | | | | | | | | | 1419.8 |
| Total FWM Outflow Conc | ppb | 76.1 | 16.6 | 9.3 | | | | | | | | | | | | | 9.3 |
| Bypass Load | kg/yr | 0 | 0 | 0 | | | | | | | | | | | | | 0.0 |
| Bypass Load | % | 0% | 0% | 0% | | | | | | | | | | | | | 0.0 |
| Maximum Inflow | hm3/d | 0.82 | 0.84 | 0.85 | | | | | | | | | | | | | 0.82 |
| Maximum Outflow | hm3/d | 0.84 | 0.85 | 0.86 | | | | | | | | | | | | | 0.86 |
| Surface Load Reduction | kg/yr | 3777 | 8965 | 1103 | | | | | | | | | | | | | 13633 |
| Load Trapped in Sediments | kg/yr | 4118 | 9514 | 1466 | | | | | | | | | | | | | 15009 |
| Overall Load Reduction | % | 25% | 78% | 44% | | | | | | | | | | | | | 91% |
| Lower Confidence Limit | % | 0.2 | 0.8 | 0.5 | | | | | | | | | | | | | 89% |
| Upper Confidence Limit | % | 0.3 | 0.8 | 0.4 | | | | | | | | | | | | | 92% |
| Daily Geometric Mean | ppb | 68.0 | 10.0 | 5.3 | | | | | | | | | | | | | #N/A |
| Outflow Geo Mean - Composites | ppb | 74.8 | 15.3 | 8.7 | | | | | | | | | | | | | 8.7 |
| Upper Confidence Limit | ppb | 80.3 | 18.6 | 10.1 | | | | | | | | | | | | | 10.1 |
| Lower Confidence Limit | ppb | 68.5 | 12.8 | 7.6 | | | | | | | | | | | | | 7.6 |
| Frequency Outflow Conc > 10 ppb | % | 100% | 97% | 22% | | | | | | | | | | | | | 22% |
| Frequency Outflow Conc > 20 ppb | % | 100% | 8% | 0% | | | | | | | | | | | | | 0% |
| Frequency Outflow Conc > 50 ppb | % | 100% | 0% | 0% | | | | | | | | | | | | | 0% |
| Freq Outflow Volume > 10 ppb | % | 100% | 82% | 40% | | | | | | | | | | | | | 40% |
| 95th Percentile Outflow Conc | ppb | 81.41 | 20.30 | 11.51 | | | | | | | | | | | | | 12 |
| Mean Biomass P Storage | mg/m2 | 2971 | 825 | 190 | | | | | | | | | | | | | 1013 |
| Storage Increase / Net Removal | % | 0% | 0% | 0% | | | | | | | | | | | | | 0% |
| Net Storage Turnover Rate | 1/yr | 11.2 | 26.0 | 26.1 | | | | | | | | | | | | | 22.1 |
| Unit Area P Load | mg/m2-yr | 3506 | 997 | 328 | | | | | | | | | | | | | 647 |
| Unit Area P Removal | mg/m2-yr | 947 | 825 | 190 | | | | | | | | | | | | | 640 |
| Mean Water Load | cm/d | 9.3 | 3.6 | 5.4 | | | | | | | | | | | | | 1.7 |
| Max Water Load | cm/d | 18.9 | 7.3 | 11.0 | | | | | | | | | | | | | 3.5 |
| Mean Depth | cm | 46 | 49 | 58 | | | | | | | | | | | | | 51 |
| Minimum Depth | cm | 38.4 | 44.1 | 50.4 | | | | | | | | | | | | | 45 |
| Maximum Depth | cm | 53.2 | 58.5 | 70.6 | | | | | | | | | | | | | 61 |
| Frequency Depth < 10 cm | % | 0% | 0% | 0% | | | | | | | | | | | | | 0.0% |
| Flow/Width | m2/day | 62 | 83 | 167 | | | | | | | | | | | | | 106.4 |
| HRT Days | days | 4.9 | 13.6 | 10.7 | | | | | | | | | | | | | 29.9 |
| Mean Velocity | cm/sec | 0.16 | 0.29 | 0.33 | | | | | | | | | | | | | 0.23 |
| Seepage Outflow / Total Outflow | % | 0% | 0% | 0% | | | | | | | | | | | | | 0% |
| Release 1 Outflow Volume | hm3/yr | 0.00 | 0.00 | 0.00 | | | | | | | | | | | | | 0.0 |
| Release 2 Outflow Volume | hm3/yr | 0.00 | 0.00 | 0.00 | | | | | | | | | | | | | 0.0 |
| 95th Percentile Outflow Volume | hm3/d | 0.7 | 0.7 | 0.7 | | | | | | | | | | | | | 0.7 |
| 95th Percentile Outflow Load | kg/d | 54.8 | 14.0 | 8.0 | | | | | | | | | | | | | 8.0 |
| Simulated / Specified Mean Depth | % | #N/A | #N/A | #N/A | | | | | | | | | | | | | #N/A |
| Release 1 Demand Met | % | #N/A | #N/A | #N/A | | | | | | | | | | | | | #N/A |
| Release 2 Demand Met | % | #N/A | #N/A | #N/A | | | | | | | | | | | | | #N/A |
| Outflow Demand Met | % | #N/A | #N/A | #N/A | | | | | | | | | | | | | #N/A |
| Range Check - Mean Depth | | | 0.79 | 0.93 | | | | | | | | | | | | | 2 |
| Range Check - Freq Depth < 10 cm | | | | | | | | | | | | | | | | | 0 |
| Range Check - Flow/Width | | | 0.51 | | | | | | | | | | | | | | 1 |
| Range Check - Inflow Conc | | | | | | | | | | | | | | | | | 0 |
| Range Check - Outflow Conc | | | | 0.62 | | | | | | | | | | | | | 1 |
| Water Balance Error | % | 0.00% | 0.00% | 0.00% | | | | | | | | | | | | | 0.00% |
| Mass Balance Error | % | 0.05% | 0.02% | 0.03% | | | | | | | | | | | | | 0.06% |
| Warning or Error Messages | | Cell 2 Depth out of calb. range for SAV_3: 49 vs. 62 - 87 cm Cell 2 Flow/Width out of calb. range for SAV_3: 83 vs. 162 - 374 m2/day Cell 3 Depth out of calb. range for SAV_3: 58 vs. 62 - 87 cm Cell 3 Outflow Conc out of calb. range for SAV_3: 9 vs. 15 - 153 ppb | | | | | | | | | | | | 4 | | | |



EAASR With Lake Okeechobee TP=100 ppb

| DMST42- Inputs & Outputs | | Project: PROJECT_EAASR_STA34_NETWORK_EIS | Model Release: 07/05/07 |
|---------------------------------------|--------------|--|---|
| | | | Current Date: 10/16/2007 |
| Input Variable | Units | Value | Case Description: |
| Design Case Name | - | A1 | Compartment A-1 |
| Input Series Name | - | TS_A1 | 15,200 acres (from A8) |
| Starting Date for Simulation | - | 01/01/65 | Inflow volumes, outflow volumes, and depths from SFWMM simulation |
| Ending Date for Simulation | - | 04/30/00 | Reservoir settling rate = 5.0 m/yr (Goforth 2006) |
| Starting Date for Output | - | 05/01/65 | Lake Okeechobee deliveries at 100 ppb |
| Integration Steps Per Day | - | 4 | |
| Number of Iterations | - | 0 | Simulation Type: |
| Output Averaging Interval | days | 1 | Output Variable |
| Inflow Conc Scale Factor | - | 1 | FWM Outflow C (ppb) |
| Rainfall P Conc | ppb | 10 | GM Outflow C (ppb) |
| Atmospheric P Load (Dry) | mg/m2-yr | 20 | Load Reduction (%) |
| | | | Bypass Load (%) |
| | | | H2O Balance Error Mean & Max |
| | | | Mass Balance Error Mean & Max |
| | | | Iterations & Convergence |
| | | | Warning/Error Messages |
| Cell Number --> | | 1 | |
| Cell Label | - | 1 | |
| Vegetation Type | -> | none | |
| Inflow Fraction | - | 1 | |
| Downstream Cell Number | - | | |
| Surface Area | km2 | 61.54 | |
| Mean Width of Flow P1 | km | 6.70 | |
| Number of Tanks in Series | - | 1.0 | |
| Minimum Depth for Releases | cm | 15 | |
| Release 1 Series Name | - | IRRIG | |
| Release 2 Series Name | - | TO_STA | |
| Outflow Series Name | - | A1_DEPTH | |
| Depth Series Name | - | | |
| Outflow Control Depth | cm | | |
| Outflow Weir Depth | cm | | |
| Outflow Coefficient - Exponent | - | 1.5 | |
| Outflow Coefficient - Intercept | - | 8 | |
| Bypass Depth | cm | | |
| Maximum Inflow | hm3/day | | |
| Maximum Outflow | hm3/day | 9.05233 | |
| Inflow Seepage Rate | (cm/d) / cm | | |
| Inflow Seepage Control Elev | cm | | |
| Inflow Seepage Conc | ppb | | |
| Outflow Seepage Rate | (cm/d) / cm | 0.00081 | |
| Outflow Seepage Control Elev | cm | 6 | |
| Max Outflow Seepage Conc | ppb | 100 | |
| Seepage Recycle to Cell Number | - | 1 | |
| Seepage Recycle Fraction | - | 0.75 | |
| Seepage Discharge Fraction | - | | |
| Initial Water Column Conc | ppb | 105 | |
| Initial P Storage Per Unit Area | mg/m2 | 525 | |
| Initial Water Column Depth | cm | 150 | |
| C0 = Conc at 0 gm2 P Storage | ppb | 3 | |
| C1 = Conc at 1 gm2 P storage | ppb | 150 | |
| C2 = Conc at Half-Max Uptake | ppb | | |
| K = Net Settling Rate at Steady State | m/yr | 5.0 | |
| Z1 = Saturated Uptake Depth | cm | 40 | |
| Z2 = Lower Penalty Depth | cm | 100 | |
| Z3 = Upper Penalty Depth | cm | 400 | |
| Output Variables | Units | 1 | 2 |
| Execution Time | sec/yr | 1.53 | |
| Run Date | - | 10/16/07 | |
| Starting Date for Simulation | - | 01/01/65 | |
| Starting Date for Output | - | 05/01/65 | |
| Ending Date | - | 04/30/00 | |
| Output Duration | days | 12784 | |
| Cell Label | - | 1 | |
| Downstream Cell Label | - | Outflow | |
| Network Simulation Label | - | EAASR_NET | |
| Simulation Type | - | Base | |
| Surface Area | km2 | 61.54 | |
| Mean Rainfall | cm/yr | 130.73 | |
| Mean ET | cm/yr | 125.71 | |
| Cell Inflow Volume | hm3/yr | 666.6 | 540417.2 |
| Cell Inflow Load | kg/yr | 63931.8 | |
| Cell Inflow Conc | ppb | 96 | |
| Treated Outflow Volume | hm3/yr | 470.5 | 381414.6 |
| Treated Outflow Load | kg/yr | 39773.0 | |
| Treated FWM Outflow Conc | ppb | 85 | |
| Upper Confidence Limit | ppb | #N/A | |
| Lower Confidence Limit | ppb | #N/A | |
| Total Outflow Volume + Bypass | hm3/yr | 656.9 | |
| Total Outflow Load + Bypass | kg/yr | 52742.5 | |
| Total FWM Outflow Conc | ppb | 80.3 | |
| Bypass Load | kg/yr | 0 | |
| Bypass Load | % | 0% | |
| Maximum Inflow | hm3/d | 14.75 | |
| Maximum Outflow | hm3/d | 10.09 | |
| Surface Load Reduction | kg/yr | 25998 | |
| Load Trapped in Sediments | kg/yr | 12095 | |
| Overall Load Reduction | % | 18% | |
| Lower Confidence Limit | % | #N/A | |
| Upper Confidence Limit | % | #N/A | |
| Daily Geometric Mean | ppb | 68.2 | |
| Outflow Geo Mean - Composites | ppb | 93.1 | |
| Upper Confidence Limit | ppb | #N/A | |
| Lower Confidence Limit | ppb | #N/A | |
| Frequency Outflow Conc > 10 ppb | % | 100% | |
| Frequency Outflow Conc > 20 ppb | % | 100% | |
| Frequency Outflow Conc > 50 ppb | % | 92% | |
| Freq Outflow Volume > 10 ppb | % | 72% | |
| 95th Percentile Outflow Conc | ppb | 683.91 | |
| Mean Biomass P Storage | mg/m2 | 406 | |
| Storage Increase / Net Removal | % | 2% | |
| Net Storage Turnover Rate | 1/yr | 16.9 | |
| Unit Area P Load | mg/m2-yr | 1039 | |
| Unit Area P Removal | mg/m2-yr | 197 | |
| Mean Water Load | cm/d | 3.0 | |
| Max Water Load | cm/d | 24.0 | |
| Mean Depth | cm | 198 | |
| Minimum Depth | cm | 1.0 | |
| Maximum Depth | cm | 387.2 | |
| Frequency Depth < 10 cm | % | 12% | |
| Flow/Width | m2/day | 272 | |
| HRT Days | days | 66.7 | |
| Mean Velocity | cm/sec | 0.16 | |
| Seepage Outflow / Total Outflow | % | 1% | |
| Release 1 Outflow Volume | hm3/yr | 186.46 | |
| Release 2 Outflow Volume | hm3/yr | 0.00 | |
| 95th Percentile Outflow Volume | hm3/d | 9.1 | |
| 95th Percentile Outflow Load | kg/d | 746.8 | |
| Simulated / Specified Mean Depth | % | 100% | |
| Release 1 Demand Met | % | 90% | |
| Release 2 Demand Met | % | #N/A | |
| Outflow Demand Met | % | 105% | |
| Range Check - Mean Depth | - | 0 | |
| Range Check - Freq Depth < 10 cm | - | 0 | |
| Range Check - Flow/Width | - | 0 | |
| Range Check - Inflow Conc | - | 0 | |
| Range Check - Outflow Conc | - | 0 | |
| Water Balance Error | % | 0.00% | |
| Mass Balance Error | % | 0.01% | |
| Warning or Error Messages | - | | 0 |
| | | | Overall |
| | | | 1.53 |
| | | | 10/16/07 |
| | | | 01/01/65 |
| | | | 05/01/65 |
| | | | 04/30/00 |
| | | | 12784 |
| | | | Total |
| | | | EAASR_NET |
| | | | Base |
| | | | 61.54 |
| | | | 130.7 |
| | | | 125.7 |
| | | | 666.6 |
| | | | 63931.8 |
| | | | 95.9 |
| | | | 470.5 |
| | | | 39773 |
| | | | 84.5 |
| | | | #N/A |
| | | | #N/A |
| | | | 656.9 |
| | | | 52742.5 |
| | | | 80.3 |
| | | | 0.0 |
| | | | 0.0 |
| | | | 14.75 |
| | | | 10.09 |
| | | | 24159 |
| | | | 12095 |
| | | | 18% |
| | | | #N/A |
| | | | #N/A |
| | | | #N/A |
| | | | 93.1 |
| | | | #N/A |
| | | | #N/A |
| | | | 100% |
| | | | 100% |
| | | | 100% |
| | | | 72% |
| | | | 684 |
| | | | 406 |
| | | | 2% |
| | | | 16.9 |
| | | | 1039 |
| | | | 197 |
| | | | 3.0 |
| | | | 24.0 |
| | | | 198 |
| | | | 1 |
| | | | 387 |
| | | | 12.1% |
| | | | 272.4 |
| | | | 66.7 |
| | | | 0.16 |
| | | | 2% |
| | | | 186.46 |
| | | | 0.00 |
| | | | 9.1 |
| | | | 746.8 |
| | | | 1.0 |
| | | | 0.9 |
| | | | #N/A |
| | | | 1.0 |
| | | | 0 |
| | | | 0 |
| | | | 0 |
| | | | 0 |
| | | | 0.00% |
| | | | 0.01% |
| | | | 0 |



EAASR With Lake Okeechobee TP=150 ppb

| DMSTA2- Inputs & Outputs | | Project: PROJECT_EAASR_STA34_NETWORK_EIS | | | | | | | | | | Model Release: 07/05/07 | | | |
|---------------------------------------|-------------|--|--|-------|------|------|-------------------------------|------|------|------------------------|----|--------------------------|----|---------|-------------|
| | | | | | | | | | | | | Current Date: 10/16/2007 | | | |
| Input Variable | Units | Value | Case Description: | | | | | | | | | | | | |
| Design Case Name | - | A1 150 | Compartment A-1 | | | | | | | | | | | | |
| Input Series Name | - | TS_A1_150 | 15,200 acres (from A8) | | | | | | | | | | | | |
| Starting Date for Simulation | - | 01/01/65 | Inflow volumes, outflow volumes, and depths from SFWM simulation | | | | | | | | | | | | |
| Ending Date for Simulation | - | 04/30/00 | Reservoir settling rate = 5.0 m/yr (Goforth 2006) | | | | | | | | | | | | |
| Starting Date for Output | - | 05/01/65 | Lake inflow = 150 ppb | | | | | | | | | | | | |
| Integration Steps Per Day | - | 4 | Simulation Type | | | | | | | | | | | | |
| Number of Iterations | - | 0 | Output Variable | | | | | | | | | | | | |
| Output Averaging Interval | days | 365 | FWM Outflow C (ppb) | 112.3 | #N/A | #N/A | H2O Balance Error Mean & Max | 0.0% | 0.0% | | | | | | |
| Inflow Conc Scale Factor | - | 1 | GM Outflow C (ppb) | 114.7 | #N/A | #N/A | Mass Balance Error Mean & Max | 0.0% | 0.0% | | | | | | |
| Rainfall P Conc | ppb | 10 | Load Reduction % | 17% | #N/A | #N/A | Iterations & Convergence | 2 | 0.8% | | | | | | |
| Atmospheric P Load (Dry) | mg/m2-yr | 20 | Bypass Load (%) | 0.0% | | | | | | Warning/Error Messages | 0 | | | | |
| Cell Number --> | - | 1 | | | | | | | | | | | | | |
| Cell Label | - | 1 | | | | | | | | | | | | | |
| Vegetation Type | -> | none | | | | | | | | | | | | | |
| Inflow Fraction | - | 1 | | | | | | | | | | | | | |
| Downstream Cell Number | - | 1 | | | | | | | | | | | | | |
| Surface Area | km2 | 61.54 | | | | | | | | | | | | | |
| Mean Width of Flow Pi | km | 6.70 | | | | | | | | | | | | | |
| Number of Tanks in Series | - | 1.0 | | | | | | | | | | | | | |
| Minimum Depth for Releases | cm | 15 | | | | | | | | | | | | | |
| Release 1 Series Name | - | IRRIG | | | | | | | | | | | | | |
| Release 2 Series Name | - | TO_STA | | | | | | | | | | | | | |
| Outflow Series Name | - | TO_STA | | | | | | | | | | | | | |
| Depth Series Name | - | A1_DEPTH | | | | | | | | | | | | | |
| Outflow Control Depth | cm | 15 | | | | | | | | | | | | | |
| Outflow West Depth | cm | 15 | | | | | | | | | | | | | |
| Outflow Coefficient - Exponent | - | 1.5 | | | | | | | | | | | | | |
| Outflow Coefficient - Intercept | - | 8 | | | | | | | | | | | | | |
| Bypass Depth | cm | 15 | | | | | | | | | | | | | |
| Maximum Inflow | hm3/day | 9.05233 | | | | | | | | | | | | | |
| Maximum Outflow | hm3/day | 9.05233 | | | | | | | | | | | | | |
| Inflow Seepage Rate | (cm/d) / cm | 0.00081 | | | | | | | | | | | | | |
| Inflow Seepage Control Elev | cm | 6 | | | | | | | | | | | | | |
| Inflow Seepage Conc | ppb | 100 | | | | | | | | | | | | | |
| Outflow Seepage Rate | (cm/d) / cm | 0.75 | | | | | | | | | | | | | |
| Outflow Seepage Control Elev | cm | 1 | | | | | | | | | | | | | |
| Max Outflow Seepage Conc | ppb | 1 | | | | | | | | | | | | | |
| Seepage Recycle to Cell Number | - | 0.75 | | | | | | | | | | | | | |
| Seepage Recycle Fraction | - | 1 | | | | | | | | | | | | | |
| Seepage Discharge Fraction | - | 1 | | | | | | | | | | | | | |
| Initial Water Column Conc | ppb | 105 | | | | | | | | | | | | | |
| Initial P Storage Per Unit Area | mg/m2 | 525 | | | | | | | | | | | | | |
| Initial Water Column Depth | cm | 150 | | | | | | | | | | | | | |
| C0 = Conc at 0 g/m2 P Storage | ppb | 3 | | | | | | | | | | | | | |
| C1 = Conc at 1 g/m2 P storage | ppb | 150 | | | | | | | | | | | | | |
| C2 = Conc at Half-Max Uptake | ppb | 5.0 | | | | | | | | | | | | | |
| K = Net Settling Rate at Steady State | m/yr | 40 | | | | | | | | | | | | | |
| Z1 = Saturated Uptake Depth | cm | 100 | | | | | | | | | | | | | |
| Z2 = Lower Penalty Depth | cm | 400 | | | | | | | | | | | | | |
| Z3 = Upper Penalty Depth | cm | 400 | | | | | | | | | | | | | |
| Output Variables | Units | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | Overall | |
| Execution Time | sec/yr | 1.56 | | | | | | | | | | | | | 1.56 |
| Run Date | - | 10/16/07 | | | | | | | | | | | | | 10/16/07 |
| Starting Date for Simulation | - | 01/01/65 | | | | | | | | | | | | | 01/01/65 |
| Starting Date for Output | - | 05/01/65 | | | | | | | | | | | | | 05/01/65 |
| Ending Date | - | 04/30/00 | | | | | | | | | | | | | 04/30/00 |
| Output Duration | days | 12784 | | | | | | | | | | | | | 12784 |
| Cell Label | - | 1 | | | | | | | | | | | | | Total |
| Downstream Cell Label | - | Outflow | | | | | | | | | | | | | |
| Network Simulation Name | - | AASR_NET 150 | | | | | | | | | | | | | EAASR_NET 1 |
| Simulation Type | - | Base | | | | | | | | | | | | | Base |
| Surface Area | km2 | 61.54 | | | | | | | | | | | | | 61.54 |
| Mean Rainfall | cm/yr | 130.73 | | | | | | | | | | | | | 130.73 |
| Mean ET | cm/yr | 125.71 | | | | | | | | | | | | | 125.71 |
| Cell Inflow Volume | hm3/yr | 666.6 | | | | | | | | | | | | | 666.6 |
| Cell Inflow Load | kg/yr | 83883.4 | | | | | | | | | | | | | 83883.4 |
| Cell Inflow Conc | ppb | 126 | | | | | | | | | | | | | 125.8 |
| Treated Outflow Volume | hm3/yr | 470.5 | | | | | | | | | | | | | 470.5 |
| Treated Outflow Load | kg/yr | 52848.7 | | | | | | | | | | | | | 52849 |
| Treated FWM Outflow Conc | ppb | 112 | | | | | | | | | | | | | 112.3 |
| Upper Confidence Limit | ppb | #N/A | | | | | | | | | | | | | #N/A |
| Lower Confidence Limit | ppb | #N/A | | | | | | | | | | | | | #N/A |
| Total Outflow Volume + Bypass | hm3/yr | 656.9 | | | | | | | | | | | | | 656.9 |
| Total Outflow Load + Bypass | kg/yr | 69304.8 | | | | | | | | | | | | | 69304.8 |
| Total FWM Outflow Conc | ppb | 105.5 | | | | | | | | | | | | | 105.5 |
| Bypass Load | kg/yr | 0 | | | | | | | | | | | | | 0.0 |
| Bypass Conc | ppb | 0% | | | | | | | | | | | | | 0.0 |
| Maximum Inflow | hm3/d | 14.75 | | | | | | | | | | | | | 14.75 |
| Maximum Outflow | hm3/d | 10.09 | | | | | | | | | | | | | 10.09 |
| Surface Load Reduction | kg/yr | 33175 | | | | | | | | | | | | | 31035 |
| Load Trapped in Sediments | kg/yr | 14901 | | | | | | | | | | | | | 14901 |
| Overall Load Reduction | % | 17% | | | | | | | | | | | | | 17% |
| Lower Confidence Limit | % | #N/A | | | | | | | | | | | | | #N/A |
| Upper Confidence Limit | % | #N/A | | | | | | | | | | | | | #N/A |
| Daily Geometric Mean | ppb | 81.8 | | | | | | | | | | | | | #N/A |
| Outflow Geo Mean - Composites | ppb | 114.7 | | | | | | | | | | | | | 114.7 |
| Upper Confidence Limit | ppb | #N/A | | | | | | | | | | | | | #N/A |
| Lower Confidence Limit | ppb | #N/A | | | | | | | | | | | | | #N/A |
| Frequency Outflow Conc > 10 ppb | % | 100% | | | | | | | | | | | | | 100% |
| Frequency Outflow Conc > 20 ppb | % | 100% | | | | | | | | | | | | | 100% |
| Frequency Outflow Conc > 50 ppb | % | 93% | | | | | | | | | | | | | 100% |
| Freq Outflow Volume > 10 ppb | % | 72% | | | | | | | | | | | | | 72% |
| 95th Percentile Outflow Conc | ppb | 806.27 | | | | | | | | | | | | | 806 |
| Mean Biomass P Storage | mg/m2 | 508 | | | | | | | | | | | | | 508 |
| Storage Increase / Net Removal | % | 3% | | | | | | | | | | | | | 3% |
| Net Storage Turnover Rate | 1/yr | 16.7 | | | | | | | | | | | | | 16.7 |
| Unit Area P Load | mg/m2-yr | 1363 | | | | | | | | | | | | | 1363 |
| Unit Area P Removal | mg/m2-yr | 242 | | | | | | | | | | | | | 242 |
| Mean Water Load | cm/d | 3.0 | | | | | | | | | | | | | 3.0 |
| Max Water Load | cm/d | 24.0 | | | | | | | | | | | | | 24.0 |
| Mean Depth | cm | 198 | | | | | | | | | | | | | 198 |
| Minimum Depth | cm | 1.0 | | | | | | | | | | | | | 1 |
| Maximum Depth | cm | 387.2 | | | | | | | | | | | | | 387 |
| Frequency Depth < 10 cm | % | 12% | | | | | | | | | | | | | 12.1% |
| Flow/Width | m2/day | 272 | | | | | | | | | | | | | 272.4 |
| HRT Days | days | 66.7 | | | | | | | | | | | | | 66.7 |
| Mean Velocity | cm/sec | 0.16 | | | | | | | | | | | | | 0.16 |
| Seepage Outflow / Total Outflow | % | 1% | | | | | | | | | | | | | 2% |
| Release 1 Outflow Volume | hm3/yr | 186.46 | | | | | | | | | | | | | 186.5 |
| Release 2 Outflow Volume | hm3/yr | 0.00 | | | | | | | | | | | | | 0.0 |
| 95th Percentile Outflow Volume | hm3/d | 9.1 | | | | | | | | | | | | | 9.1 |
| 95th Percentile Outflow Load | kg/d | 1022.9 | | | | | | | | | | | | | 1022.9 |
| Simulated / Specified Mean Depth | % | 100% | | | | | | | | | | | | | 1.0 |
| Release 1 Demand Met | % | 90% | | | | | | | | | | | | | 0.9 |
| Release 2 Demand Met | % | #N/A | | | | | | | | | | | | | #N/A |
| Outflow Demand Met | % | 105% | | | | | | | | | | | | | 1.0 |
| Range Check - Mean Depth | - | - | | | | | | | | | | | | | 0 |
| Range Check - Freq Depth < 10 cm | - | - | | | | | | | | | | | | | 0 |
| Range Check - Flow/Width | - | - | | | | | | | | | | | | | 0 |
| Range Check - Inflow Conc | - | - | | | | | | | | | | | | | 0 |
| Range Check - Outflow Conc | - | - | | | | | | | | | | | | | 0 |
| Water Balance Error | % | 0.00% | | | | | | | | | | | | | 0.00% |
| Mass Balance Error | % | 0.01% | | | | | | | | | | | | | 0.01% |
| Warning or Error Messages | - | - | | | | | | | | | | | | | 0 |



STA-6

| DMSTA2- Inputs & Outputs | | Project: STA 6 EIS | | Model Release: 07/05/07 | | | | | | | | | | | |
|---------------------------------------|-------|--------------------------|---|-------------------------|---|---|---|---|---|---|----|----|----|---------|--|
| | | Current Date: 10/18/2007 | | | | | | | | | | | | | |
| Input Variable | Units | Value | Case Description: | | | | | | | | | | | | |
| Design Case Name | | | STA-6 flowways | | | | | | | | | | | | |
| Input Series Name | | | Inflows are a mixture of C-139 Basin and C-139 Annex (USSO) | | | | | | | | | | | | |
| Starting Date for Simulation | | | 05/01/94 | | | | | | | | | | | | |
| Ending Date for Simulation | | | 04/30/07 | | | | | | | | | | | | |
| Starting Date for Output | | | 05/01/94 | | | | | | | | | | | | |
| Integration Steps Per Day | | | 4 | | | | | | | | | | | | |
| Number of Iterations | | | 0 | | | | | | | | | | | | |
| Output Averaging Interval | | | 365 | | | | | | | | | | | | |
| Inflow Conc Scale Factor | | | 1 | | | | | | | | | | | | |
| Rainfall P Conc | | | 10 | | | | | | | | | | | | |
| Atmospheric P Load (Dry) | | | 20 | | | | | | | | | | | | |
| Cell Number -> | | | 1 2 3 4 5 6 7 8 9 10 11 12 | | | | | | | | | | | | |
| Cell Label | | | 3 5 2 4 | | | | | | | | | | | | |
| Vegetation Type | | | PEW_3 PEW_3 EMG_3 SAV_3 | | | | | | | | | | | | |
| Inflow Fraction | | | 0.0665 0.2301 0.6835 4 | | | | | | | | | | | | |
| Downstream Cell Number | | | | | | | | | | | | | | | |
| Surface Area | | | 0.99 2.64 2.23 5.62 | | | | | | | | | | | | |
| Mean Width of Flow Path | | | 0.61 1.31 1.14 2.39 | | | | | | | | | | | | |
| Number of Tanks in Series | | | 3.0 3.0 3.0 3.0 | | | | | | | | | | | | |
| Minimum Depth for Releases | | | | | | | | | | | | | | | |
| Release 1 Series Name | | | | | | | | | | | | | | | |
| Release 2 Series Name | | | | | | | | | | | | | | | |
| Outflow Series Name | | | | | | | | | | | | | | | |
| Depth Series Name | | | | | | | | | | | | | | | |
| Outflow Control Depth | | | 28.3464 35.3568 38.1 48.8 | | | | | | | | | | | | |
| Outflow Weir Depth | | | | | | | | | | | | | | | |
| Outflow Coefficient - Exponent | | | 4 4 4 4 | | | | | | | | | | | | |
| Outflow Coefficient - Intercept | | | 1 1 1 1 | | | | | | | | | | | | |
| Bypass Depth | | | cm | | | | | | | | | | | | |
| Maximum Inflow | | | hm3/day | | | | | | | | | | | | |
| Maximum Outflow | | | hm3/day | | | | | | | | | | | | |
| Inflow Seepage Rate | | | (cm/d) / cm | | | | | | | | | | | | |
| Inflow Seepage Control Elev | | | cm | | | | | | | | | | | | |
| Inflow Seepage Conc | | | ppb | | | | | | | | | | | | |
| Outflow Seepage Rate | | | (cm/d) / cm | | | | | | | | | | | | |
| Outflow Seepage Control Elev | | | cm | | | | | | | | | | | | |
| Max Outflow Seepage Conc | | | ppb | | | | | | | | | | | | |
| Seepage Recycle to Cell Number | | | | | | | | | | | | | | | |
| Seepage Recycle Fraction | | | | | | | | | | | | | | | |
| Seepage Discharge Fraction | | | | | | | | | | | | | | | |
| Initial Water Column Conc | | | ppb 30 30 30 30 | | | | | | | | | | | | |
| Initial P Storage Per Unit Area | | | mg/m2 500 500 500 500 | | | | | | | | | | | | |
| Initial Water Column Depth | | | cm 200 200 200 200 | | | | | | | | | | | | |
| C0 = Conc at 0 g/m2 P Storage | | | ppb 3 3 3 3 | | | | | | | | | | | | |
| C1 = Conc at 1 g/m2 P storage | | | ppb 22 22 22 22 | | | | | | | | | | | | |
| C2 = Conc at Half-Max Uptake | | | ppb 300 300 300 300 | | | | | | | | | | | | |
| K = Net Settling Rate at Steady State | | | m/yr 34.9 34.9 16.8 52.5 | | | | | | | | | | | | |
| Z1 = Saturated Uptake Depth | | | cm 40 40 40 40 | | | | | | | | | | | | |
| Z2 = Lower Penalty Depth | | | cm 100 100 100 100 | | | | | | | | | | | | |
| Z3 = Upper Penalty Depth | | | cm 200 200 200 200 | | | | | | | | | | | | |
| Output Variables | Units | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | Overall | |
| Execution Time | | | | | | | | | | | | | | | |
| Run Date | | | | | | | | | | | | | | | |
| Starting Date for Simulation | | | | | | | | | | | | | | | |
| Ending Date for Output | | | | | | | | | | | | | | | |
| Output Duration | | | | | | | | | | | | | | | |
| Cell Label | | | | | | | | | | | | | | | |
| Downstream Cell Label | | | | | | | | | | | | | | | |
| Network Simulation Name | | | | | | | | | | | | | | | |
| Simulation Type | | | | | | | | | | | | | | | |
| Surface Area | | | | | | | | | | | | | | | |
| Mean Rainfall | | | | | | | | | | | | | | | |
| Mean ET | | | | | | | | | | | | | | | |
| Cell Inflow Volume | | | | | | | | | | | | | | | |
| Cell Inflow Conc | | | | | | | | | | | | | | | |
| Treated Outflow Volume | | | | | | | | | | | | | | | |
| Treated Outflow Load | | | | | | | | | | | | | | | |
| Treated FWM Outflow Conc | | | | | | | | | | | | | | | |
| Upper Confidence Limit | | | | | | | | | | | | | | | |
| Lower Confidence Limit | | | | | | | | | | | | | | | |
| Total Outflow Volume + Bypass | | | | | | | | | | | | | | | |
| Total FWM Outflow Conc | | | | | | | | | | | | | | | |
| Bypass Load | | | | | | | | | | | | | | | |
| Bypass Load % | | | | | | | | | | | | | | | |
| Maximum Inflow | | | | | | | | | | | | | | | |
| Maximum Outflow | | | | | | | | | | | | | | | |
| Surface Load Reduction | | | | | | | | | | | | | | | |
| Load Trapped in Sediments | | | | | | | | | | | | | | | |
| Overall Load Reduction | | | | | | | | | | | | | | | |
| Lower Confidence Limit | | | | | | | | | | | | | | | |
| Upper Confidence Limit | | | | | | | | | | | | | | | |
| Daily Geometric Mean | | | | | | | | | | | | | | | |
| Outflow Geo Mean - Composites | | | | | | | | | | | | | | | |
| Upper Confidence Limit | | | | | | | | | | | | | | | |
| Lower Confidence Limit | | | | | | | | | | | | | | | |
| Frequency Outflow Conc > 10 ppb | | | | | | | | | | | | | | | |
| Frequency Outflow Conc > 20 ppb | | | | | | | | | | | | | | | |
| Frequency Outflow Conc > 50 ppb | | | | | | | | | | | | | | | |
| Freq Outflow Volume > 10 ppb | | | | | | | | | | | | | | | |
| 95th Percentile Outflow Conc | | | | | | | | | | | | | | | |
| Mean Biomass P Storage | | | | | | | | | | | | | | | |
| Storage Increase / Net Removal | | | | | | | | | | | | | | | |
| Net Storage Turnover Rate | | | | | | | | | | | | | | | |
| Unit Area P Load | | | | | | | | | | | | | | | |
| Unit Area P Removal | | | | | | | | | | | | | | | |
| Mean Water Load | | | | | | | | | | | | | | | |
| Max Water Load | | | | | | | | | | | | | | | |
| Mean Depth | | | | | | | | | | | | | | | |
| Minimum Depth | | | | | | | | | | | | | | | |
| Maximum Depth | | | | | | | | | | | | | | | |
| Frequency Depth < 10 cm | | | | | | | | | | | | | | | |
| Flow/Width | | | | | | | | | | | | | | | |
| HRT Days | | | | | | | | | | | | | | | |
| Mean Velocity | | | | | | | | | | | | | | | |
| Seepage Outflow / Total Outflow | | | | | | | | | | | | | | | |
| Release 1 Outflow Volume | | | | | | | | | | | | | | | |
| Release 2 Outflow Volume | | | | | | | | | | | | | | | |
| 95th Percentile Outflow Volume | | | | | | | | | | | | | | | |
| 95th Percentile Outflow Load | | | | | | | | | | | | | | | |
| Simulated / Specified Mean Depth | | | | | | | | | | | | | | | |
| Release 1 Demand Met | | | | | | | | | | | | | | | |
| Release 2 Demand Met | | | | | | | | | | | | | | | |
| Outflow Demand Met | | | | | | | | | | | | | | | |
| Range Check - Mean Depth | | | | | | | | | | | | | | | |
| Range Check - Freq Depth < 10 cm | | | | | | | | | | | | | | | |
| Range Check - Flow/Width | | | | | | | | | | | | | | | |
| Range Check - Inflow Conc | | | | | | | | | | | | | | | |
| Range Check - Outflow Conc | | | | | | | | | | | | | | | |
| Water Balance Error | | | | | | | | | | | | | | | |
| Mass Balance Error | | | | | | | | | | | | | | | |
| Warning or Error Messages | | | | | | | | | | | | | | | |

