# Everglades Agricultural Area Regional Feasibility Study

## Deliverable 2.2 – Optimum Allocation of Loads to the STAs for the Period 2006-2009 (Final Report)

(Contract No. CN040912-WO04 Phase 2)

Prepared for:



South Florida Water Management District (SFWMD)

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September 7, 2005



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South Florida Water Management District EAA Regional Feasibility Study ADA Contract No. CN040912-WO04 Phase 2 Inflow Data Sets for the Period 2006-2009 <u>B&McD Project No. 38318</u>

Dear Mr. Vazquez:

Burns & McDonnell is pleased to submit this Final report on "Optimum Allocation of Loads to the STAs for the Period 2006-2009". This document constitutes Deliverable 2.2 under ADA Engineering, Inc. Task Order No. BM-05WO04-02 dated April 27, 2005. The August 9, 2005 draft deliverable has been updated to respond to comments received as indicated in my memorandum of September 2, 2005 to Mr. Roger Copp of ADA.

We gratefully acknowledge the valuable contributions of both your staff and that of the South Florida Water Management District in the development of the information presented herein.

#### Certification

I hereby certify, as a professional engineer in the State of Florida, that the information in this document was assembled under my direct personal charge. This report is not intended or represented to be suitable for reuse without specific verification or adaptation by the Engineer. This certification is provided in accordance with the provisions of the Laws and Rules of the Florida Board of Professional Engineers under Chapter 61G15-29, Florida Administrative Code.

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## 1. INTRODUCTION

This document and the analyses it summarizes were prepared by Burns & McDonnell Engineering Co., Inc. under contract to ADA Engineering, Inc (ADA). The conduct of these analyses and preparation of this document were authorized by the South Florida Water Management District (SFWMD or District) through its March 27, 2005 issuance of Work Order No. CN040912-WO04 to ADA, and subsequently authorized by ADA through its April 27, 2005 issuance of Task Order BM-05WO04-02 to Burns & McDonnell.

#### 1.1. Background

Under the Everglades Construction Project (ECP), the South Florida Water Management District has constructed several STAs and the U.S. Army Corps of Engineers has constructed STA-1E to help improve the quality of waters released to the Everglades Protection Area (EPA). In addition to the existing STAs, the District is planning certain STA expansions and enhancements, Everglades Agricultural Area (EAA) canal improvements, construction of the EAA Storage Reservoir Project, and other EAA improvements. With recognition of these planned improvements, the EAA Regional Feasibility Study (RFS) will evaluate alternatives for redistributing inflow volumes and phosphorus loads to the various STAs to optimize phosphorus removal performance. This study is not intended to define the final arrangement, location or character of these proposed projects but is a fact-finding exercise to develop the information necessary for the subsequent planning, design and construction of these future projects.

#### 1.2. Scope of Work

This document was prepared under Task 2 "Optimum Allocation of Phosphorus and Hydraulic Loading to the Existing STAs" of the SFWMD Work Order No. CN040912-WO04. The overall objective of Task 2 is to evaluate the redistribution of hydraulic and total phosphorus loads to the STAs (both existing and the currently planned STA-6, Section 2, STA-2 Cell 4, and STA-5 third flow-way) to optimize phosphorus reduction. Information developed in this document is also intended to provide basic input to the completion of work under Task 3, Phase 1 (Draft Operating Strategy for Optimizing STA Performance with Existing EAA Canals) being prepared by ADA Engineering, Inc. under SFWMD Contract No. CN040912-WO03.





This analysis is specific to the period 2006-2009 (e.g., following completion of the currently planned STA-6, Section 2; STA-2 Cell 4; and third flow-way at STA-5, but prior to completion of presently planned CERP or Acceler8 projects in the basins considered).

Estimates of the overall inflow volumes and TP loads to be accommodated in the various STAs were developed under Task 1 of Contract CN040912-WO04. Basins considered include the following:

- C-51 West Canal
- S-5A (West Palm Beach Canal)
- Ch. 298 Districts:
  - East Beach Water Control District
  - East Shore Water Control District
  - 715 Farms (State Lease No. 3420)
  - South Shore Drainage District
  - South Florida Conservancy District, Unit 5 (S-236 Basin)
- > S2/S-6/S-7 (Hillsboro and North New River Canals)
- ➢ S-3/S-8 (Miami Canal)
- ➢ C-139 and C-139 Annex
- ➢ L-8 Canal
- > Lake Okeechobee deliveries south to the STAs and Everglades

## 1.3. Analytical Methods for Estimating TP Reduction in STAs

The estimated performance of the various STAs in reducing total phosphorus concentrations presented in this document were developed employing the July 1, 2005 issue of the Dynamic Model for Stormwater Treatment Areas, Version 2 (DMSTA2), developed for the U.S. Department of the Interior and the U.S. Army Corps of Engineers by W. Walker and R. Kadlec. Additional information on DMSTA2 can be found on the Internet at:

www.wwalker.net/dmsta





#### 1.4. Reference Information

This section summarizes previous studies, reports and data employed in the conduct of the analyses presented herein.

#### 1.4.1. Inflow Volumes, TP Concentrations and TP Loads

Inflow volumes, TP concentrations and TP loads employed in this analysis are based on information presented in the following reports, all prepared for the South Florida Water Management District by Burns & McDonnell Engineering Co., Inc. under subcontract to ADA Engineering, Inc. as elements of Task 1 of the scope of work under District Contract CN040912-WO04:

- Deliverable 1.1.2: Evaluation of 2006 Hydrologic Simulation Results, Final Report dated June 27, 2005;
- Deliverable 1.3.2: Historic Inflow Volumes and Total Phosphorus Concentrations by Source, Final Report dated June 27, 2005;
- Deliverable 1.4.2: Methodology for Development of Daily Total Phosphorus Concentrations, Final Report dated June 30, 2005;
- Deliverable 1.5.2: Inflow Data Sets for the Period 2006-2009, Final Report dated August 9, 2005.

#### 1.4.2. Basic Designs of Proposed STA Expansions

Information on the presently planned configuration and basic layout and design of STA-6, Section 2; Cell 4 of STA-2; and the third flow-way of STA-5 was taken from the following documents:

Basis of Design Report (BODR) Stormwater Treatment Area 6 – Section 2 and Modifications to Section 1; prepared for the South Florida Water Management District by URS Corporation under Contract CN040936-WO02; June 1, 2005;





- Basis of Design Report (BODR) STA-2/Cell 4 Expansion Project; prepared for the South Florida Water Management District by Brown & Caldwell under Contract CN040935-WO04; May 12, 2005;
- Draft Basis of Design Report (BODR) Stormwater Treatment Area 5 Flow-way 3; prepared for the South Florida Water Management District by URS Corporation under Contract CN040936-WO05; April 20, 2005.

#### 1.4.3. Rainfall and Evapotranspiration

Estimates of daily rainfall and evapotranspiration (ET) at each of the STAs was taken from a District-furnished data file (ET\_RF\_STAs\_ECP2006.xls). That file includes daily values for both rainfall and ET at each cell of the SFWMM occupied by STA. The data extends from January 1, 1965 through December 31, 2000. For this analysis, daily data for those STAs occupying multiple cells of the SFWMM was estimated as the average of the individual cell values.

#### 1.4.4. Previous Studies and Reports

Certain of the background data and information discussed in this document was taken from the following previous studies and reports:

- (Draft) Supplemental Analysis, Everglades Protection Area Tributary Basins, prepared for the Everglades Agricultural Area Environmental Protection District by Burns & McDonnell; March 2, 2005 (hereinafter referred to as the Supplemental Analysis);
- Final Report, Everglades Protection Area Tributary Basins, Long-Term Plan for Achieving Water Quality Goals; prepared for the South Florida Water Management District by Burns & McDonnell; October, 2003 (hereinafter referred to as the Long-Term Plan), together with such modifications to the Long-Term Plan that are embodied in a revised Part 2 (dated November, 2004) submitted to the Florida Department of Environmental Protection (FDEP), and approved by FDEP in December, 2004;



- Basin-Specific Feasibility Studies, Everglades Protection Area Tributary Basins; Evaluation of Alternatives for the ECP Basins; prepared for the South Florida Water Management District by Burns & McDonnell; October 23, 2002 (hereinafter referred to as the BSFS Evaluation of Alternatives).
- Addendum to Design Documentation Report, Stormwater Treatment Area 1 East; prepared for the Jacksonville District, U.S. Army Corps of Engineers by Burns & McDonnell; November 2000;
- (Draft) Stormwater Treatment Area 1-East (STA-1E) Water Control Plan, Jacksonville District, U.S. Army Corps of Engineers; August, 2005;
- (Draft) Design Analysis Report for the STA-1E Cells 1-2 PSTA/SAV Field-Scale Demonstration Project, Palm Beach County, Florida; prepared for the Jacksonville District, U.S. Army Corps of Engineers by SAIC Engineering, Inc.; June 28, 2005.

#### 1.4.5. DMSTA2 Parameters for Existing STAs

Basic physical parameters for the various existing STAs reflected in the DMSTA2 analyses reported herein were taken from the BSFS Evaluation of Alternatives, with the following modifications:

- Marsh outflow coefficients (exponent and intercept) were modified to 4 and 1, respectively, consistent with basic guidance contained in the DMSTA2 documentation. They had previously been estimated on the basis of results taken from two-dimensional hydrodynamic analyses in certain of the STAs. It was concluded on the basis of trial runs that this change did not influence projected outflow concentrations, and modified peak and mean depths in the STAs resulting from the DMSTA2 by less than 5 centimeters.
- Seepage estimates were updated to reflect the results of water balance analyses prepared by the District for operating STAs. In addition, cell-to-cell seepage (at STA-1W and STA-1E) considered in the BSFS Evaluation of Alternatives was





eliminated from this analysis due to its minor influence on the results and to improve the clarity of the estimates.

The most significant modification to DMSTA parameters, as compared to those considered in the BSFS Evaluation of Alternatives, was the use of updated calibration data sets for the performance of various vegetation types in reducing total phosphorus concentrations. Three basic vegetation calibrations were considered in this analysis:

- EMG\_3: An updated calibration of the performance of emergent macrophyte vegetation, using data from full-scale STAs (replaced EMG in the 4/01/2002 version of DMSTA used in the BSFS Evaluation of Alternatives).
- SAV\_3: An updated calibration of the performance of submerged aquatic vegetation, using data from full-scale STAs (replaced SAV\_C4 and NEWS in the 4/01/2002 version of DMSTA used in the BSFS Evaluation of Alternatives).
- PEW\_3 (Pre-Existing Wetland): A new calibration data set developed to reflect the performance of those cells in the operating STAs (and in other wetland data sets, such as WCA-2A) in which the wetland vegetation existed naturally. As applied to the existing STAs, the application of this data set is limited to Cells 1 and 2 of STA-2; STA-6 Section 1; and Cell 1B of STA-3/4.

Water quality improvement projections on which the Long-Term Plan was based were predicated on an ability to reproduce the performance of the best two years of operation of Cell 4 in STA-1W (SAV\_C4) in those cells containing Submerged Aquatic Vegetation. A range in performance of those cells was also considered, employing the NEWS (Non-Emergent Wetland Systems) calibration data sets.

Comparison of summary data presented in Tables 2.4 and 2.6 of Deliverable 1.4.2 indicates that, for no other change in input data, the substitution of SAV\_3 in DMSTA2 for SAV\_C4 in the April 2002 version of DMSTA results in roughly a 20% increase in the projected flow-weighted mean TP concentration in outflows from STA-1W, following its enhancement as recommended in the Long-Term Plan, and roughly a 30%





increase in the estimated geometric mean TP concentration in those outflows. However, the projected flow-weighted and geometric mean concentrations using the SAV\_3 data set in DMSTA2 fall below those estimated using the NEWS calibration data set in the April 2002 version of DMSTA.

The net effect of this change in calibration data sets is to, as compared to projections considered in development of the Long-Term Plan and with all other inputs unchanged, result in higher projected outflow concentrations than the mean estimates considered in the Long-Term Plan, but still within the probable range of performance reported in the Long-Term Plan.

## 2. STA-1W

For this analysis, the enhancements to STA-1W recommended in the Long-Term Plan are assumed to be complete. This analysis considers the full area of the various flow paths as being effective for treatment, resulting in a total effective treatment area of 6,670 acres. In the BSFS Evaluation of Alternatives, the effective area of Cells 3 and 4 had been reduced by 326 and 108 acres, respectively.

All inflows to STA-1W enter through Structure G-302, a gated spillway situated in Levee L-7. That structure discharges from the STA-1 Inflow and Distribution Works. Inflows to the STA-1 Inflow and Distribution Works historically include pumped discharges from Pump Station S-5A and gravity inflows from the L-8 Borrow Canal through Structure S-5AS. In addition to G-302, discharges from the STA-1 Inflow and Distribution Works can be made through G-300 and G-301 (to the L-40 and L-7 borrow canals, respectively, in the Loxahatchee National Wildlife Refuge, or LNWR) and G-311 (to the West Distribution Cell of STA-1E).

The nominal capacity of S-5A is 4,800 cfs; of G-301 is 3,250 cfs; and of G-311 is 1,550 cfs.

In development of the South Florida Water Management Model (SFWMM) 2006 ECP simulation on which the estimated inflow volumes and TP loads is based, certain significant changes in overall system management from historic operations were assumed. Those assumptions include the following that directly and materially influence the projected performance of STA-1W in reducing total phosphorus loads and concentrations:





- Cessation of Lake Okeechobee regulatory releases at Structure S-352;
- Elimination of inflows to the STA-1 Inflow and Distribution Works from the L-8 Borrow Canal, including both L-8 Basin runoff and Lake Okeechobee releases to the L-8 Borrow Canal at Culvert C-10A;
- Water supply releases to the West Palm Beach Canal at S-352 destined for the Lower East Coast and delivered through the LNWR would only be made when the stage in the LNWR is at or below the floor of its regulation schedule.

Implementation of each of the above assumptions in the Operations Plan for STA-1W and related elements of the system is critical to the water quality improvement performance projections presented herein. In some instances, such as the elimination of inflows to the STA-1 Inflow and Distribution Works from the L-8 Borrow Canal, it may be necessary to institute certain structural changes in addition to the operations changes defined above.

For the period 2006-2009, inflows to the STA-1 Inflow and Distribution Works are assumed to be limited to runoff from the S-5A Basin in the Everglades Agricultural Area (EAA), runoff from the East Beach Water Control District (EBWCD) diverted to the West Palm Beach Canal, and water supply releases from Lake Okeechobee; those water supply releases are assumed to simply pass through the STA-1 Inflow and Distribution Works, and not require treatment. A summary of the estimated average annual inflows to the STA-1 Inflow and Distribution Works is presented in Table 2.1.

Source	Estimated Average Annual Inflow, WY 1966-2000			Remarks
	Volume (ac-ft)	TP Load (kg)	TP Conc. (ppb)	
S-5A Basin	234,809	44,582	154	Deliverable 1.5.2, Table 3.11
EBWCD	15,212	9,386	500	Deliverable 1.5.2, Table 2.3
Lake Okeechobee	16,726	2,468	120	Deliverable 1.5.2, Table 6.8
Total Inflow	266,747	56,436	172	
Assumed Bypass	16,726	2,468	120	Water Supply to LEC and L-8
Inflow to be Treated	250,021	53,968	175	

Table 2.1 Estimated Inflows to STA-1 I&D Works





Of the total water supply bypass volume, an average annual volume of 3,622 acre-feet per year is simulated as discharged to the LNWR, with the balance delivered to the L-8 borrow canal (reference Table 5.3 of Deliverable 1.1.2). The average annual TP load discharged to the LNWR in the water supply bypass is estimated to be 0.53 metric tons. It should also be noted that the S-5A Basin runoff listed in Table 2.1 excludes that part of the basin runoff considered diverted to STA-2 through the S-5A Basin Diversion Works.

#### 2.1. Inflows to STA-1W Based on Current Operations of G-302

At present, operations of the STA-1 Inflow and Distribution Works are normally structured to maximize the proportion of inflows to that area delivered through G-302 to STA-1W. As a result, it might be practicable to simply assign inflows up to the nominal capacity G-302 (3,250 cfs) to STA-1W, with the balance (e.g., S-5A discharges exceeding 3,250 cfs) considered delivered either to STA-1E through G-311 or bypassed to the LNWR through G-300 and G-301. However, application of a such a simplistic distribution of flow to the results of the SFWMM simulation is not considered advisable.

The simulation reports estimated mean daily discharges. In the instance of pumping station operations, such as at S-5A, the District's operational practice is to, in the interest of limiting operational expenditures, limit pumping operations to a single shift per day when practicable, and to minimize the use of second and third shifts. As a result, much of the simulated mean daily discharges at any given pumping station will occur at rates higher than the mean daily rates resulting from the simulation. In most application in the ECP, where the pumping stations discharge to large stormwater treatment areas, the influence of that operational distinction may be neglected. However, S-5A discharges to the relatively small footprint of the STA-1 Inflow and Distribution Works, where available storage is limited. It is therefore desirable to assess the distribution of outflows from that area on a basis other than simple assignation of mean daily inflows on the basis of relative capacity of the various discharge structures.

For this analysis, the distribution of discharges from the STA-1 Inflow and Distribution Works is based on evaluation of the distribution of inflows resulting from the District's actual operations of G-302 during full operation of STA-1W.





The initial filling of Cell 5 of STA-1W was begun on March 18, 1999; flow-through operations began July 7, 2000. Review of discharge data for Water Year 2001 reveals that roughly 38% of the total pumped discharges passed through Pumping Station G-310; pump testing at G-310 was not completed until the fall of 2000. That low utilization of the primary outflow pumping station leads to the presumption that STA-1W was not in full flow-through operations during significant parts of Water Year 2001.

In addition, Cells 5A and 5B were taken off line over the period February 15, 2003 through August 15, 2003 (Water Years 2003 and 2004) to permit construction of a limerock berm across Cell 5B as one element of the Process Development and Engineering (PDE) component of the *Long-Term Plan*. Cells 2 and 4 were taken off line over the period February 2004 through August 2004 (affecting the data for Water Year 2004) to allow an opportunity for tussocks in those cells to re-root, and to provide a "resting" interval following a period of extreme high inflows from Lake Okeechobee.

The above periods subsequent to July 2000 were excluded from the analysis, as the reduced utilization of STA-1W during those periods would suggest that discharges through G-302 would have been at less than normal capacity. In addition, discharges during Water Year 2005 were not considered in this analysis, as discharges to STA-1W have been curtailed in connection with on-going recovery actions in that STA.

Daily discharges were downloaded from the District's DBHYDRO data base for S-5A (DBKEY JW226), S-5AS (DBKEY TA410), and G-302 (DBKEY JJ806). Only positive discharges were considered in the analysis. The data was then screened to limit the analysis to the remaining periods of full operation of STA-1W during WY 2002-2004 (total of 824 days of full operation). Discharges from G-302 were then plotted against same-day inflows to the STA-1 Inflow and Distribution Works, and an approximate relationship was fit to the plotted data. For total daily inflows to the Inflow and Distribution Works up to 2,000 cfs, all inflows were assigned to STA-1W through G-302 (note that a daily inflow of 2,000 cfs is equivalent to pumping S-5A at capacity for a 10-hour period). For daily inflows above 2,000 cfs, the discharge at G-302 was computed as:

Q(G-302) = 2,000 + (Q(total) - 2,000)exp(0.8984)





For a total inflow to the STA-1 Inflow & Distribution Works of 4,800 cfs (capacity of S-5A), the distribution resulting from the above relationship would assign 3,250 cfs to G-302 (equal to its nominal capacity), and 1,550 cfs to other points of discharge (equal to the nominal capacity of G-311).

A plot of the data employed in this analysis, on which the flow distribution resulting from the above relationship is superimposed, is presented in Figure 2.1.



Figure 2.1 Distribution of Discharges through G-302

#### 2.2. Cases Considered in DMSTA2 Analysis of STA-1W

A total of three potential inflow cases were considered in the DMSTA2 analysis of STA-1W. The three cases considered are described as follows:

2006 All: All inflows to the STA-1 Inflow and Distribution Works were assigned to STA-1W. This case was included in the analysis to provide a frame of reference for assessment of the influence of diversions to STA-1E on the projected performance of STA-1W, and was structured upon the assumption that all runoff from the S-5A Basin could be delivered to and carried through STA-1W. That condition is not





physically possible, due to the hydraulic limitations of G-302 and the STA itself, analysis of this case (and Case "2006 All for STA-1E") was included simply to permit a basic assessment of potential imbalances in loading to the two treatment areas.

- 2006 Base: For this case, discharges from the STA-1 Inflow and Distribution Works to STA-1W were assumed governed by the relationship presented above. This case would be considered most representative of current operations in STA-1W coupled with the revised inflows applicable to the period 2006-2009. Inflows to the STA-1W Inflow and Distribution Works exceeding the assigned discharges at G-302 were considered as delivered to the West Distribution Cell of STA-1E through G-311.
- 2006 Mod: For this case, a modified distribution of discharges through G-302 and G-311 was assumed. Daily inflows to the STA-1 Inflow and Distribution Works were distributed 70% to STA-1W at G-302, and 30% to STA-1E at G-311. Those distributions closely parallel the relative capacities of G-302 and G-311.

#### 2.3. Summary of DMSTA2 Results

Table 2.2 presents a summary of the results of the DMSTA2 analyses for STA-1W. Summary DMSTA2 input and output data for each case are included in Appendix A.





Parameter	Units	Summary of Results by Case				
		2006 All	2006 Base	2006 Mod		
	Average Annual Inflow					
Volume	1,000 ac-ft	250.2	240.8	175.1		
TP Load	metric tons	53.80	51.73	37.66		
FWM TP Concentration	ppb	174	174	174		
	Average A	nnual Outflow				
Volume	1,000 ac-ft	251.0	241.6	176.7		
FWM TP Concentration						
Upper Confidence Limit	ppb	24.2	21.8	16.7		
Mean Estimate	ppb	30.0	27.5	20.3		
Lower Confidence Limit	ppb	37.3	34.5	25.2		
Geometric Mean TP Conc.						
Upper Confidence Limit	ppb	16.0	15.2	9.8		
Mean Estimate	ppb	21.8	20.9	13.5		
Lower Confidence Limit	ppb	29.1	27.9	18.5		
TP Load (Using Mean FWM Conc.)	metric tons	9.30	8.19	4.43		
For Detailed Results, See Appe	endix A	Table A.1	Table A.2	Table A.3		
Sur	nmary of Bypa	asses and Diversio	ns			
Water Supply to LEC and L-8						
Volume	1,000 ac-ft	16.7	16.7	16.7		
TP Load	metric tons	2.47	2.47	2.47		
FWM TP Concentration	ppb	120	120	120		
Divert to STA-1E Through G-311						
Volume	1,000 ac-ft	0.0	9.4	75.1		
TP Load	metric tons	0.00	2.07	16.14		
FWM TP Concentration	ppb		174	174		

#### Table 2.2 Summary of DMSTA2 Analyses, STA-1W, WY 1966-2000

## 3. STA-1E

For this analysis, STA-1E is assumed to be in full operation, and the enhancements to STA-1E recommended in the Long-Term Plan are assumed to be complete. This analysis considers the West and East Distribution Cells of STA-1 as integral elements of the treatment works, modeled as emergent vegetation with poor hydraulics (0.5 CSTRs in series).

Inflows to STA-1E enter through Structure G-311, a gated spillway situated in Levee L-40; Pumping Station S-319 on the C-51 West Canal; and Pumping Station S-361, which discharges to the upper end of Cell 4S of STA-1E. Structure G-311 discharges from the STA-1 Inflow and Distribution Works; inflows to STA-1E from that source are considered to be controlled by operations at G-302 and STA-1W. Pumping Station S-361 is projected to discharge an average of 2.5% of the total C-51 West Basin runoff; for this analysis, those discharges are assumed included in the total inflows to the C-51 West Canal.





In development of the South Florida Water Management Model (SFWMM) 2006 ECP simulation on which the estimated inflow volumes and TP loads are based, certain significant changes in overall system management from historic operations were assumed. Those assumptions include the following that directly and materially influence the projected performance of STA-1E in reducing total phosphorus loads and concentrations:

- Cessation of Lake Okeechobee regulatory releases to the L-8 Borrow Canal at Culvert C-10A (in particular those eventually discharged through Structure S-5AE);
- Elimination of inflows to the STA-1 Inflow and Distribution Works from the L-8 Borrow Canal, including both L-8 Basin runoff and Lake Okeechobee releases to the L-8 Borrow Canal at Culvert C-10A;
- Elimination of regulatory releases from the LNWR through Structure S-5AS and S-5AE.

Implementation of each of the above assumptions in the Operations Plan for STA-1E and related elements of the system is critical to the water quality improvement performance projections presented herein. In some instances, such as the elimination of inflows to the STA-1 Inflow and Distribution Works from the L-8 Borrow Canal, it may be necessary to institute certain structural changes in addition to the operations changes defined above.

In addition to the above assumptions, the operation of structures in and along the C-51 West Canal is assumed developed to send a volume through S-155A (bypassing STA-1E) equal to inflows to the C-51 West Canal from the L-8 Basin at S-5AE. For this analysis, those bypass volumes were assigned as equal to same-day inflows at S-5AE. The total phosphorus concentration in those bypasses was assigned equal to the flow-weighted mean concentration in all inflows to the C-51 West Canal on that same date. The net effect of this assumption is to bypass a larger total phosphorus load through S-155A than is delivered from the L-8 Basin through S-5AE.





For the period 2006-2009, inflows to the C-51 West Canal are considered limited to:

- Runoff from the C-51 West Basin;
- Runoff from Basin B of the Acme Improvement District, which is assumed to be diverted from its present points of discharge (to the LNWR) to the C-51 West Canal;
- Runoff from the L-8 Basin through Structure S-5AE (volumes assumed bypassed through S-155A as discussed above).

To the extent that water supply deliveries may be made through the C-51 West Canal, those water supply releases are assumed to simply pass through to S-155A and not require treatment. A summary of the estimated average annual inflows to the C-51 West Canal is presented in Table 3.1.

Source	Estimated Average Annual Inflow, WY 1966-2000			Remarks
	Volume (ac-ft)	TP Load (kg)	TP Conc. (ppb)	
C-51 West Basin	128,013	21,913	139	Deliverable 1.5.2, Table 5.5
Acme Basin B	33,196	4,633	113	Deliverable 1.5.2, Table 5.7
L-8 Basin	71,528	6,903	78	Deliverable 1.5.2, Table 5.2
Total Inflow	232,737	33,449	117	
Assumed Bypass	71,528	9,407	107	L-8 Runoff Through S-155A
Inflow to be Treated	161,209	24,042	121	

#### Table 3.1 Estimated Inflows to C-51 West Canal

In addition to the above average annual inflows to the C-51 West Canal, discharges from the STA-1 Inflow and Distribution Works through Structure G-311 are considered in the overall inflows to STA-1E.

#### 3.1. Cases Considered in DMSTA2 Analysis of STA-1E

A total of three potential inflow cases were considered in the DMSTA-2 analysis of STA-1E. The three cases considered are described as follows:

2006 All: All inflows to the C-51 West Canal (including inflows from the L-8 Basin) were assigned to STA-1E. For this case, which would parallel the "2006 All"





case for STA-1W, there would be no inflows to STA-1E from G-311. All inflows to the STA-1 Inflow and Distribution Works were assigned to STA-1W. This case was included in the analysis to provide a frame of reference for assessment of the influence of diversions at G-311 on the projected performance of STA-1E, and was structured upon the assumption that all runoff from the S-5A Basin could be delivered to and carried through STA-1W. That condition is not physically possible, due to the hydraulic limitations of G-302 and the STA itself, analysis of this case (and Case "2006 All for STA-1E") was included simply to permit a basic assessment of potential imbalances in loading to the two treatment areas, and to confirm the need for bypass of inflows from the L-8 Basin.

- 2006 Base: For this case, inflows to STA-1E from the C-51 West Canal at S-319 and at S-362 were assumed to be consistent with the summary data presented in Table 3.1 (e.g., bypass of inflow volumes from the L-8 Basin). In addition, inflows to the westerly flow path (Cells 5-7) of STA-1E would include discharges from the STA-1 Inflow and Distribution Works through Structure G-311. For this case, those discharges were considered equal to those developed under the "2006 Base" case for STA-1W (approximately 9,400 acre-feet per year at a flow-weighted mean TP concentration of 174 ppb). This case would be considered most representative of current operations in STA-1W and intended operations in STA-1E coupled with the revised inflows applicable to the period 2006-2009. For analysis of this case, the westerly flow path (Cells 5-7) was considered separately from the two easterly flow paths (Cells 1-4S). Analysis of the westerly flow path is included in Case 5\_7 2006 **Base.** Inflows to the westerly flow path included discharges from G-311 and 20% of the inflow to STA-1E at S-319. Analysis of the two easterly flow paths (Cells 1-4S) is included in **Case 1\_4 2006 Base**, for which inflows were limited to the remaining 80% of the inflow to STA-1E at S-319.
- 2006 Mod: For this case, a modified distribution of discharges through G-302 and G-311 was assumed, consistent with Case "2006 Mod" for STA-1W. Daily inflows to the STA-1 Inflow and Distribution Works were distributed 70% to STA-1W at G-302, and 30% to STA-1E at G-311. Those distributions closely parallel the relative capacities of G-302 and G-311. For analysis of this case, the westerly flow path





(Cells 5-7) was considered separately from the two easterly flow paths (Cells 1-4S). Analysis of the westerly flow path is included in **Case 5\_7 2006 Mod.** Inflows to the westerly flow path were limited to discharges from G-311, which were assigned at 30% of the total inflow to the STA-1 Inflow and Distribution Works (approximately 75,100 acre-feet per year at a flow-weighted mean TP concentration of 174 ppb). Analysis of the two easterly flow paths (Cells 1-4S) is included in **Case 1\_4 2006 Mod,** for which inflows were assigned at 100% of the inflow to STA-1E at S-319.

For both **2006 Base** and **2006 Mod**, seepage lost from the West Distribution Cell of STA-1E to the C-51 West Canal was assigned to the two easterly flow paths, essentially reducing total flows through Cells 5-7 and increasing total flows through Cells 1-4S.

#### 3.2. Summary of DMSTA2 Results

Table 3.2 presents a summary of the results of the DMSTA2 analyses for STA-1E. Summary DMSTA2 input and output data for each case are included in Appendix A.

Parameter	Units	Summary of Results by Case				
			2006	Base	2006	Mod
		2006 All	1_4 2006Base	5_72006Base	1_4 2006Mod	5_7 2006 Mod
		Average	Annual Inflow			
Volume	1,000 ac-ft	232.9	135.6	41.7	167.8	75.1
TP Load	metric tons	33.47	20.21	6.88	25.02	16.14
FWM TP Concentration	ppb	117	121	134	121	174
Total for Case						
Volume	1,000 ac-ft	232.9	17	7.2	24	2.9
TP Load	metric tons	33.47	27	.09	41	.16
FWM TP Concentration	ppb	117	12	24	1:	37
		Average A	Annual Outflow			
Volume	1,000 ac-ft	229.3	137.0	37.9	169.4	71.6
FWM TP Concentration						
Upper Confidence Limit	ppb	17.3	13.8	18.0	18.4	21.5
Mean Estimate	ppb	22.1	18.3	19.5	24.3	27.2
Lower Confidence Limit	ppb	27.9	24.1	21.7	31.2	34.9
Geometric Mean TP Conc.						
Upper Confidence Limit	ppb	10.5	10.6	4.9	14.8	12.3
Mean Estimate	ppb	14.5	14.9	5.9	20.5	17.5
Lower Confidence Limit	ppb	19.8	20.5	7.6	27.3	24.5
TP Load (Using Mean FWM Conc.)	metric tons	6.24	3.10	0.91	5.08	2.41
For Detailed Results, See Appe	endix A	Table A.4	Table A.5	Table A.6	Table A.7	Table A.8
Total For Case						
Volume	1,000 ac-ft	229.3	17	4.9	24	0.9
Flow-Weighted Mean TP Concentra	ation					
Upper Confidence Limit	ppb	17.3	14	1.7	19	9.3
Mean Estimate	ppb	22.1	2.1 18.6		25	5.2
Lower Confidence Limit	ppb	27.9	7.9 23.6		32	2.3
TP Load (Using Mean FWM Conc.)	metric tons	6.24	4.02		7.	48
		Summary of Byp	asses and Diversio	ns		
Bypass Through S-155A						
Volume	1,000 ac-ft	0.0	71.5 71.5		.5	
TP Load	metric tons	0.00	9.	41	9.	41
FWM TP Concentration	ppb		10	)7	1	)7

#### Table 3.2 Summary of DMSTA2 Analyses, STA-1E, WY 1966-2000

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# 3.3. Treated Discharges to Loxahatchee National Wildlife Refuge (LNWR)

Table 3.3 summarizes total estimated discharges to the Loxahatchee National Wildlife Refuge (LNWR) from STA-1W and STA-1E under the three basic cases considered for the combined operations of those two treatment areas. That tabulation excludes water supply bypasses to the Lower East Coast.

Sourco	Estimated Aver	age Annual Inflov	Pomarka	
Source	Volume (ac-ft)	TP Load (kg)	TP Conc. (nnh)	Reindiks
	Volume (ac-rt)			
STA-1W				Table 2.2
Upper Conf Limit	251.0	7 50	24.2	
Mean Estimate	251.0	9.30	30.0	
Lower Conf. Limit	251.0	11.56	37.3	
STA-1F	201.0	11.00	01.0	Table 3.2
Upper Conf. Limit	229.3	4.88	17.3	
Mean Estimate	229.3	6.24	22.1	
Lower Conf. Limit	229.3	7.88	27.9	
Total Inflow				
Upper Conf. Limit	480.3	12.4	20.9	
Mean Estimate	480.3	15.5	26.2	
Lower Conf. Limit	480.3	19.4	32.8	
		Case 20	06 Base	
STA-1W				Table 2.2
Upper Conf. Limit	241.6	6.49	21.8	
Mean Estimate	241.6	8.19	27.5	
Lower Conf. Limit	241.6	10.27	34.5	
STA-1E				Table 3.2
Upper Conf. Limit	174.9	3.18	14.7	
Mean Estimate	174.9	4.02	18.6	
Lower Conf. Limit	174.9	5.10	23.6	
Total Inflow				
Upper Conf. Limit	416.5	9.7	18.8	
Mean Estimate	416.5	12.2	23.7	
Lower Conf. Limit	416.5	15.4	29.9	
		Case 20	006 Mod	
STA-1W				Table 2.2
Upper Conf. Limit	176.7	3.65	16.7	
Mean Estimate	176.7	4.43	20.3	
Lower Conf. Limit	176.7	5.50	25.2	
STA-1E				Table 3.2
Upper Conf. Limit	240.9	5.74	19.3	
Mean Estimate	240.9	7.48	25.2	
Lower Conf. Limit	240.9	9.60	32.3	
Total Inflow				
Upper Conf. Limit	417.6	9.4	18.2	
Mean Estimate	417.6	11.9	23.1	
Lower Conf. Limit	417.6	15.1	29.3	

Table 3.3	Total	Estimated	Treated	Discharges	to 1	LNWR
I ubic bib	I Otul	Louinacea	II cuttu	Discharges		

In addition, for each of the three basic cases considered, there would also be untreated discharges from the STA-1 Inflow and Distribution Works for Lower East Coast water





supply when stages in the LNWR are at or below the floor of the LNWR regulation schedule (see Table 2.2 and the text immediately following that table).

#### 3.4. Bypass Flows and Loads Through S-155A

Under the "2006 All" cases for STA-1W and STA-1E, there would be little or no bypass of flow and TP load (other than dedicated water supply) through Structure S-155A. Under both "2006 Base" and "2006 Mod", an average annual volume of 71.5 thousand acre-feet at a flow-weighted mean TP concentration of 107 ppb (average annual TP load of 9.41 metric tons) would bypass STA-1E through S-155A. That bypass would result in a reduction in the total average annual TP load discharged to the LNWR of roughly 3.5 metric tons.

Table 3.4 summarizes the estimated average annual historic discharges to the east at the current location of S-155A. That summary is generally based on information presented in Deliverable 1.3.2 (historic data for Water Years 1995-2004), with the exception of the volumes and loads associated with C-51 West Basin runoff (for which no direct records are available). Average annual volumes and loads associated with C-51 West Basin runoff are taken from the results presented in Deliverable 1.5.2, and should thus be considered understated, as for much of that period the area now comprising STA-1E contributed to overall basin runoff.

Table 3.4 Historic	: (WY 1995-2004)	<b>Discharges</b> at	<b>Current Locat</b>	ion of S-155A
	. (		04110110 100000	

Source	Estimated Averag	e Annual Discharg	Remarks	
	Volume (ac-ft)	TP Load (kg)	TP Conc. (ppb)	
C-51 West Basin	128,013	21,913	139	Deliverable 1.5.2, Table 5.5
L-8 Basin at S-5AE	66,266	5,068	62	Deliverable 1.3.2, Table 6.20
S-5AS	49,581	6,067	99	Deliverable 1.3.2, Table 6.19
Lake Okeechobee	34,623	7,908	185	Deliverable 1.3.2, Table 6.18
Total Discharge	278,483	40,956	119	

For both Case "2006 Base" and "2006 Mod", the average annual volume discharged through S-155A would be roughly 26% of historic; the average annual TP load discharged through S-155A would be roughly 23% of historic. The bulk of that differential is associated with the assumed elimination of Lake Okeechobee regulatory releases and regulatory releases from the LNWR through S-5AS and S-5AE.





## 4. STA-2

For this analysis, STA-2 (including the addition of Cell 4) is considered to be in full operation. However, the enhancements to the existing STA-2 (before Cell 4 expansion) recommended in the Long-Term Plan are considered as not in place, as the District has indicated (through its December 2004 amendment of the Long-Term Plan) its intent not to immediately proceed with the subdivision of existing flow paths. In addition, Cells 1 and 2 of STA-2 are analyzed using DMSTA2 calibration data sets for pre-existing vegetation (PEW\_3), as no efforts are presently underway to convert those cells (which are at present performing well) to SAV.

Inflows to STA-2 include discharges from Pumping Station S-6 and Pumping Station G-328 (an agricultural pumping station situated on the STA-2 Supply Canal intermediate to S-6 and STA-2). Those inflows are considered limited to:

- ➢ Basin runoff from the S-2/S-6 Basin;
- Basin runoff from the East Shore Water Control District/715 Farms Chapter 298 districts (ESWCD/715) diverted from Lake Okeechobee;
- Basin runoff from the S-5A Basin diverted to the Hillsboro Canal through the S-5A Basin Diversion Works.

In addition, analyses summarized in the Supplemental Analysis suggest that a substantial volume of water is introduced to STA-2 as seepage from the L-6 Borrow Canal and WCA-2A, ascribed primarily to the length of the STA-2 Supply Canal between S-6 and STA-2. For this analysis, that induced seepage inflow is assigned at a uniform rate of 38 cfs (27,500 acre-feet per year) and an assigned flow-weighted mean TP concentration of 15 ppb.

In development of the SFWMM 2006 ECP simulation on which the estimated inflow volumes and TP loads are based, certain significant changes in overall system management from historic operations were assumed. Those assumptions include the following that directly and materially influence the projected performance of STA-2 in reducing total phosphorus loads and concentrations:





- Cessation of Lake Okeechobee regulatory releases to the Hillsboro Canal and STA-2 at Structure S-351;
- Water supply releases to the Hillsboro Canal at S-351 destined for the Lower East Coast Service Area 2 (term "WL2351" in the 2006 ECP simulation) would only be made when the stage in WCA-2A is at or below the floor of its regulation schedule, and would bypass STA-2.

Implementation of the first of the above assumptions in the Operations Plan for STA-2 and related elements of the system is critical to the water quality improvement performance projections presented herein. The second assumption addresses relatively minor volumes and TP loads as simulated.

A summary of the estimated average annual inflows to STA-2 is presented in Table 4.1.

Source	Estimated Average Annual Inflow, WY 1966-2000			Remarks	
	Volume (ac-ft)	TP Load (kg)	TP Conc. (ppb)		
S-2/S-6 Basin	226,654	27,015	97	Deliverable 1.5.2, Table 3.2	
ESWCD/715	29,818	4,588	125	Deliverable 1.5.2, Table 2.6	
S-5A Basin	59,342	11,260	154	Deliverable 1.5.2, Table 3.12	
Seepage from WCA-2	27,500	509	15	See text	
				Deliverable 1.1.2 Table 5.1 (WL2351) with Conc from Deliverable 1.5.2, Table	
Lake Okeechobee	461	48	85	6.7	
Total Inflow	343,775	43,420	102		
Assumed Bypass	461	48	85	Water Supply to LEC SA2 (WL2351)	
Inflow to be Treated	343,314	43,372	102		

 Table 4.1 Estimated Inflows to STA-2

#### 4.1. Cases Considered in DMSTA2 Analysis of STA-2

A total of two potential cases were considered in the DMSTA2 analysis of STA-2. The two cases considered are described as follows; each case used the inflow time series summarized in Table 4.1.

Exist: This case was developed upon the assumption that the inflows to STA-2 are to be treated in the existing footprint of STA-2 (e.g., excludes consideration of the influence of the Cell 4 expansion to STA-2). This case was included in the analysis





for reference only, as its inclusion permits an assessment of the impact of the addition of Cell 4. In addition, its inclusion permits an at least approximate comparison to the historic operation of STA-2.

2006 Base: This case varies from "Exist" only in that Cell 4 is considered complete and inflows to STA-2 are redistributed accordingly. Cell 4 was considered as developed in Submerged Aquatic Vegetation (SAV\_3). Dimensional information on Cell 4 was taken from the BODR for STA-2 Cell 4. No other modifications to the existing cells of STA-2 were considered in the analysis.

#### 4.2. Summary of DMSTA2 Results

Table 4.2 presents a summary of the results of the DMSTA2 analyses for STA-2. Summary DMSTA2 input and output data for each case are included in Appendix A.

Parameter	Units	Summary of Results by Case				
		Exist	2006 Base			
Average Annual Inflow						
Volume	1,000 ac-ft	343.6	343.6			
TP Load	metric tons	43.32	43.32			
FWM TP Concentration	ppb	102	102			
Avera	ge Annual Ou	tflow				
Volume	1,000 ac-ft	346.7	347.5			
FWM TP Concentration						
Upper Confidence Limit	ppb	22.5	17.1			
Mean Estimate	ppb	27.8	21.0			
Lower Confidence Limit	ppb	33.8	25.7			
Geometric Mean TP Conc.						
Upper Confidence Limit	ppb	15.9	11.2			
Mean Estimate	ppb	21.1	15.0			
Lower Confidence Limit	ppb	27.2	19.7			
TP Load (Using Mean FWM Conc.)	metric tons	11.87	8.90			
For Detailed Results, See Appe	endix A	Table A.9	Table A.10			
Summary of Bypasses and Diversions						
Water Supply to LEC						
Volume	1,000 ac-ft	0.5	0.5			
TP Load	metric tons	0.04	0.04			
FWM TP Concentration	ppb	85	85			

Table 4.2 Summary of DMSTA2 Analyses, STA-2, WY 1966-2000

Based on the above tabulation, the addition of Cell 4 is projected to reduce mean TP loads in discharges from STA-2 by approximately 25% (as compared to the existing STA-2).





#### 4.2.1. Comparison to Historic Operation

An evaluation of the historic performance to date of STA-2 in reducing total phosphorus loads and concentrations was presented in Table 2.5 of the Supplemental Analysis for Water Years 2002-2004. Given the disparity in period of analysis and inflow time series considered in the Supplemental Analysis to that presented above for Case "Exist:", a direct comparison is not possible.

However, it is noted that the estimated average annual inflow TP loads in Table 4.2 (43.32 metric tons per year) are approximately 92% greater than those summarized in Table 2.5 of the Supplemental Analysis (22.58 metric tons per year). The reduction in TP loads in STA-2 from the above Table 4.2 is 79.5%. The reduction in TP loads in STA-2 from Table 2.5 of the Supplemental Analysis was estimated to be 75.4%.

Employing the simplified, steady-state form of analysis presented in the Supplemental Analysis with the average annual inflow data presented in the above Table 4.1 (using 3 CSTRs in series and a mean steady-state settling rate of 28.4 m/yr), the projected flow-weighted mean TP concentration in discharges from STA-2 would be 28.8 ppb, which compares reasonably well to the mean estimate of 27.8 ppb summarized in Table 4.2 for Case "Exist". It is concluded from this approximate comparison that the DMSTA2 results for existing STA-2 are reasonably consistent with the historic performance of STA-2 summarized in the Supplemental Analysis.

## 5. STA-3/4

For this analysis, all enhancements to STA-3/4 recommended in the Long-Term Plan are considered complete, with but one exception. Cell 1B of STA-3/4 (originally scheduled for conversion to SAV) is considered for this analysis to perform as pre-existing vegetation (PEW\_3), as the District is seeking FDEP approval for a modification of the Long-Term Plan to defer conversion of this well-performing cell. This cell comprises the former Terrytown Wildlife Management Area, and has been out of agricultural production since the early 1990's. The District is currently evaluating methods to convert this cell from emergent to SAV in a manner that would allow continued flow-through operations in lieu of a method that would require taking the cell completely offline to complete the conversion.





Inflows to STA-3/4 include discharges from Pumping Station G-370 (on the North New River Canal) and G-372 (on the Miami Canal). Those inflows are considered to include:

- ▶ Basin runoff from the S-2/S-7 Basin (North New River Canal);
- Regulatory releases from Lake Okeechobee at S-351 directed to the North New River Canal;
- ▶ Basin runoff from the S-3/S-8 Basin (Miami Canal);
- Basin runoff from the Chapter 298 South Shore Drainage District (SSDD) diverted from Lake Okeechobee (diverted to the Miami Canal);
- Basin runoff from the Chapter 298 South Florida Conservancy District No. 5 (SFCD), also known as the S-236 Basin, diverted to the Miami Canal;
- Basin runoff from the C-139 Basin diverted to the Miami Canal through Structure G-136 (term "G136SO" from the ECP 2006 SFWMM simulation);
- > Regulatory releases from Lake Okeechobee at S-354 directed to the Miami Canal.

In development of the SFWMM 2006 ECP simulation on which the estimated inflow volumes and TP loads are based, certain significant changes in overall system management from historic operations were assumed. Those assumptions include the following that directly and materially influence the projected performance of STA-3/4 in reducing total phosphorus loads and concentrations:

Water supply releases to the North New River Canal at S-351 destined for the Lower East Coast Service Area 2 (terms "WL1351" and "WL3351" in the 2006 ECP simulation) would only be made when the stage in WCA-2A (for "WL 1351") or WCA-3A (for "WL-3351") is at or below the floor of their regulation schedules, and would bypass STA-3/4.





Water supply releases to the Seminole Tribe's Big Cypress Reservation at S-354 would bypass STA-3/4.

Implementation of each of the above assumptions in the Operations Plan for STA-3/4 and related elements of the system is critical to the water quality improvement performance projections presented herein.

In addition, the total phosphorus concentration in discharges from the C-139 Basin through G-136 were assumed reduced by 10% from historic levels as a result of ongoing BMP implementation in that basin. A summary of the estimated average annual inflows to STA-3/4 is presented in Table 5.1.

Source	Estimated Average Annual Inflow, WY 1966-2000		Remarks	
	Volume (ac-ft)	TP Load (kg)	TP Conc. (ppb)	
S-2/S-7 Basin	226,012	22,334	80	Deliverable 1.5.2, Table 3.4
S-3/S-8 Basin	232,712	23,617	82	Deliverable 1.5.2, Table 3.8
SSDD	10,559	1,390	107	Deliverable 1.5.2, Table 2.9
SFCD	21,145	3,183	122	Deliverable 1.5.2, Table 2.12
C-139 Basin (G-136)	13,204	2,958	182	Deliverable 1.5.2, Table 4.3
Lake Reg. Release at				
S-351	61,600	5,539	73	Deliverable 1.5.2, Table 6.10
Lake Reg. Release at				
S-354	77,386	5,902	62	Deliverable 1.5.2, Table 6.12
Lake WS Release at				Deliverable 1.5.2, Table 6.7, less
S-351	14,060	1,475	85	WL2351
Lake WS Release at				
S-354	36,624	3,255	74	Deliverable 1.5.2, Table 6.9
Total Inflow	693,302	69,653	81	
				Water Supply to LEC and Big Cypress
Assumed Bypass	50,684	4,730	76	Reservation
Inflow to be Treated	642,618	64,923	82	

Table 5.1 Estimated Inflows to STA-3/4

#### 5.1. Cases Considered in DMSTA2 Analysis of STA-3/4

A total of two potential cases were considered in the DMSTA2 analysis of STA-3/4. The two cases considered are described as follows; each case used the inflow time series summarized in Table 5.1.





- 2006 Base: This case was developed upon the assumption that the inflows to STA-3/4 would be distributed to the three parallel flow paths in such a fashion as to result in essentially equal flow-weighted mean outflow concentrations.
- 2006 Split: This case varies from "2006 Base" only in that inflows to STA-3/4 from the North New River Canal are assigned to Cells 1A and 1B (Case ST3\_06Base), and inflows to STA-3/4 from the Miami Canal are assigned to cells 2A, 2B, 3A, and 3B (Case ST4\_06 Base).

#### 5.2. Summary of DMSTA2 Results

Table 5.2 presents a summary of the results of the DMSTA2 analyses for STA-3/4. Summary DMSTA2 input and output data for each case are included in Appendix A.

Parameter	Units	Sumn	mary of Results by Case				
			2006	Split			
		2006 Base	ST3_06Base	ST4_06Base			
Average Annual Inflow							
Volume	1,000 ac-ft	643.1	287.8	355.3			
TP Load	metric tons	64.94	27.89	37.04			
FWM TP Concentration	ppb	82	79	85			
Total for Case							
Volume	1,000 ac-ft	643.1	64	3.1			
TP Load	metric tons	64.94	64	.94			
FWM TP Concentration	ppb	82	8	32			
	Average A	nnual Outflow					
Volume	1,000 ac-ft	624.2	279.7	344.1			
FWM TP Concentration							
Upper Confidence Limit	ppb	16.2	19.9	15.1			
Mean Estimate	ppb	20.1	25.3	18.2			
Lower Confidence Limit	ppb	24.8	31.4	22.1			
Geometric Mean TP Conc.							
Upper Confidence Limit	ppb	11.9	16.0	10.3			
Mean Estimate	ppb	15.6	21.2	13.2			
Lower Confidence Limit	ppb	20.1	27.3	16.8			
TP Load (Using Mean FWM Conc.)	metric tons	15.46	8.72	7.74			
For Detailed Results, See Appe	endix A	Table A.13	Table A.11	Table A.12			
Total For Case							
Volume	1,000 ac-ft	624.2	623.8				
Flow-Weighted Mean TP Concentration							
Upper Confidence Limit	ppb	16.2	17.3				
Mean Estimate	ppb	20.1	21.4				
Lower Confidence Limit	ppb	24.8	26.3				
TP Load (Using Mean FWM Conc.)	metric tons	15.46	16.46				
Summary of Bypasses and Diversions							
Water Supply Bypass							
Volume	1,000 ac-ft	50.7	50.7				
TP Load	metric tons	4.73	4.	73			
FWM TP Concentration	ppb	76	76				

Table 5.2 Summary	of DMSTA2 Analyses.	STA-3/4.	WY 1966-2000
		, ~ <b>_ _ _ , , ,</b>	>

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From the above, it would appear desirable to attempt to balance the overall inflows to STA-3/4 to redistribute a part of the inflow from the North New River Canal through Structure G-383 to the two westerly flow paths. A part of that need results from the assumed continuation of Cell 1B as "PEW\_3", in lieu of conversion to SAV.

## 6. STA-5

In this analysis, all enhancements to existing STA-5 recommended in the Long-Term Plan are assumed to be complete by the end of 2006. In addition, the proposed third flow-way at STA-5 is assumed complete, generally as described in the BODR for STA-5, for all analyses applicable to the period 2006-2009. Analyses discussed herein also consider the historic operation of STA-5 over Water Years 2002-2005 in an attempt to bench mark the performance to date of STA-5.

Inflows to STA-5 are limited to runoff from the C-139 Basin delivered to the L-3 Borrow Canal. Over the period Water Years 1995-2005, those total inflows are estimated to average 159,030 acre-feet per year at a flow-weighted mean TP concentration of 199 ppb. That mean inflow concentration has been reduced from historic data by 10% in anticipation of reductions in basin TP load discharges resulting from continued BMP implementation in the C-139 Basin. For analysis of historic operations, measured inflow concentrations were used without reduction; inflows to STA-5 were reduced by historic bypass volumes through Structure G-406.

#### 6.1. Cases Considered in DMSTA2 Analysis of STA-5

A total of three potential cases were considered in the DMSTA2 analysis of STA-5. The three cases considered are described as follows:

2006 Base: All inflows to the L-3 Borrow Canal from the C-139 Basin over Water Years 1995-2004 are assigned to STA-5 (e.g., no bypass). Inflow concentrations are assigned at 90% of those measured over Water Years 1995-2005. The BODR for STA-5 was generally silent on the amount of effective treatment area that would be added in the third flow-way. It was assumed for this analysis that the westerly part of the third flow path would be ineffective for treatment, similar to that for the two existing flow paths. In addition, the separation between Cells 3A and 3B was assigned at the location shown in the BODR, which is further east than the separation in the two existing flow paths. New Cell 3A (1,140 acres) was considered





as emergent vegetation (EMG\_3); New Cell 3B (917 acres) was considered as submerged aquatic vegetation (SAV\_3).

- 2006 Base Emg: This case is identical to "2006 Base" with the single exception that the downstream cells (1B, 2B, and 3B) were assigned the EMG\_3 calibration data set in lieu of SAV\_3;
- Exist: This case was structured to parallel as closely as practicable the historic operation of STA-5 over the period Water Years 2002-2005. Total inflows from the C-139 Basin to STA-5 were reduced to reflect those discharges bypassed at G-406. TP concentrations in the inflows were not reduced from historic. Cell 1B was analyzed as Submerged Aquatic Vegetation (SAV\_3), with the other three cells analyzed as Emergent vegetation (EMG\_3). As shown in the DMSTA2 results summarized below, the flow-weighted mean inflow concentration to Cell 1B was estimated to fall essentially equal to the upper end of the calibration range for SAV\_3 (153 ppb), leading to inclusion of the following case in the analysis.
- Exist All Emerg: This case varied from Case "Exist" only in that Cell 1B was analyzed as for Emergent vegetation (EMG\_3) in lieu of SAV.

As outlined above, Cases "2006 Base" and "2006 Base Emg" assumed no bypass from STA-5 to STA-6. This assumption may require additional analysis in the future, as the maximum mean daily inflow in the record for Water Years 1995-2005 was 1,575 cfs (525 cfs per flow path), somewhat in excess of the maximum measured mean daily inflow to Cells 1A and 2A (434 cfs each) over Water Years 2002-2005. Should bypass be experienced, the net result would be a reduction in the estimated TP concentrations and loads discharged from STA-5, with a concurrent increase in the estimated TP concentrations and loads discharged from STA-6.

#### 6.2. Summary of DMSTA2 Results

Table 6.1 presents a summary of the results of the DMSTA2 analyses for STA-5. Summary DMSTA2 input and output data for each case are included in Appendix A. Data for cases





"2006 Base" and "2006 Base Emg" is for the entire period Water Years 1995-2005; data for cases "Exist" and "Exist Emerg" are limited to Water Years 2002-2005.

No rainfall or evapotranspiration data at STA-5 was available from the District-furnished data files after December 31, 2000. As a result, all simulation data subsequent to that date excludes rainfall and evapotranspiration. This exclusion is not expected to materially influence the results of the simulation.

Parameter Unite Summary of Results by Case					
		Eviet			2006 Dees Eme
		EXIST	Exist All Emerg	2006 Base	2006 Base Emg
Effective Treatment Area	acres	4,110	4,110	6,167	6,167
		Average Annual In	flow		
Volume	1,000 ac-ft	150.6	150.6	159.1	159.1
TP Load	metric tons	45.09	45.09	39.14	39.14
FWM TP Concentration	ppb	243	243	199	199
	Α	verage Annual Ou	itflow		
Volume	1,000 ac-ft	150.5	150.5	149.7	149.7
FWM TP Concentration					
Upper Confidence Limit	ppb	51.6	74.9	16.7	41.9
Mean Estimate	ppb	66.5	94.3	22.1	57.2
Lower Confidence Limit	ppb	82.1	113.1	29.6	74.3
Geometric Mean TP Conc.					
Upper Confidence Limit	ppb	47.7	70.8	11.2	35.9
Mean Estimate	ppb	62.3	89.8	16.2	50.5
Lower Confidence Limit	ppb	77.5	108.0	23.2	66.7
TP Load (Using Mean FWM Conc.)	metric tons	12.34	17.52	4.08	10.56
For Detailed Results, See Appendix A		Table A.14	Table A.15	Table A.16	Table A.17
Summary of Bypasses and Diversions					
Bypass at G-406					
Volume	1,000 ac-ft	32.6	32.6	0.0	0.0
TP Load	metric tons	13.39	13.39	0.00	0.00
FWM TP Concentration	ppb	333	333		

Table 6.1 Summary of DMSTA2 Analyses, STA-5

Over the period Water Years 2002-2005, the measured flow-weighted mean TP concentration in discharges from STA-5 was approximately 99 ppb, closely paralleling the mean flow-weighted TP concentration estimated for Case "Exist All Emerg", suggesting that the relatively high inflow concentrations to Cell 1B have influenced the performance of the vegetation in that cell. For the case "2006 Base", the addition of the third flow-way and resulting reduction of loads applied to the existing flow paths in STA-5, coupled with the District's ongoing efforts to address flow distribution and enhancement of the existing STA-5, provides a degree of assurance that the downstream cells of the expanded STA may be forecast to perform more as SAV\_3 than has historically been the case. However, until such improvement in performance of STA-5 as encompassing the full range of uncertainty in



performance of the three downstream cells (e.g., range from upper limit of performance for SAV\_3 to the lower limit of performance for EMG\_3).

## 7. STA-6

For analysis of the period 2006-2009, STA-6 is considered to have been expanded to include STA-6 Section 2. The general configuration of Section 2 has been taken from the BODR for STA-6 Section 2; although not stated expressly in the BODR, it has been assumed for this analysis that Section 2 will be developed in Submerged Aquatic Vegetation. Enhancements to STA-6 Section 1 originally recommended in the Long-Term Plan are assumed not to be complete, consistent with the District's intent as stated in its December 2004 amendment to the Long-Term Plan.

Sources of inflow to STA-6 include the following:

- Runoff from the former U.S. Sugar Corporation's Southern Division Ranch, Unit 2. For the period 2006-2009, historic discharges from that area have been reduced 20% to reflect the conversion of certain lands in Unit 2 to use in STA-6 Section 2 and the third flow-way at STA-5;
- Runoff from the C-139 Annex;
- Bypass at G-406 of C-139 Basin runoff exceeding the inflow capacity of the G-342 inflow control structures at STA-5. As discussed earlier for STA-5, this analysis has assumed no bypass of C-139 Basin runoff to STA-6.

A summary of the estimated average annual inflows to STA-6 during the period 2006-2009 is presented in Table 7.1. Average annual inflows from Unit 2 are based on record data over the period Water Years 1998-2005, reduced by 20% as stated above. Average annual inflows from the C-139 Annex are based on record data from the C-139 Annex (as measured at station "USSO") over the period Water Years 1997-2005.



Source	Estimated Average Annual Inflow			Remarks
	Volume (ac-ft)	TP Load (kg)	TP Conc. (ppb)	
USSC SDR Unit 1				
(WY 1998-2005)	38,400	3,447	74	Deliverable 1.5.2, Table 4.6
C-139 Annex (WY				
1997-2005)	40,176	4,873	98	Deliverable 1.5.2, Table 4.5
Total Inflow	78,576	8,320	86	
Assumed Bypass	0	0		No bypass assumed
Inflow to be Treated	78,576	8,320	86	

#### Table 7.1 Estimated Inflows to STA-6

#### 7.1. Cases Considered in DMSTA2 Analysis of STA-6

A total of five potential cases were considered in the DMSTA2 analysis of STA-6. The five cases considered are described below. Two cases (SEC1\_SDR and HIST\_SAV) were included in the analysis for comparison to the historic performance of STA-6 Section 1. Two cases (Sect1 \_USSO and Sect1\_USSO\_SAV) are more generally applicable to the period after 2009, but were included herein primarily as a matter of interest. The fifth case (2006 Base) is considered the case directly applicable to anticipated inflows and operations during the period 2006-2009.

- SEC1\_SDR: This analysis was intended to parallel as closely as practicable the actual performance of STA-6 Section 1 over the period Water Years 2001-2004. Inflows from the USSC Southern Division Ranch Unit 2 were assigned at recorded values (e.g., not reduced to reflect the future conversion of lands to use in STA-6 Section 2 and the third flow-way at STA-5). Vegetation in STA-6 Section 1 was analyzed as pre-existing wetland vegetation (PEW\_3). The results of this analysis appeared to over-predict the flow-weighted mean TP concentration in discharges from STA-6 Section 1, leading to inclusion of the following case in the analysis;
- HIST\_SAV: This case was developed identical to Case "SEC1\_SDR", with the lone variation being the vegetation was assumed to act as Submerged Aquatic Vegetation (SAV\_3);
- 2006 Base: This case was developed to reflect the anticipated treatment area configuration and operation expected to exist over the period 2006-2009. The analysis extends over the period Water Years 1998-2005 (no data for the Southern Division Ranch was available in Water Year 1997). For this analysis, the vegetation in Section 1 (Cells 3)




and 5) was assigned as pre-existing wetland vegetation (PEW\_3); the vegetation in Section 2 was assigned as SAV\_3.

- Sect1\_USSO: This case was developed for potential applicability post-2009, and was structured on the basic assumption that STA-6, Section 1 would be dedicated to runoff from the C-139 Annex. Vegetation in Section 1 was considered as PEW\_3. The analysis considers all available data at station USSO (Water Years 1997-2005);
- Sect1\_USSO\_SAV: This case is identical to the case described immediately above, with the exception that the vegetation in Section 1 was considered as SAV\_3 in lieu of PEW\_3.

# 7.2. Summary of DMSTA2 Results

Table 7.2 presents a summary of the results of the DMSTA2 analyses for STA-6. Summary DMSTA2 input and output data for each case are included in Appendix A.

Parameter Units Summary of Results by Case								
		SEC1_SDR	HIST_SAV	2006 Base	Sect1_USSO	Sec1_USSO_SAV		
Effective Treatment Area	acres	897	897	2,197	897	897		
		Average	Annual Inflow	- 	• 	•		
Volume	1,000 ac-ft	50.5	50.5	78.6	40.2	40.2		
TP Load	metric tons	5.06	5.06	8.30	4.88	4.88		
FWM TP Concentration	ppb	81	81	86	98	98		
		Average A	Innual Outflow			•		
Volume	1,000 ac-ft	44.3	44.3	70.7	34.1	34.1		
FWM TP Concentration								
Upper Confidence Limit	ppb	22.1	16.4	11.8	19.4	14.1		
Mean Estimate	ppb	27.7	19.5	14.3	25.1	17.0		
Lower Confidence Limit	ppb	34.0	23.3	17.6	32.0	20.6		
Geometric Mean TP Conc.								
Upper Confidence Limit	ppb	17.5	12.1	7.7	15.5	10.3		
Mean Estimate	ppb	22.7	15.1	10.3	21.2	13.1		
Lower Confidence Limit	ppb	28.4	18.6	13.7	28.0	16.7		
TP Load (Using Mean FWM Conc.)	metric tons	1.52	1.07	1.25	1.05	0.71		
For Detailed Results, See Appe	endix A	Table A.18	Table A.19	Table A.20	Table A.21	Table A.22		

 Table 7.2 Summary of DMSTA2 Analyses, STA-6

No rainfall or evapotranspiration data at STA-6 was available from the District-furnished data files after December 31, 2000. As a result, all simulation data subsequent to that date excludes rainfall and evapotranspiration. This exclusion is not expected to materially influence the results of the simulation.

Table 2.11 of the Supplemental Analysis summarizes the historic performance of STA-6 Section 1 over the period Water Years 2001-2004, and may be considered to somewhat



parallel the above DMSTA2 analyses for "SEC1\_SDR" and "HIST\_SAV". As developed in the Supplemental Analysis, the estimated flow-weighted mean TP concentration in discharges from STA-6 Section 1 over that period was 21 ppb, falling intermediate to the mean flow-weighted estimates presented in Table 7.2 for that same period, but falling closer to that resulting from an assumption that the vegetation in Section 1 acts as "SAV\_3".

That result is consistent with observations made by Walker. It is possible that the inflow time series from the Southern Division Ranch (based on historical records) overestimates the inflows to STA-6 at G-600, as the recorded flows at G-600 do not reflect (unrecorded) irrigation withdrawals from the tailwater of G-600 (which would act to reduce the actual inflows to STA-6). Walker reports his simulations of STA-6 using the PEW\_3 calibration also over-estimated the observed outflow concentrations by approximately 15%. The results of the DMSTA2 simulations would place the historic flow-weighted mean TP concentration in outflows from STA-6 within the range of estimates for both PEW\_3 (historic concentration approaching the upper confidence limit) and SAV\_3 (historic concentrations approaching the lower confidence limit).

# 8. SUMMARY PROJECTIONS

A summary of the projected performance of the various stormwater treatment areas over the period 2006-2009 is presented in Table 8.1. That tabulation includes identification of the specific case for each STA considered as most applicable to this summary. That tabulation also summarizes all bypass volumes and TP loads presented in earlier sections of this document. The results presented in Table 8.1 for STA-5 include the full range of uncertainty associated with the performance of the three downstream cells.





Parameter	Units		Summary of DMSTA2 Results by Treatment Area and Case									
		STA-1W	STA-1E	STA-2	STA-3/4	STA-5*	STA-6					
		2006 Mod	2006 Mod	2006 Base	2006 Base	2006 Base	2006 Base	All				
Effective Treatment Area	acres	6,670	6,175	8,140	16,543	6,167	2,197	45,892				
			Average A	Annual Inflow								
Volume	1,000 ac-ft	175.1	242.9	343.6	643.1	159.1	78.6	1642.4				
TP Load	metric tons	37.7	41.16	43.3	64.94	39.14	8.30	234.52				
FWM TP Concentration	ppb	174.3	137	102	82	199	86	116				
			Average A	nnual Outflow								
Volume	1,000 ac-ft	176.7	240.9	347.5	624.2	149.7	70.7	1609.7				
FWM TP Concentration												
Upper Confidence Limit	ppb	16.7	19.3	17.1	16.2	16.7	11.8	16.8				
Mean Estimate	ppb	20.3	25.2	21.0	20.1	39.7	14.3	22.6				
Lower Confidence Limit	ppb	25.2	32.3	25.7	24.8	113.1	17.6	34.1				
Geometric Mean TP Conc.												
Upper Confidence Limit	ppb	9.8		11.2	11.9	11.2	7.7					
Mean Estimate	ppb	13.5		15.0	15.6	33.4	10.3					
Lower Confidence Limit	ppb	18.5		19.7	20.1	66.7	13.7					
TP Load (Using Mean FWM Conc.)	metric tons	4.43	7.48	8.90	15.46	7.3	1.25	44.84				
			Summary of Bypas	ss Volumes and Lo	ads							
Bypass Volume, TP Load and TP Cor	centration for	each Treatment A	rea									
Volume	1,000 ac-ft	16.7	71.5	0.5	50.7	0.0	0.0	139.4				
TP Load	metric tons	2.47	9.41	0.04	4.73	0.00	0.00	16.64				
FWM TP Concentration	ppb	120	107	66	76			97				

Table 8.1 Summary Projections for all STAS, for Period 2006-2005	Tab	ole 8	8.1	Summary	Projections	for	all STAs,	for	Period	2006-2009
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\* At STA-5, upper confidence limit reported based on the assumption that the three downstream cells act as SAV\_3; lower confidence limit reported based on the assumption that the three downstream cells act as EMG\_3. Mean estimate of outflow concentration and outflow TP load taken as average of those two conditions

In the above table, bypasses at STA-1E are untreated bypass through S-155A. All other bypasses indicated in Table 8.1 consist of water supply releases bypassing the STAs. The sensitivity of the outflow projections to the assumption that those water supply releases bypass the STAs is examined later in this Part 8.

The total inflow volume shown in Table 8.1 varies from that reported in Table 7.1 of Deliverable 1.5.2 due primarily to:

- The addition of 27,500 acre-feet per year in STA-2 inflows due to seepage return to the STA-2 Supply Canal from the L-6 Borrow Canal and WCA-2A;
- At STA-1E, the westerly flow path was analyzed separately from the easterly two flow paths. Seepage lost from the West Distribution Cell to the C-51 West Canal was added to the projected inflows to the easterly two flow paths – total outflow volumes from STA-1E were not affected by that assignment.





The estimated values of inflow volumes and TP loads to the various STAs are materially and significantly influenced by system management choices reflected in the SFWMM 2006 ECP simulation and described in detail in earlier sections of this document. Principal among those management choices are the elimination of Lake Okeechobee regulatory releases to the West Palm Beach Canal and L-8 Borrow Canal; the assumption that Lake Okeechobee water supply releases destined for the Lower East Coast (when receiving WCA's are at or below the floor of their respective regulation schedules) and the Big Cypress Reservation will bypass the STAs; and that the volume of L-8 Basin runoff entering the C-51 West Canal will be bypassed untreated through Structure S-155A.

Table 8.2 summarizes estimated average annual back pumping or back flow to Lake Okeechobee during the period 2006-2009.

Location	Estimated Ave.	Annual Discharge,	Remarks	
	Volume (ac-ft)	TP Load (kg)	TP Conc. (ppb)	
S-2 (S-2/S-6/S-7)	42,554	4,640	88	Deliverable 1.5.2, Table 3.6
S-3 (S-3/S-8)	5,921	594	81	Deliverable 1.5.2, Table 3.9
C-10A (L-8)	49,905	6,474	105	Deliverable 1.5.2, Table 5.4
Total Discharge	98,380	11,708	96	

# 8.1. Sensitivity of Projections to Assumption of Water Supply Bypass

As noted throughout this document, the water quality analyses summarized in Table 8.1 were developed upon the assumption that water supply releases destined for the Lower East Coast and certain other destinations (such as the Big Cypress Reservation) are permitted to bypass the STAs when the receiving water conservation area is at or below the floor of its regulation schedule. For the period 2006-2009, this assumption is of particular significance only at STA-3/4 and at the STA-1 complex. Additional DMSTA2 simulations were conducted to assess the impact of inclusion of those water supply releases in the inflows to the STAs.





# 8.1.1. STA-3/4

As summarized in Table 8.1, an average annual volume of just less than 51,000 acre-feet per year (at a flow-weighted mean TP concentration of 76 ppb) was assumed to bypass STA-3/4. The STA-3/4 simulation summarized in Table 8.1 was modified to include those water supply releases in the inflows to STA-3/4. Detailed output from that DMSTA2 simulation (Case "2006 WS") is presented in Appendix A (Table A.23).

Inclusion of those water supply releases in the inflows to STA-3/4 would act to increase the average annual inflow volume by 7.8%, and the average annual inflow load by 7.4%, as compared to the "2006 Base" case for STA-3/4. The estimated flow-weighted mean TP concentration in the outflows from STA-3/4 is estimated to increase from 20.1 ppb to 20.3 ppb (using the mean estimate); the average annual TP load in the outflows from STA-3/4 is estimated to increase from 15.46 metric tons per year to 16.88 metric tons per year (an increase of 9.2%). Overall TP loads (including both treated and bypass flows) would reduce from 20.19 metric tons per year for the "2006 Base" simulation (for which bypasses are assumed) to 16.88 metric tons per year for the "2006 WS" simulation (which has no bypass).

#### 8.1.2. STA-1W

As summarized in Table 8.1, an average annual volume of just less than 17,000 acre-feet per year (at a flow-weighted mean TP concentration of 120 ppb) was assumed to bypass STA-1W and STA-1E. Of that total, slightly more than 3,600 acre-feet were simulated water supply releases directed to the LNWR, with the balance consisting of water supply releases directed to the L-8 Borrow Canal (for which no need for treatment is assumed). Inspection of data summarized in Tables 2.2 (for STA-1W) and 3.2 (for Cells 5-7 of STA-1E) for Case "2006 Mod" indicates that the projected outflow concentrations from the westerly flow path of STA-1E are consistently above those projected at STA-1W. The STA-1W simulation summarized in Table 8.1 was modified to include Lower East Coast water supply releases in the inflows to STA-1W. Detailed output from that DMSTA2 simulation (Case "2006 WS") is presented in Appendix A (Table A.24).

Inclusion of those water supply releases in the inflows to STA-1W would act to increase the average annual inflow volume by 2.1%, and the average annual inflow load by 1.3%,





as compared to the "2006 Mod" case for STA-1W. The estimated flow-weighted mean TP concentration in the outflows from STA-1W is estimated to increase from 20.3 ppb to 20.5 ppb (using the mean estimate); the average annual TP load in the outflows from STA-1W is estimated to increase from 4.43 metric tons per year to 4.56 metric tons per year (an increase of 2.9%). Overall TP loads (including both treated and bypass flows) would reduce from 4.96 metric tons per year for the "2006 Mod" simulation (for which bypasses are assumed) to 4.56 metric tons per year for the "2006 WS" simulation (which has no bypass).

It is possible that, should the revised inflows reflected in the "2006 Mod" simulations for STA-1W and STA-1E be implemented, the volume of water supply release to the LNWR would increase by some (presently not quantifiable) amount, as the total discharge from STA-1W and STA-1E to the Refuge would reduce from an estimated average annual volume of 484 thousand acre feet per year (as for cases "2006 All") to 418 thousand acre feet per year. Accordingly, the actual impact of introducing those water supply releases to STA-1W for treatment could exceed that discussed above.

# 8.2. Additional Analyses for STA-1E

Review of the summary data presented in Table 8.1 leads to the observation that, with the exception of STA-5 (for which uncertainty in the performance of the SAV community in the downstream cell of each flow path leads to a wide range of potential performance), the highest estimated flow-weighted mean TP concentrations in treatment area outflows occur at STA-1E. Estimated outflow concentrations at this STA are roughly 20% greater than those estimated for the other treatment areas (again excluding STA-5). Given that result, and the presence of a means to further reduce the inflow volumes and TP loads to STA-1E (through increased bypass to the C-51 East Canal at S-155A), it is considered appropriate to consider the impact of an increased bypass through S-155A on estimated outflow concentrations from STA-1E.

In addition, the analyses summarized in Table 8.1 were developed upon the assumption that the full area of STA-1E is available for use. However, during much of the period 2006-2009,





it is anticipated that use of Cells 1 and 2 will be curtailed due to the construction and operation of a Periphyton Assisted Stormwater Treatment Area (PSTA) demonstration project in Cell 2 being designed and constructed by the Jacksonville District, U.S. Army Corps of Engineers.

Additional analyses of STA-1E were conducted to assess the potential influence on treatment performance that would result from consideration of both of the above modifying conditions.

#### 8.2.1. Influence of PSTA Demonstration Project

The most recent information on the basic character and design of the PSTA demonstration project in Cells 1 and 2 was taken from the June 28, 2005, (*Draft*) Design Analysis Report for the STA-1E Cells 1-2 PSTA/SAV Field Scale Demonstration Project. A draft water control and operations plan for that project is being prepared, but was not available for consideration at the time of this analysis.

The apparent maximum design rate of discharge through Cells 1 and 2 during operation of the PSTA/SAV demonstration project, taken from the *Design Analysis Report*, is 55.32 cfs. In comparison, the maximum design rate of discharge through Cells 1 and 2 considered in the original design of STA-1E was 860 cfs. In the absence of a water control and operations plan for the demonstration project, it is not possible to project a time series of inflows to Cells 1 and 2 of STA-1E. Analysis of the projected treatment performance of STA-1E, given the presence of the demonstration project, was conducted upon the assumption that Cells 1 and 2 are off line and unavailable for use. This assumption is conservative in nature, but does permit the maximum flexibility in actual operation of the demonstration project to achieve its objectives. Any beneficial result from the direction of STA-1E inflows through Cells 1 and 2 would serve to modify (improve) the projections summarized herein.

The DMSTA2 input file for STA-1E "2006 Mod", Cells 1-4 (see Table 3.2) was selected as the base condition for analysis of the impact of the Cells 1-2 PSTA/SAV Demonstration Project. The input data for that case was modified to:





- ➢ Eliminate Cells 1 and 2;
- Direct all inflows to STA-1E at Pump Station S-319 through Cells 3-4S (including the west half of the East Distribution Cell), limited only by the original maximum design rate of discharge through those cells (1,540 cfs).

The inflow fraction to Cells 3-4S was assigned at 100% of the discharge at S-319, with bypass of all discharges above 1,540 cfs. Those bypassed volumes are considered released to the C-51 East Canal at S-155A, in essence adding to the bypass volumes originally considered for Case "2006 Mod". Detailed DMSTA2 input and output data for this new case at STA-1E (Case "3\_4 2006PSTA") are presented in Table A.25 of Appendix A. Estimated flow-weighted mean outflow concentrations from Cells 3-4S are roughly 50% greater than for Case "2006 Mod" in Cells 1-4S (mean estimate of the long-term flow-weighted mean TP concentration in discharges from Cells 3-4S are projected to increase from 24.3 ppb to 37.1 ppb).

It is estimated that it would be possible to pass virtually the entire inflow at S-319 through Cells 3-4S, despite the disparity between the design capacity of S-319 (3,980 cfs) and the design capacity of the S-366 inflow control structures to Cell 3 (1,540 cfs). In the ECP 2006 SFWMM simulation, the maximum daily inflow (summation of C-51 West Basin and Acme Improvement District, Basin B runoff) over Water Years 1966-2000 is shown as 1,714 cfs, suggesting that the capacity of S-319 greatly exceeds that necessary to accommodate runoff from those two sources. The combined area of the C-51 West Basin and Acme Improvement District, Basin B is approximately 53,100 acres. The 3,980-cfs capacity of S-319 is equivalent to a removal rate of 1.78 inches per day from that tributary area. The maximum simulated inflow rate of 1,714 cfs is equivalent to a removal rate of 0.77 inches per day from that tributary area. The maximum daily simulated runoff from the 44,400-acre C-51 West Basin is 1,435 cfs (0.77 inches per day).





The results of Case "3\_4 200PSTA" in Cells 3-4S must be considered with some caution due to the relatively low simulated daily removal rate from the C-51 West Basin. It is possible that actual daily removal rates may be higher than reflected in the simulation, with the result that a limitation on inflow rates to Cell 3 based on its hydraulic capacity could increase bypass volumes at S-155A to greater extent than the results of this analysis would suggest.

#### 8.2.2. Analysis for Increased Bypass at S-155A

As summarized in Table 8.1, the estimated average annual volume of bypass at S-155A for Case "2006 Mod" at STA-1E is roughly 71,500 acre-feet per year, established equal to the simulated inflows to the C-51 West Canal resulting from L-8 Basin runoff. However, Section 7.11 "L-8 Basin- Interim Operations" of the (*Draft*) Stormwater Treatment Area 1-East (STA-1E) Water Control Plan provides that:

- 1. Primary outlets are Lake Okeechobee and the Refuge, however, in order to minimize the discharge of untreated water to the Refuge, the operation of the S-5A structures, G-311, S-319, and S-155A will be coordinated in order to either treat L-8 runoff in STA-1E or discharge to the east to the maximum extent practical based on available capacity.
- 2. To ensure the STA-1E is not overloaded, until the L-8 Basin runoff is diverted north into the proposed CERP project, the operation of the S-155A divide will be synchronized with S-5AE and S-319 to pass at least the same volume of stormwater to tide as L-8 presently discharges to C-51 (approaching 150,000 AF/yr). Water passing through S-155A will be a mixture of L-8 and C-51W basin runoff.

The DMSTA2 analysis described above for Case "3\_4 2006PSTA" was modified to increase the volume bypassed at S-155A for consistency with the above provisions of the *(Draft) Water Control Plan.* The target average annual bypass at S-155A is considered to consist of:

> The currently simulated bypass of approximately 71,500 acre-feet per year;





- An allowance of approximately 6,000 acre-feet per year for seepage inflows to the C-51 West Canal not considered in the analysis for Case "3\_4 2006PSTA";
- An additional bypass of approximately 72,500 acre-feet per year through S-155A.

The additional bypass at S-155A was "forced" in the simulation by limiting inflows to STA-1E at S-319 to 55% of the combined volume of runoff from the C-51 West Basin and Acme Improvement District, Basin B. Detailed DMSTA2 input and output data for this case are included as Table A.26 in Appendix A.

A summary of the overall projected outflows from STA-1E for a modified, interim operation to reflect both the impact of the PSTA/SAV Demonstration project in Cells 1 and 2 and the potential for increased bypasses through S-155A (within the limits defined in the (*Draft*) *Water Control Plan* for STA-1E is presented in Table 8.3. Inflows to Cells 5-7 would be identical to those for Case "5\_7 2006 Mod"; inflows to Cells 3-4S are taken from "3\_4 PSTADiv".

It is possible that, should the revised inflows reflected in Table 8.3 STA-1E be implemented, the volume of water supply release to the LNWR would increase by some (presently not quantifiable) amount, as the total discharge from STA-1W and STA-1E to the Refuge would reduce from an estimated average annual volume of 484 thousand acre feet per year (as simulated in the ECP 2006 SFWMM simulation) to 333 thousand acre feet per year. That reduction could trigger the need for additional water supply releases to STA-1W and STA-1E, impacting treatment projections to the extent those additional water supply releases might first be introduced to the STAs for treatment.





Parameter	Units	Summary of Results by Case						
		2006	Mod					
		3_4 PSTADiv	5_7 2006 Mod					
Aver	age Annual In	flow						
Volume	1,000 ac-ft	88.7	75.1					
TP Load	metric tons	13.23	16.14					
FWM TP Concentration	ppb	121	174					
Total for Case								
Volume	1,000 ac-ft	16	3.8					
TP Load	metric tons	29	.37					
FWM TP Concentration	ppb	14	45					
Avera	age Annual Ou	Itflow						
Volume	1,000 ac-ft	84.6	71.6					
FWM TP Concentration								
Upper Confidence Limit	ppb	12.8	21.5					
Mean Estimate	ppb	17.0	27.2					
Lower Confidence Limit	ppb	22.6	34.9					
Geometric Mean TP Conc.								
Upper Confidence Limit	ppb	9.4	12.3					
Mean Estimate	ppb	13.5	17.5					
Lower Confidence Limit	ppb	18.9	24.5					
TP Load (Using Mean FWM Conc.)	metric tons	1.78	2.41					
For Detailed Results, See App	endix A	Table A.26	Table A.8					
Total For Case								
Volume	1,000 ac-ft	15	6.2					
Flow-Weighted Mean TP Concentra	ation	-						
Upper Confidence Limit	ppb	15	5.4					
Mean Estimate	ppb	20	).0					
Lower Confidence Limit	ppb	26.3						
TP Load (Using Mean FWM Conc.)	metric tons	4.18						

# Table 8.3 Modified Interim Operations at STA-1E

# 9. PRINCIPAL CONCLUSIONS

Review of the summary projections presented in Table 8.1 suggests that, for conditions expected to prevail over the period 2006-2009, there would be little value in attempting to redistribute the inflow volumes and TP loads to the various STAs, with one exception. That exception is for the combined operation of STA-1W and STA-1E. It would appear desirable to redistribute volumes and loads discharged from the STA-1 Inflow and Distribution Works, reducing the proportion delivered to STA-1W and increasing the proportion delivered to STA-1E, so that projected outflow concentrations from those two treatment areas are more closely in parallel. However, that redistribution, in and of itself, would not materially change the aggregate of total phosphorus loads delivered to the Loxahatchee National Wildlife Refuge (LNWR).





However, the volumes and TP loads discharged to the LNWR are materially influenced by the assumption that volumes associated with L-8 Basin runoff will bypass both STA-1W and STA-1E, being delivered first through S-5AE to the C-51 West Canal and then discharged at Structure S-155A. It is not apparent that sufficient hydraulic capacity presently exists in the water control structures to effect that assumption. It would appear desirable that, under Phase 1, Task 3 of District Contract No. CN040912-WO03, the potential need for increasing the capacity of S-5AE (and, potentially, S-155A) prior to the end of 2006 be evaluated in detail.

An interim operation of STA-1E, in which Cells 1 and 2 are considered to be off line due to the construction and operation of the PSTA/SAV Demonstration Project by the Jacksonville District, U.S. Army Corps of Engineers, coupled with an increased bypass through S-155A, could be expected to:

- > Reduce overall TP loads discharged to the LNWR;
- Result in a relatively close balance in the estimated TP concentrations in outflows from STA-1E and STA-1W;
- Increase (as compared the data summarized in Table 8.1) volumes and TP loads bypassed to the C-51 East Canal through S-155A, but within the constraints permitted by the (*Draft*) Water Control Plan for STA-1E.

That interim operation could be considered to further delay realization of the full flood control benefits of the Central and Southern Florida Flood Control Project in the C-51 Basin, and would require additional analysis of the capacity and operations of S-155A.

\* \* \* \* \*





# Appendix A DMSTA2 Output Data List of Tables

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Table A.26 STA-1E: Case "3_4 PSTADiv"

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# Table A.1 STA-1W: Case "2006 All"

	llnite	Value	Project:	PROJECT_S								Cu	rrent Date:	08/03/05
Design Case Name Input Series Name	-	2006_All TS_2006All	STA-1W wit Inflows Inclu	tion: h Long Term P de all S-5A Ba	lan Enhancem sin and EBWC	ents D Runoff fro	om ECP 20	06 Simulatio	on (no diver	sion to STA	-1E)			
Starting Date for Simulation Ending Date for Simulation		05/01/65 04/30/00	Seepage inf Cell-to-cell s	lows from WCA	A-1 based on i nsidered in an	mean stage alysis	of 15.75 ft.	NGVD						
Starting Date for Output Integration Steps Per Day	1	05/01/65 4	Simulation Typ	be:	Uncertainty A	nalysis								
Number of Iterations Output Averaging Interval	- days	0 7	Output Variat FWM Outflow	e (ppb)	Mean 30.0	Lower CL 37.3	Upper CL 24.2		Diagnostic: H20 Baland	s ce Error Me	an & Max	0.0%	0.0%	
Inflow Conc Scale Factor Rainfall P Conc	- ppb	1 10	GM Outflow C Load Reducti	c (ppb) on %	21.8 83%	29.1 79%	16.0 86%		Mass Balar Iterations 8	Converge	lean & Max nce	-0.2%	0.4% 0.1%	
Atmospheric P Load (Dry) Cell Number>	mg/m2-yr	20	Bypass Load	(%) 3	0.0% 4	5	6	7	Warning/E	rror Messag 9	jes 10	<u>9</u> 11	12	r
Vegetation Type	>	EMG_3 0.38	SAV_3	SAV_3	EMG_3 0.17	SAV_3	SAV_3	EMG_3 0.45	SAV_3					
Downstream Cell Number Surface Area	- km2	2 3.02	3 3.02	4.15	5	6 1.91	1.45	8 2.27	9.28					
Mean Width of Flow Path Number of Tanks in Series	km -	1.10 2.0	1.10 2.0	1.10 2.0	2.40 2.0	2.00 2.0	1.30 2.0	1.78 2.0	2.34 3.0					
Minimum Depth for Releases Release 1 Series Name	cm													
Release 2 Series Name Outflow Series Name														
Depth Series Name Outflow Control Depth	cm	55	55	46	60	60	60	60	60					
Outflow Coefficient - Exponent	-	4	4	4	4	4	4	4	4					
Bypass Depth Maximum Inflow	cm hm3/dav		•					•						
Maximum Outflow Inflow Seepage Rate	hm3/day (cm/d) / cm	0.0035	0.0018	0.0023										
Inflow Seepage Control Elev Inflow Seepage Conc	cm ppb	172 20	172 20	185 20										
Outflow Seepage Rate Outflow Seepage Control Elev	(cm/d) / cm cm			0.0014	0.0016	0.0016	0.0021	0.0156	0.0049					
Seepage Recycle to Cell Number	ррь			20	20	20	20	1	20					
Seepage Discharge Fraction	-	30	30	30	30	30	30	30	30					
Initial P Storage Per Unit Area Initial Water Column Depth	mg/m2 cm	500 200	500 200	500 200	500 200	500 200	500 200	500 200	500 200					
C0 = Conc at 0 g/m2 P Storage C1 = Conc at 1 g/m2 P storage	ppb ppb	3 22	3 22	3 22	3 22	3 22	3 22	3 22	3 22					
C2 = Conc at Half-Max Uptake K = Net Settling Rate at Steady State	ppb m/yr	300 16.8	300 52.5	300 52.5	300 16.8	300 52.5	300 52.5	300 16.8	300 52.5					
Z1 = Saturated Uptake Depth Z2 = Lower Penalty Depth	cm	40	40	40 100	40	40 100	40 100	40	40 100					
Output Variables	Units	200	200	200	200	200	<u>200</u>	200	200		10	11	12	Overall
Execution Time Run Date	sec/yr	6.94 08/03/05	7.31 08/03/05	7.69	8.06 08/03/05	8.40 08/03/05	8.77 08/03/05	9.14 08/03/05	9.60 08/03/05					9.60 08/03/05
Starting Date for Simulation Starting Date for Output	1	05/01/65 05/01/65	05/01/65 05/01/65	05/01/65 05/01/65	05/01/65 05/01/65	05/01/65 05/01/65	05/01/65 05/01/65	05/01/65 05/01/65	05/01/65 05/01/65					05/01/65 05/01/65
Ending Date Output Duration	- days	04/30/00 12784	04/30/00 12784	04/30/00 12784	04/30/00 12784	04/30/00 12784	04/30/00 12784	04/30/00 12784	04/30/00 12784					04/30/00 12784
Cell Label Downstream Cell Label		1A 1B	1B 3	3 Outflow	2A 2B	2B 4	4 Outflow	5A 5B	5B Outflow					Total
Network Simulation Name Simulation Type	-	none Uncerta	none Uncerta	none Uncerta	none Uncerta	none Uncerta	none Uncerta	none Uncerta	none Uncerta					none Uncerta
Mean Rainfall Mean FT	cm/yr	134.9 129.8	134.9 129.8	4.15 134.9 129.8	134.9	134.9	134.9	134.9	9.28 134.9 128.4					134.9
Cell Inflow Volume Cell Inflow Load	hm3/yr kg/yr	117.3 20442	145.7 17281	147.9 9555	52.5 9145	51.4 6435	50.4 2596	138.9 24208	126.7 17589					308.6 53796
Cell Inflow Conc Treated Outflow Volume	ppb hm3/yr	174.3 145.7	118.6 147.9	64.6 149.4	174.3 51.4	125.2 50.4	51.6 49.3	174.3 126.7	138.8 111.0					174.3 309.6
Treated Outflow Load Treated FWM Outflow Conc	kg/yr ppb	17281 118.6	9555 64.6	4553 30.5	6435 125.2	2596 51.6	1406 28.5	17589 138.8	3337 30.1					9296 30.0
Upper Confidence Limit Lower Confidence Limit	ppb ppb	124.1 111.9	74.4 54.7	38.4 23.9	134.5 114.4	62.3 42.0	35.8 23.0	144.3 132.4	36.4 25.0					37.3 24.2
Total Outflow Load + Bypass	kg/yr	145.7	9555	4553	6435 125.2	2596 51.6	49.3	126.7	3337					9296.3 30.0
Bypass Load Bypass Load	kg/yr %	0	0	0	0	0	0.0	0.0	0					0.0
Maximum Inflow Maximum Outflow	hm3/d hm3/d	3.51 3.62	3.62 3.78	3.78 3.96	1.57 1.56	1.56 1.63	1.63 1.68	4.15 4.02	4.02 4.33					9.23 9.97
Surface Load Reduction Load Trapped in Sediments	kg/yr kg/yr	3161 3810	7726 7865	5002 5137	2710 2630	3839 3843	1189 1212	6619 2881	14252 13377					44500 40754
Overall Load Reduction Lower Confidence Limit	%	15% 12%	45% 39%	52% 48%	30% 24%	60% 55%	46%	27%	81% 78%					83% 79%
Daily Geometric Mean	ppb	103.2	52.8 53.7	22.4	109.2	35.3 42.1	46% 15.8 20.4	151.0	19.3 21.8					#N/A 21.8
Upper Confidence Limit	ppb	110.3	63.43 44.03	30.4	126.3 105.1	52.6 32.9	27.4	134.3	27.8					29.1 16.0
Frequency Outflow Conc > 10 ppb Frequency Outflow Conc > 20 ppb	%	100% 100%	100% 100%	100% 66%	100% 100%	100% 100%	97% 51%	100% 100%	99% 58%					100% 89%
Frequency Outflow Conc > 50 ppb Freq Outflow Volume > 10 ppb	% %	100% 100%	64% 100%	1% 100%	100% 100%	26% 100%	1% 99%	100% 100%	2% 100%					60% 100%
95th Percentile Outflow Conc Mean Biomass P Storage	mg/m2	131 3970	78 2615	39 1240	138 4332	65 2020	36 837	165 3978	38 1444					36 2251
Net Storage Turnover Rate	% 1/yr	11.1	34.9	34.9	11.1	34.9	34.9	11.1	34.9 1442					23.5
Mean Water Load Max Water Load	cm/d cm/d	10.6	13.2 120.1	9.8 91.0	7.5	7.4	9.5	16.7	3.7					3.1 34.2
Mean Depth Minimum Depth	cm	70 55	71 54	70 43	58 13	57 13	57 7	52 1	53 1					60 21
Maximum Depth Frequency Depth < 10 cm	cm %	132 0.0%	134 0.0%	136 0.0%	88 0.0%	93 0.0%	105 0.2%	120 9.5%	115 4.1%					119 2.2%
Flow/Width HRT Days	m2/day days	292 6.6	363 5.3	368 7.1	60 7.6	70 7.7	106 6.0	214 3.1	148 14.2					213.6 19.2
Mean Velocity Seepage Outflow / Total Outflow	cm/sec %	0.48 0%	0.59 0%	0.61 2%	0.12 2%	0.14 2%	0.21 2%	0.47 1%	0.32 3%					0.40 3%
Release 1 Outflow Volume Release 2 Outflow Volume	hm3/yr hm3/yr	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0					0.0
95th Percentile Outflow Volume 95th Percentile Outflow Load Simulated (Specified Mean Dopth	kg/d	1.44 185.17 #N/A	1.44 104.94 #N/A	1.40 49.49 #N/A	77.79	34.04	19.13	1.55 217.49	1.41 46.30					3.4 116.0
Release 1 Demand Met Release 2 Demand Met	%	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A					#N/A #N/A
Outflow Demand Met Range Check - Mean Depth	%	#N/A	#N/A	#N/A	#N/A	#N/A 0.92	#N/A 0.92	#N/A	#N/A 0.85					#N/A 3
Range Check - Freq Depth < 10 cm Range Check - Flow/Width	1	- 1.39	1	Ξ	1	0.43	0.66	1.06 1.02	- 0.92					1 5
Range Check - Inflow Conc Range Check - Outflow Conc	-	-	-	-	-	-	-	-	-					0
Water Balance Error Mass Balance Error	%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00% 0.05%	0.00% -0.45%	0.00% -0.03%					0.00%
warning or Error Messages		Cell# 1 1A Flow/ Cell# 5 2B Depth Cell# 5 2B Flow	width out of calib. a out of calib. range Width out of calib	range for EMG_3: for SAV_3: 57 v range for SAV_3:	292 vs. 26 - 210 s. 62 - 87 cm 70 vs. 162 - 374	m2/day								9
		Cell#64 Depth Cell#64 FlowA	out of calib. range	or SAV_3: 57 vs inge for SAV_3:	. 62 - 87 cm 106 vs. 162 - 374	m2/day								
		Cell# 7 5A Freq Cell# 7 5A Flow/	Z < 10 cm out of ca Width out of calib.	alib. range for EMG_3:	G_3: 10 vs. 0 - 9 214 vs. 26 - 210	% m2/day								
		Cell#85B Depth Cell#85B Flow/	out of calib. range Width out of calib.	for SAV_3: 53 v range for SAV_3:	s. 62 - 87 cm 148 vs. 162 - 374	4 m2/day								

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#### Table A.2 STA-1W: Case "2006 Base"

	Unite	Value	Project.	PROSECT_5								Cu	rrent Date:	08/03/05
Design Case Name Input Series Name	-	2006_Base TS_2006Base	STA-1W with Potential infl	tion: h Long Term P ows include all	lan Enhancerr basin runoff fr	ients om S-5A Ba	asin and EB	WCD to ST	A-1 Inflow 8	Distributio	n Works			
Starting Date for Simulation Ending Date for Simulation	-	05/01/65 04/30/00	Delivery to S All STA-1 I&	TA-1W at G-3 D inflows up to	01 based on a 2000 cfs/day	nalysis of hi delivered to	storic distrib STA-1W; a	ution bove 2000	cfs/day, G-3	02 Inflow to	STA-1W c	omputed as	2000+(Q-2	2000)**0.898
Starting Date for Output Integration Steps Per Day	-	05/01/65	Simulation Typ	pe:	Uncertainty A	nalysis								
Number of Iterations Output Averaging Interval	- days	0 7	FWM Outflow	de C (ppb)	Mean 27.5	Lower CL 34.5	Upper CL 21.8		Diagnostics H20 Balance	e Error Me	an & Max	0.0%	0.0%	
Atmospheric P Load (Dp)	- ppb mg/m2-v/r	1 10 20	Load Reduction	, (ppb) on % (%)	20.9 84% 0.0%	80%	15.2 87%		Iterations &	Converger	nce	-0.2% 3	0.5%	
Cell Number> Cell Label	-	1 1A	2 1B	3 3	4 2A	5 2B	<b>6</b> 4	7 5A	8 5B	9 9	10	11	12	
Vegetation Type Inflow Fraction	>	EMG_3 0.38	SAV_3	SAV_3	EMG_3 0.17	SAV_3	SAV_3	EMG_3 0.45	SAV_3					
Downstream Cell Number Surface Area	- km2	2 3.02	3 3.02	4.15	5 1.91	6 1.91	1.45	8 2.27	9.28					
Mean Width of Flow Path Number of Tanks in Series	km -	1.10 2.0	1.10 2.0	1.10 2.0	2.40 2.0	2.00 2.0	1.30 2.0	1.78 2.0	2.34 3.0					
Minimum Depth for Releases Release 1 Series Name	cm													
Outflow Series Name														
Outflow Control Depth Outflow Weir Depth	cm	55	55	46	60	60	60	60	60					
Outflow Coefficient - Exponent Outflow Coefficient - Intercept	-	4 1	4 1	4 1	4 1	4	4 1	4	4					
Bypass Depth Maximum Inflow	cm hm3/day													
Maximum Outflow Inflow Seepage Rate	hm3/day (cm/d) / cm	0.0035	0.0018	0.0023										
Inflow Seepage Control Elev Inflow Seepage Conc	cm ppb	172 20	172 20	185 20	0.0016	0.0016	0.0024	0.0156	0.0040					
Outflow Seepage Control Elev Max Outflow Seepage Conc	cm			-60	-46	-46	-46 20	-46	-46 20					
Seepage Recycle to Cell Number Seepage Recycle Fraction	-			20	20	20	20	1 0.91	1 0.8					
Seepage Discharge Fraction Initial Water Column Conc	- ppb	30	30	30	30	30	30	30	30					
Initial P Storage Per Unit Area Initial Water Column Depth	mg/m2 cm	500 200	500 200	500 200	500 200	500 200	500 200	500 200	500 200					
C0 = Conc at 0 g/m2 P Storage C1 = Conc at 1 g/m2 P storage	ppb ppb	3 22	3 22	3 22	3 22	3 22	3 22	3 22	3 22					
C2 = Conc at Half-Max Uptake K = Net Settling Rate at Steady State	ppb m/yr	300 16.8	300 52.5	300 52.5	300 16.8	300 52.5	300 52.5	300 16.8	300 52.5					
Z1 = Saturated Optake Depth Z2 = Lower Penalty Depth Z2 = Upper Repaity Depth	cm	40 100 200	40 100 200	40 100 200	40 100 200	40 100 200	40 100 200	40 100 200	40 100 200					
Output Variables	Units	1	200	3	4	5	6	7	8	9	10	11	12	Overall
Execution Time Run Date	sec/yr	7.57 08/03/05	7.97 08/03/05	8.37 08/03/05	8.77 08/03/05	9.17 08/03/05	9.57 08/03/05	9.97 08/03/05	10.49 08/03/05					10.49 08/03/05
Starting Date for Simulation Starting Date for Output	-	05/01/65 05/01/65	05/01/65 05/01/65	05/01/65 05/01/65	05/01/65 05/01/65	05/01/65 05/01/65	05/01/65 05/01/65	05/01/65 05/01/65	05/01/65 05/01/65					05/01/65 05/01/65
Ending Date Output Duration	- days	04/30/00 12784	04/30/00 12784	04/30/00 12784	04/30/00 12784	04/30/00 12784	04/30/00 12784	04/30/00 12784	04/30/00 12784					04/30/00 12784
Cell Label Downstream Cell Label		1A 1B	1B 3	3 Outflow	2A 2B	2B 4	4 Outflow	5A 5B	5B Outflow					Total -
Network Simulation Name Simulation Type	- -	Uncerta	Uncerta	Uncerta	Uncerta	Uncerta	Uncerta	none Uncerta	Uncerta					Uncerta
Mean Rainfall Mean ET	cm/yr cm/yr	134.9 129.8	134.9 129.8	134.9	134.9 129.8	134.9 129.8	134.9 129.8	134.9 123.7	134.9 128.4					134.9 128.8
Cell Inflow Volume Cell Inflow Load	hm3/yr kg/yr	112.9 19656	141.3 16502	143.5 8891	50.5 8793	49.4 6092	48.4 2345	133.7 23276	121.5 16673					297.0 51725
Cell Inflow Conc Treated Outflow Volume	ppb hm3/yr	174.2 141.3	116.8 143.5	62.0 144.9	174.2 49.4	123.2 48.4	48.5 47.3	174.2 121.5	137.2 105.8					174.2 298.0
Treated Outflow Load Treated FWM Outflow Conc	kg/yr ppb	16502 116.8	8891 62.0	4095 28.3	6092 123.2	2345 48.5	1225 25.9	16673 137.2	2864 27.1					8185 27.5
Upper Confidence Limit Lower Confidence Limit	ppb ppb	122.4 110.0	71.9 52.1	36.0 21.9	132.9 112.2	59.2 39.1	32.9 20.6	143.0 130.6	33.1 22.3					34.5 21.8
Total Outflow Load + Bypass Total EWM Outflow Conc	kg/yr	16502	8891	4095	6092 123.2	2345	1225	16673	2864					8184.7 27.5
Bypass Load Bypass Load	kg/yr %	0	0	0	0	0	0	0	0					0.0
Maximum Inflow Maximum Outflow	hm3/d hm3/d	2.55 2.67	2.67 2.75	2.75 2.83	1.14 1.15	1.15 1.18	1.18 1.20	3.02 2.94	2.94 3.05					6.70 7.08
Surface Load Reduction Load Trapped in Sediments	kg/yr kg/yr	3153 3800	7612 7751	4796 4935	2701 2622	3747 3753	1120 1144	6604 2877	13809 12990					43541 39871
Overall Load Reduction Lower Confidence Limit	%	16% 12%	46% 40%	54% 49%	31% 25%	62% 56%	48%	28%	83% 80%					84% 80%
Daily Geometric Mean Outflow Geo Mean - Composites	ppb	102.8	52% 51.7 52.5	21.4	108.6 115.6	34.1 40.8	48% 14.9 19.5	150.6 127.8	18.3 20.8					#N/A 20.9
Upper Confidence Limit	ppb ppb	109.9	62.28 42.87	29.3 15.8	125.7	51.3 31.8	26.2 14.5	134.0 120.8	26.6 16.3					27.9
Frequency Outflow Conc > 10 ppb Frequency Outflow Conc > 20 ppb	%	100% 100%	100% 100%	100% 61%	100% 100%	100% 99%	97% 48%	100% 100%	99% 54%					100% 87%
Frequency Outflow Conc > 50 ppb Freq Outflow Volume > 10 ppb	% %	100% 100%	61% 100%	0% 100%	100% 100%	23% 100%	0% 99%	100% 100%	0% 100%					55% 100%
95th Percentile Outflow Conc Mean Biomass P Storage	mg/m2	130 3959	75 2577	36 1191	137 4319	61 1972	33 791	163 3973	35 1402					34 2216
Net Storage Turnover Rate	1/yr	11.1 1260	34.9 2574	34.9 1189	0% 11.1 1376	34.9 1960	0% 34.9 780	0% 11.1 1265	34.9 1400					23.3 1477
Mean Water Load Max Water Load	cm/d cm/d	10.2	12.8	9.5	7.3	7.1	9.1 81.7	16.1 132.6	3.6					3.0
Mean Depth Minimum Depth	cm cm	70 55	71 54	70 43	57 13	57 13	57 7	52 1	53 1					60 21
Maximum Depth Frequency Depth < 10 cm	cm %	124 0.0%	125 0.0%	126 0.0%	83 0.0%	87 0.0%	98 0.2%	113 9.5%	106 4.1%					111 2.2%
Flow/Width HRT Days	m2/day days	281 6.8	352 5.5	357 7.3	58 7.9	68 8.0	102 6.3	206 3.2	142 14.7					206.1 19.9
Mean Velocity Seepage Outflow / Total Outflow	cm/sec %	0.47	0.58	0.59 2%	0.12	0.14 2%	0.21 2%	0.46	0.31 3%					0.38 3%
Release 1 Outflow Volume Release 2 Outflow Volume	hm3/yr hm3/yr	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0					0.0
95th Percentile Outflow Volume 95th Percentile Outflow Load Simulated / Specified Maan Dooth	kg/d	1.41 176.18 #N/A	1.37 94.47 #N/A	1.34 43.57 #N/A	0.59 75.12	31.16	0.58 16.70	208.36	1.34 39.31 #N/A					3.2 99.7
Release 1 Demand Met Release 2 Demand Met	%	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A					#N/A #N/A
Outflow Demand Met Range Check - Mean Depth	%	#N/A	#N/A	#N/A	#N/A	#N/A 0.92	#N/A 0.92	#N/A	#N/A 0.85					#N/A 3
Range Check - Freq Depth < 10 cm Range Check - Flow/Width	1	1.34	1	1	1	0.42	0.63	1.06	- 0.88					1 4
Range Check - Inflow Conc Range Check - Outflow Conc	-	-	-	-	-		-	-	-					0
Water Balance Error Mass Balance Error Wareing or Error	%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00% -0.47%	0.00%					-0.20%
warning or error messages		Cell# 5 2B Depth Cell# 5 2B Element	wath out of calib. out of calib. range Width out of calib	range for EMG_3: a for SAV_3: 57 v range for SAV_3:	281 vs. 26 - 210 s. 62 - 87 cm 68 vs. 162 - 274	m2/day								8
		Cell# 6 4 Depth o Cell# 6 4 Flow/W	out of calib. range t idth out of calib. ra	for SAV_3: 57 vs ange for SAV_3:	. 62 - 87 cm 102 vs. 162 - 374	m2/day								
		Cell# 7 5A Freq 2 Cell# 8 5B Depth	Z < 10 cm out of ca out of calib. range	alib. range for EM0 for SAV_3: 53 v	3_3: 10 vs. 0 - 9 s. 62 - 87 cm	%								
		Cell# 8 5B Flow/	Width out of calib.	range for SAV_3:	142 vs. 162 - 37	4 m2/day								

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#### Table A.3 STA-1W: Case "2006 Mod"

DMSTA2- Inputs & Outpu	uts		Project:	PROJECT_S	TA1W							Mode Cu	I Release: rrent Date:	7/1/2005 08/06/05
Input Variable Design Case Name Input Series Name	Units -	Value 2006_Mod	Case Descrip STA-1W with	tion: h Long Term P	lan Enhancem	ients	from ECD 2	006 Cimula	tion to STA	4.18 D. Ward				
Input Series Name Starting Date for Simulation	-	05/01/65	Distribution (	of outflows from	n STA-1 I&D a	ssigned 70%	% to STA-1	006 Simula V, 30% to v	tion to STA vesterly flow	path of ST	ks A-1E			
Ending Date for Simulation Starting Date for Output	-	04/30/00 05/01/65	Circulation To		Lineasteint A	e e lucio								
Number of Iterations	- -	4 0 7	Output Variab	be. ble (C (ppb)	Mean 20.3	Lower CL 25.2	Upper CL		Diagnostic:	s e Error Me	an & Max	0.0%	0.0%	
Inflow Conc Scale Factor	- -	1	GM Outflow C	C (ppb) C (ppb)	13.5	18.5	9.8		Mass Balan	nce Error M	ean & Max	-0.2%	0.5%	
Atmospheric P Load (Dry)	mg/m2-yr	20	Bypass Load	(%)	0.0%	5	6	7	Warning/E	rror Messag	jes 10	7	12	
Cell Label Vegetation Type	>	1A EMG_3	1B SAV_3	3 SAV_3	2A EMG_3	2B SAV_3	4 SAV_3	5A EMG_3	5B SAV_3					
Inflow Fraction Downstream Cell Number	1	0.263 2	3		0.121 5	6		0.316 8						
Surface Area Mean Width of Flow Path	km2 km	3.02 1.10	3.02 1.10	4.15 1.10	1.91 2.40	1.91 2.00	1.45 1.30	2.27 1.78	9.28 2.34					
Number of Tanks in Series Minimum Depth for Releases	- cm	2.0	2.0	2.0	2.0	2.0	2.0	2.0	3.0					
Release 1 Series Name Release 2 Series Name														
Outflow Series Name Depth Series Name				40										
Outflow Control Depth Outflow Weir Depth	cm	55	55	46	60	60	60	60	60					
Outflow Coefficient - Exponent Outflow Coefficient - Intercept Bypass Depth	-	4	1	4	1	1	1	4	4					
Maximum Inflow Maximum Outflow	hm3/day													
Inflow Seepage Rate	(cm/d) / cm cm	0.0035 172	0.0018	0.0023										
Inflow Seepage Conc Outflow Seepage Rate	ppb (cm/d) / cm	20	20	20 0.0014	0.0016	0.0016	0.0021	0.0156	0.0049					
Outflow Seepage Control Elev Max Outflow Seepage Conc	cm ppb			-60 20	-46 20	-46 20	-46 20	-46 20	-46 20					
Seepage Recycle to Cell Number Seepage Recycle Fraction	1							1 0.91	1 0.8					
Seepage Discharge Fraction Initial Water Column Conc	- ppb	30	30	30	30	30	30	30	30					
Initial P Storage Per Unit Area Initial Water Column Depth	mg/m2 cm	500 200	500 200	500 200	500 200	500 200	500 200	500 200	500 200					
C0 = Conc at 0 g/m2 P Storage C1 = Conc at 1 g/m2 P storage	ppb	3 22	3 22	3 22	3 22	3 22	3 22	3 22	3 22					
K = Net Settling Rate at Steady State	ppp m/yr	16.8	52.5	52.5	16.8	52.5	52.5	16.8	52.5					
Z2 = Lower Penalty Depth Z3 = Loper Penalty Depth	cm	100	100	100	100	100	100	100	100					
Output Variables	Units	1	2	3	4	5	6	7	8	9	10	11	12	Overall
Execution Time Run Date	sec/yr	7.31 08/06/05	7.69 08/06/05	8.06 08/06/05	8.43 08/06/05	8.80 08/06/05	9.14 08/06/05	9.51 08/06/05	9.97 08/06/05					9.97 08/06/05
Starting Date for Simulation Starting Date for Output	1	05/01/65 05/01/65	05/01/65 05/01/65	05/01/65 05/01/65	05/01/65 05/01/65	05/01/65 05/01/65	05/01/65 05/01/65	05/01/65 05/01/65	05/01/65 05/01/65					05/01/65 05/01/65
Ending Date Output Duration	- days	04/30/00 12784	04/30/00 12784	04/30/00 12784	04/30/00 12784	04/30/00 12784	04/30/00 12784	04/30/00 12784	04/30/00 12784					04/30/00 12784
Cell Label Downstream Cell Label		1A 1B	1B 3	3 Outflow	2A 2B	2B 4	4 Outflow	5A 5B	5B Outflow					Total -
Simulation Type	-	Uncerta	Uncerta	Uncerta	Uncerta	Uncerta	Uncerta	Uncerta	Uncerta					Uncerta
Mean Rainfall Mean FT	cm/yr	134.9 129.8	134.9 129.8	4.15 134.9 129.8	134.9	134.9	134.9 129.3	134.9	9.28 134.9 126.6					134.9
Cell Inflow Volume Cell Inflow Load	hm3/yr kg/yr	81.2 14148	108.6	110.8 5216	37.3 6509	36.3 4011	35.3 1307	97.5 17000	85.8 10864					216.0 37657
Cell Inflow Conc Treated Outflow Volume	ppb hm3/yr	174.3 108.6	103.7 110.8	47.1 112.5	174.3 36.3	110.5 35.3	37.1 34.2	174.3 85.8	126.5 71.2					174.3 217.9
Treated Outflow Load Treated FWM Outflow Conc	kg/yr ppb	11256 103.7	5216 47.1	2204 19.6	4011 110.5	1307 37.1	692 20.2	10864 126.5	1537 21.6					4433 20.3
Upper Confidence Limit Lower Confidence Limit	ppb ppb	110.2 96.0	56.2 38.5	25.1 15.4	121.7 98.1	45.7 30.0	24.8 16.9	133.5 118.8	25.5 18.6					25.2 16.7
Total Outflow Volume + Bypass Total Outflow Load + Bypass	hm3/yr kg/yr	108.6 11256	110.8 5216	112.5 2204	36.3 4011	35.3 1307	34.2 692	85.8 10864	71.2					217.9 4433.3
Bypass Load	kg/yr	0	47.1	19.6	0	37.1 0	0	126.5	0					20.3
Maximum Inflow Maximum Qutflow	hm3/d hm3/d	2.43	2.57	2.71	1.12	1.12	1.18	2.92	2.83					6.46 7.13
Surface Load Reduction Load Trapped in Sediments	kg/yr kg/yr	2893 3493	6040 6181	3012 3181	2498 2434	2704 2726	615 647	6136 2664	9327 8830					33224 30157
Overall Load Reduction Lower Confidence Limit	%	20% 15%	54% 48%	58% 55%	38% 32%	67% 64%	47% 47%	36% 33%	86% 84%					88% 85%
Upper Confidence Limit Daily Geometric Mean	% ppb	26% 90.4	59% 38.1	59% 13.7	45% 94.9	70% 22.3	45% 9.2	40% 137.0	87% 11.6					90% #N/A
Outflow Geo Mean - Composites Upper Confidence Limit	ppb ppb	91.9 98.1	38.7 47.62	14.0 19.3	101.8 113.5	28.7 37.1	13.1 17.4	115.8 123.2	14.5 18.2					13.5 18.5
Frequency Outflow Conc > 10 ppb	ppb %	84.5 100%	30.38	10.1 84%	89.1 100%	22.0 100%	10.2 72%	107.7	11.7 84%					9.8 82%
Frequency Outflow Conc > 20 ppb Frequency Outflow Conc > 50 ppb	%	100%	15%	0%	100%	3%	0%	100%	0%					11%
95th Percentile Outflow Conc Mean Biomass P Storage	ppb mg/m2	116	58	25	123	46	27	155	29 953					24
Storage Increase / Net Removal Net Storage Turnover Rate	% 1/yr	0% 11.1	0% 34.9	0% 34.9	0% 11.1	0% 34.9	0% 34.9	0% 11.1	0% 34.9					0% 21.7
Unit Area P Removal Mean Water Load	g/m2-yr cm/d	1159 7.4	2050 9.9	766 7.3	1277 5.4	1430 5.2	446 6.7	1171 11.7	952 2.5					1117 2.2
Max Water Load Mean Depth	cm/d cm	80.5 67	85.2 68	65.2 66	58.6 56	58.9 55	81.6 54	128.2 49	30.5 48					23.9 57
Minimum Depth Maximum Depth	cm	54 122	50 124	39 126	13 81	11 86	1 97	1 110	1 106					20 109
Frequency Depth < 10 cm Flow/Width	m2/day	202	270	276	43	50	1.7%	150	100					3.4% 152.8
Mean Velocity	cm/sec	0.35	0.46	0.48	0.09	0.10	0.16	4.2 0.35	0.24					0.30
Release 1 Outflow Volume	hm3/yr	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0					0.0
95th Percentile Outflow Load	hm3/d	1.03	1.01	1.00	0.44	0.44	0.43	1.08	0.95					2.3 54.8
Simulated / Specified Mean Depth Release 1 Demand Met	%	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A					#N/A #N/A
Release 2 Demand Met Outflow Demand Met	% %	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A					#N/A #N/A
Range Check - Mean Depth Range Check - Freq Depth < 10 cm	1	1	-	-	1	0.89	0.87	- 1.18	0.78					3 1
Range Check - Flow/Width Range Check - Inflow Conc	1	1	1	1	1	0.31	0.46	1	0.62					3 0
Range Check - Outflow Conc Water Balance Error	%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%					0.00%
Mass Balance Error Warning or Error Messages	%	0.01% Cell# 5 2B Depth	0.01% out of calib. range	0.01% e for SAV_3: 55 v	0.03% /s. 62 - 87 cm	0.03%	0.05%	-0.51%	-0.01%		-			-0.21% 7
		Cell# 5 2B Flow/ Cell# 6 4 Depth of Cell# 6 4 Flower	width out of calib. out of calib. range t lidth out of calib	for SAV_3: 54 vs	50 vs. 162 - 374 . 62 - 87 cm 74 vs. 162 - 374	m2/day								
		Cell# 7 5A Freq 2 Cell# 8 5B Denth	Z < 10 cm out of ca	alib. range for EM for SAV 3: 48	G_3: 11 vs. 0 - 9 /s. 62 - 87 cm	%								
		Cell# 8 5B Flow/	Width out of calib.	range for SAV_3:	100 vs. 162 - 37	4 m2/day								

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### Table A.4 STA-1E: Case "2006 All"

			Project.	FROSECT_5								Cu	rrent Date:	08/03/05
Input Variable Design Case Name Input Series Name	- -	2006 All TS 2006 All	STA-1E with	tion: East and West de all C-51 West	at Distribution (	Cells	B rupoff plu	s I -8 Basin	rupoff disc	harged through	udb S-54E			1
Starting Date for Simulation	-	05/01/65	Cell-to-cell s	eepage not co	nsidered in ana Cells each mo	alysis ideled as tw	o cells in na	arallel	Turion disc	larged thro	Ign 3-3AE			
Starting Date for Output Integration Steps Per Day	1	05/01/65	Simulation Typ	pe:	Uncertainty A	nalysis	o cons in pe							
Number of Iterations Output Averaging Interval	- days	0 7	Output Variat FWM Outflow	vie v C (ppb)	Mean 22.1	Lower CL 27.9	Upper CL 17.3		Diagnostic H20 Balan	<u>s</u> ce Error Me	an & Max	0.0%	0.0%	
Inflow Conc Scale Factor Rainfall P Conc	ppb	1 10	GM Outflow C Load Reduction	C (ppb) on %	14.5 81%	19.8 76%	10.5 85%		Mass Bala Iterations 8	nce Error M Converger	ean & Max 1ce	0.0%	0.0% 0.1%	
Atmospheric P Load (Dry) Cell Number>	mg/m2-yr	20 1	Bypass Load	(%) 3	0.0% 4	5	6	7	Warning/E 8	rror Messag 9 7	10	6 11	12	
Vegetation Type	>	EMG_3	EMG_3	SAV_3	EMG_3	EMG_3	SAV_3	45 SAV_3	EMG_3	EMG_3	EMG_3	EMG_3	SAV_3	
Downstream Cell Number Surface Area	- km2	2 0.95	3 2.25	2.23	5 0.95	6 2.38	7 2.61	3.04	9	12 1.69	11.00	12.00 2.31	4.25	
Mean Width of Flow Path Number of Tanks in Series	km -	0.66 0.5	1.55 3.0	1.55 3.0	0.66 0.5	1.55 3.0	1.55 3.0	1.55 3.0	0.75 0.5	1.18 3.0	0.75 0.5	1.61 3.0	1.61 3.0	
Minimum Depth for Releases Release 1 Series Name	cm													
Release 2 Series Name Outflow Series Name Depth Series Name														
Outflow Control Depth Outflow Weir Depth	cm cm	40	40	60	90	40	60	60	100	40	40	40	60	
Outflow Coefficient - Exponent Outflow Coefficient - Intercept	-	4 1	4 1	4 1	4 1	4	4 1	4	4	4 1	4 1	4 1	4 1	
Bypass Depth Maximum Inflow	cm hm3/day													
Maximum Outflow Inflow Seepage Rate	hm3/day (cm/d) / cm									0.0054			0.0057	
Inflow Seepage Control Elev Inflow Seepage Conc Outflow Seepage Rate	ppb (cm/d) / cm	0.0095	0.0042	0.0042	0.0095			0.0054	0.01	20	0.01		94 20	
Outflow Seepage Control Elev Max Outflow Seepage Conc	cm	-137	-137	-99 20	-87 20			-38 20	-15		-76			
Seepage Recycle to Cell Number Seepage Recycle Fraction		1	1	1	4 1			7	8		10			
Seepage Discharge Fraction Initial Water Column Conc	- ppb	30	30	30	30	30	30	30	30	30	30	30	30	
Initial P Storage Per Unit Area Initial Water Column Depth	mg/m2 cm	500 50	500 50	500 50	500 50	500 50	500 50	500 50	500 50	500 50	500 50	500 50	500 50	
C1 = Conc at 1 g/m2 P storage C2 = Conc at 1 g/m2 P storage C2 = Conc at Half-Max Uptake	ppb	22 300	3 22 300	22 300	22 300	22 300	22 300	22 300	22 300	22 300	3 22 300	3 22 300	22 300	
K = Net Settling Rate at Steady State Z1 = Saturated Uptake Depth	m/yr cm	16.8 40	16.8 40	52.5 40	16.8 40	16.8 40	52.5 40	52.5 40	16.8 40	16.8 40	16.8 40	16.8 40	52.5 40	
Z2 = Lower Penalty Depth Z3 = Upper Penalty Depth	cm cm	100 200	100 200	100 200	100 200	100 200	100 200	100 200	100 200	100 200	100 200	100 200	100 200	
Output Variables	Units	1	2	3	4	5	6	7	8	9	10	11	12	Overall
Execution Time Run Date Starting Date for Simulation	sec/yr	08/03/05	12.11 08/03/05 05/01/65	12.63 08/03/05	12.94 08/03/05 05/01/65	13.43 08/03/05 05/01/65	13.94 08/03/05 05/01/65	14.49 08/03/05	14.80 08/03/05	15.31 08/03/05	15.63 08/03/05	08/03/05	16.63 08/03/05	16.63 08/03/05
Starting Date for Simulation Starting Date for Output	-	05/01/65	05/01/65	05/01/65	05/01/65	05/01/65	05/01/65	05/01/65	05/01/65	05/01/65	05/01/65	05/01/65	05/01/65	05/01/65
Output Duration Cell Label	days	12784 EDCE	12784	12784	12784 EDCW	12784	12784 4N	12784 4S	12784 WDCW	12784	12784 WDCE	12784	12784	12784 Total
Downstream Cell Label Network Simulation Name	-	1 none	2 none	Outflow none	3 none	4N none	4S none	Outflow none	7 none	6 none	5 none	6 none	Outflow none	- none
Simulation Type Surface Area	km2	Uncerta 0.95	Uncerta 2.25	Uncerta 2.23	Uncerta 0.95	Uncerta 2.38	Uncerta 2.61	Uncerta 3.04	Uncerta 1.17	Uncerta 1.69	Uncerta 1.17	Uncerta 2.31	Uncerta 4.25	Uncerta 25.00
Mean Rainfall Mean ET Coll Inflow Volumo	cm/yr cm/yr	142.9 129.7	142.9 129.7	142.9 129.7	142.9 129.7	142.9 129.7	142.9 129.7	142.9 129.7	142.9 129.7	142.9 129.7	142.9 129.7 71.9	142.9 129.7	142.9 129.7	142.9 129.7
Cell Inflow Load Cell Inflow Load	kg/yr	6694 116.5	5690 81.7	3633 57.3	13054	11513 102.6	9159 81.4	4567	5356 116.5	3751 90.9	8368 116.5	6474 97.9	6995 64.4	33472 116.5
Treated Outflow Volume Treated Outflow Load	hm3/yr kg/yr	69.6 5690	63.4 3633	58.1 1284	112.2 11513	112.5 9159	112.8 4567	113.2 2301	41.3 3751	42.1 2491	66.1 6474	66.4 4504	111.5 2655	282.9 6240
Treated FWM Outflow Conc Upper Confidence Limit	ppb ppb	81.7 84.2	57.3 64.0	22.1 27.6	102.6 104.6	81.4 87.4	40.5 48.2	20.3 25.8	90.9 95.8	59.2 68.1	97.9 101.5	67.8 76.1	23.8 30.1	22.1 27.9
Lower Confidence Limit Total Outflow Volume + Bypass	ppb hm3/yr	78.7 69.6	50.1 63.4	17.4 58.1	100.2 112.2	74.5	33.1 112.8	15.9 113.2	85.3 41.3	49.9 42.1	93.7 66.1	58.8 66.4	18.6 111.5	17.3 282.9
Total Outflow Load + Bypass Total FWM Outflow Conc Bypass Load	ppb kg/yr	81.7 0	57.3 0	1284 22.1	11513 102.6	81.4	40.5	2301 20.3	90.9	59.2 0	97.9	4504 67.8	2655 23.8	22.1 0.0
Bypass Load Maximum Inflow	% hm3/d	0.0 1.30	0.0 1.34	0.0 1.33	0.0 2.53	0.0	0.0 2.54	0.0 2.54	0.0 1.04	0.0 1.03	0.0	0.0 1.61	0.0 2.66	0.0
Maximum Outflow Surface Load Reduction	hm3/d kg/yr	1.34 1005	1.33 2057	1.30 2349	2.53 1541	2.54 2354	2.54 4592	2.50 2266	1.03 1604	1.05 1260	1.61 1894	1.61 1970	2.63 4340	6.44 27233
Load Trapped in Sediments Overall Load Reduction	kg/yr %	895 15%	1742 36%	2287 65%	1110 12%	2435 20%	4681 50%	2346 50%	1229 30%	1330 34%	1321 23%	2049 30%	4534 62%	25960 81%
Lower Confidence Limit Upper Confidence Limit	%	12% 18%	31% 42%	60% 68%	10% 14%	16% 25%	45% 55%	46% 52%	26% 34%	27% 40%	20% 26%	25% 37%	58% 65%	76% 85%
Outflow Geo Mean - Composites	ppb	73.5	51.0 58.00	15.4	96.9 99.3	74.1 74.3 81.0	30.3 30.7 38.5	13.2	86.0 91.6	50.7 50.8 60.3	93.1 97.5	59.6 68.7	15.8	14.5 19.8
Lower Confidence Limit Frequency Outflow Conc > 10 ppb	ppb %	70.4 100%	43.45 100%	11.3 81%	93.8 100%	66.5 100%	23.5 100%	9.5 68%	79.6 100%	41.2 100%	88.0 100%	50.0 100%	11.4 79%	10.5 75%
Frequency Outflow Conc > 20 ppb Frequency Outflow Conc > 50 ppb	%	100% 100%	100% 55%	30% 0%	100% 100%	100% 100%	85% 9%	23% 0%	100% 100%	100% 56%	100% 100%	100% 82%	35% 0%	52% 28%
Freq Outflow Volume > 10 ppb 95th Percentile Outflow Conc	% ppb	100% 96	100% 66	94% 30	100% 120	100% 90	100% 53	87% 28	100% 101	100% 69	100% 110	100% 77	94% 33	92% 30
Mean Biomass P Storage Storage Increase / Net Removal	mg/m2 %	2958 0%	2431 0%	1027 0%	3702 0%	3213 0%	1797 0% 24.0	773 0%	3330	2472 0%	3549 0%	2785 0%	1069 0%	2079 0%
Unit Area P Removal Mean Water Load	g/m2-yr cm/d	941	774	1025	1167 32.3	1023 12.9	1793 11.8	772	1051	787 6.7	1129 16.8	887	1067 7.0	1038
Max Water Load Mean Depth	cm/d cm	136.2 68	59.7 54	59.6 62	265.7 95	106.4 60	97.5 67	83.4 67	88.6 99	61.2 51	138.4 61	69.5 52	62.7 67	25.9 65
Minimum Depth Maximum Depth	cm cm	41 119	39 96	13 96	90 140	35 113	42 113	36 113	42 108	26 97	1 121	12 100	50 113	35 109
Frequency Depth < 10 cm Flow/Width	% m2/day	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0% 96	0.6%	0.0%	0.0%	0.0%
HRT Days Mean Velocity Scoppage Outflow (Total Outflow	cm/sec	4.1 0.40	0.27	0.21	0.57	4.7 0.38	0.34	0.35	9.2 0.20	0.22	0.50	0.25	9.5 0.32	0.32
Release 1 Outflow Volume Release 2 Outflow Volume	hm3/yr hm3/yr	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
95th Percentile Outflow Volume 95th Percentile Outflow Load	hm3/d kg/d	0.59 51.81	0.57 35.25	0.55 14.87	1.07 110.31	1.07 92.72	1.07 51.78	1.06 27.21	0.42 39.80	0.43 27.87	0.66 65.91	0.66 48.27	1.08 31.91	2.7 74.4
Simulated / Specified Mean Depth Release 1 Demand Met	% %	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A
Release 2 Demand Met Outflow Demand Met	%	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A
Range Check - Mean Depth Range Check - Freq Depth < 10 cm Range Check - Flow/Width	-	1 14	-		1.24 - 2.21	÷	-	-	1.30	-		-	-	0
Range Check - Inflow Conc Range Check - Outflow Conc	-		-	-			-	-		-	-	-	-	0
Water Balance Error Mass Balance Error	% %	0.00% 0.00%	0.00% 0.00%	0.00% 0.01%	0.00% 0.00%	0.00% 0.01%	0.00% 0.01%	0.00% 0.01%	0.00% 0.00%	0.00% 0.01%	0.00% 0.00%	0.00% 0.01%	0.00% 0.01%	0.01% 0.02%
Warning or Error Messages		Cell# 1 EDCE Flo Cell# 3 2 Flow/W	w/Width out of ca idth out of calib. ra	lib. range for EMG ange for SAV_3:	_3: 238 vs. 26 - 112 vs. 162 - 374	210 m2/day m2/day								6
		Cell# 4 EDCW D Cell# 4 EDCW F	epth out of calib. r low/Width out of ca	ange for EMG_3: alib. range for EMC	95 vs. 35 - 76 cm 3_3: 465 vs. 26 - 99 vs. 25 - 76	210 m2/day								
		Cell# 10 WDCE	Flow/Width out of	calib. range for EN	IG_3: 262 vs. 26	- 210 m2/day								





Table A.5	STA-1E:	Cells 1-4:	Case "1	4 2006Base"
1 abic 11.5		CCII5 I 4.	Cube I	- a accordance

DMSTA2- Inputs & Outpu	uts		Project:	PROJECT_S1	TA1E							Mode Cu	rrent Date:	7/1/2005 08/03/05
Input Variable Design Case Name	Units -	Value	Case Descrip	tion: f STA-1E with	East Distributio	on Cell								
Input Series Name Starting Date for Simulation		TS_1_42006AI 05/01/65	Inflows from Cell-to-cell s	C-51 West ad	justed for sam	e-day bypas alysis; inflow	s to S-155A	of L-8 run by 5% (ap	off through s prox. 6,400	S-5AE,; 20% ac-ft/yr) for	% assigned seepage re	to Cells 5-7 cycle from v	est flow pa	ath
Ending Date for Simulation Starting Date for Output	1	04/30/00 05/01/65	East Distribu	ition Cell mode	led as two cell	s in parallel						-		
Integration Steps Per Day Number of Iterations	1	4 0	Simulation Ty Output Variat	be: ble	Uncertainty Ar Mean	halysis Lower CL	Upper CL		Diagnostic	5				-
Output Averaging Interval Inflow Conc Scale Factor	days -	7 1	FWM Outflow GM Outflow 0	C (ppb) (ppb)	18.3 14.9	24.1 20.5	13.8 10.6		H20 Balan Mass Bala	ce Error Me nce Error M	an & Max lean & Max	0.0% 0.0%	0.0% 0.0%	
Rainfall P Conc Atmospheric P Load (Dry)	ppb mg/m2-yr	10 20	Load Reducti Bypass Load	on % (%)	85% 0.0%	80%	88%		Iterations & Warning/E	k Converge	nce Jes	3 5	0.1%	
Cell Number> Cell Label	-	1 EDCE	2	<b>3</b> 2	4 EDCW	5 3	6 4N	7 4S	8	9	10	11	12	1
Vegetation Type Inflow Fraction	>	EMG_3 0.269	EMG_3	SAV_3	EMG_3 0.571	EMG_3	SAV_3	SAV_3						
Downstream Cell Number Surface Area	km2	2 0.95	3 2.25	2.23	5 0.95	6 2.38	7 2.61	3.04						
Mean Width of Flow Path Number of Tanks in Series	km -	0.66 0.5	1.55 3.0	1.55 3.0	0.66 0.5	1.55 3.0	1.55 3.0	1.55 3.0						
Minimum Depth for Releases Release 1 Series Name	cm													
Outflow Series Name														
Outflow Control Depth	cm	40	40	60	90	40	60	60						
Outflow Coefficient - Exponent	-	4	4	4	4	4	4	4						
Bypass Depth Maximum Inflow	cm hm3/day	-												
Maximum Outflow Inflow Seepage Rate	hm3/day (cm/d) / cm													
Inflow Seepage Control Elev Inflow Seepage Conc	cm ppb													
Outflow Seepage Rate Outflow Seepage Control Elev	(cm/d) / cm cm	0.0095 -137	0.0042 -137	0.0042 -99	0.0095 -87			0.0054 -38						
Max Outflow Seepage Conc Seepage Recycle to Cell Number	ppb -	20 1	20 1	20 1	20 4			20 7						
Seepage Recycle Fraction Seepage Discharge Fraction	1	1	1	1	1			1						
Initial Water Column Conc Initial P Storage Per Unit Area	ppb mg/m2	30 500	30 500	30 500	30 500	30 500	30 500	30 500						
C0 = Conc at 0 g/m2 P Storage	cm ppb	50 3	50 3	50 3	50 3	50 3	50 3	50 3						
C1 = Conc at 1 g/m2 P storage C2 = Conc at Half-Max Uptake	ppb	22 300	22 300	22 300	22 300	22 300	22 300	22 300						
K = Net Settling Rate at Steady State Z1 = Saturated Uptake Depth	m/yr cm	16.8 40	16.8 40	52.5 40	16.8 40	16.8 40	52.5 40	52.5 40						
Z2 = Lower Penalty Depth Z3 = Upper Penalty Depth	cm	200	200	200	200	200	200	200						
Output Variables	Units	1	<b>2</b>	<b>3</b> 8 37	<b>4</b>	5	<b>6</b>	7 10.20	8	9	10	11	12	Overall
Run Date Starting Date for Simulation	-	08/03/05	08/03/05	08/03/05	08/03/05	08/03/05	08/03/05	08/03/05						08/03/05
Starting Date for Output		05/01/65	05/01/65	05/01/65	05/01/65	05/01/65	05/01/65	05/01/65						05/01/65
Output Duration	days	12784 EDCE	12784	12784	12784 EDCW	12784	12784 4N	12784 4S						12784 Total
Downstream Cell Label Network Simulation Name	-	1 none	2 none	Outflow	3 none	4N none	4S none	Outflow						- none
Simulation Type Surface Area	- km2	Uncerta 0.95	Uncerta 2.25	Uncerta 2.23	Uncerta 0.95	Uncerta 2.38	Uncerta 2.61	Uncerta 3.04						Uncerta 14.41
Mean Rainfall Mean ET	cm/yr cm/yr	142.9 129.7	142.9 129.7	142.9 129.7	142.9 129.7	142.9 129.7	142.9 129.7	142.9 129.7						142.9 129.7
Cell Inflow Volume Cell Inflow Load	hm3/yr kg/yr	53.5 6472	65.7 5426	59.4 3316	113.6 13738	113.8 12138	114.1 9654	114.4 4573						167.2 20210
Cell Inflow Conc Treated Outflow Volume	ppb hm3/yr	120.9 65.7	82.6 59.4	55.8 54.2	120.9 113.8	106.7 114.1	84.6 114.4	40.0 114.8						120.9 169.0
Treated Outflow Load Treated FWM Outflow Conc	kg/yr ppb	5426 82.6	3316 55.8	1002 18.5	12138 106.7	9654 84.6	4573 40.0	2100 18.3						3102 18.3
Upper Confidence Limit Lower Confidence Limit	ppb ppb	85.3 79.2	63.0 47.9	23.9 14.1	108.8 104.2	90.9 77.2	48.5 31.8	24.2 13.7						24.1 13.8
Total Outflow Volume + Bypass Total Outflow Load + Bypass	hm3/yr kg/yr	65.7 5426	59.4 3316	54.2 1002	113.8 12138	114.1 9654	114.4 4573	114.8 2100						169.0 3102.0
Bypass Load	kg/yr	0	0	0	0	0	40.0 0	0						0.0
Maximum Inflow Maximum Outflow	hm3/d hm3/d	0.96	1.00	0.98	2.03	2.03	2.04	2.03						2.98
Surface Load Reduction	kg/yr kg/yr	1046 922	2110 1785	2313 2255	1600 1148	2484 2565	5082 5171	2473 2556						17108 16402
Overall Load Reduction Lower Confidence Limit	%	16% 13%	39% 33%	70% 65%	12% 10%	20% 16%	53% 46%	54% 50%						85% 80%
Upper Confidence Limit Daily Geometric Mean	% ppb	20% 75.8	45% 52.4	73%	14% 100.3	26% 79.9	59% 34.9	57% 14.6						88% #N/A
Outflow Geo Mean - Composites Upper Confidence Limit	ppb ppb	76.1 78.5	52.5 59.69	15.3 20.6	100.4 102.6	80.0 86.5	35.3 43.8	14.8 20.6						14.9 20.5
Lower Confidence Limit Frequency Outflow Conc > 10 ppb	ppb %	72.9 100%	44.56 100%	11.0 95%	97.7 100%	72.4 100%	27.3 100%	10.4 90%						10.6 92%
Frequency Outflow Conc > 20 ppb Frequency Outflow Conc > 50 ppb	% %	100% 100%	100% 67%	14% 0%	100% 100%	100% 100%	99% 5%	15% 0%						51% 15%
Freq Outflow Volume > 10 ppb 95th Percentile Outflow Conc	% ppb	100% 98	100% 62	99% 23	100% 127	100% 93	100% 50	97% 24						98% 24
Storage Increase / Net Removal	mg/m2 %	3044	2491	0%	3809	0%	0%	842 0%						2094
Unit Area P Removal Mean Water Load	g/m2-yr	969	794	1011 7 3	1207	1078	1981 12.0	841 10.3						1138
Max Water Load Max Dopth	cm/d	100.4	44.5	44.1	213.2	85.5	78.1	66.6						20.7
Minimum Depth Maximum Depth	cm	45 111	40	26	90 132	37	49 107	36						42
Frequency Depth < 10 cm Flow/Width	% m2/dav	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%						0.0%
HRT Days Mean Velocity	days cm/sec	4.5	6.8 0.25	8.5	2.9	4.8	5.6 0.35	6.5 0.35						20.6
Seepage Outflow / Total Outflow Release 1 Outflow Volume	% hm3/yr	0%	0%	0%	0%	0%	0%	0%						0%
Release 2 Outflow Volume 95th Percentile Outflow Volume	hm3/yr hm3/d	0.0 0.43	0.0 0.43	0.0 0.43	0.0 0.83	0.0 0.85	0.0 0.86	0.0 0.89						0.0 1.3
95th Percentile Outflow Load Simulated / Specified Mean Depth	kg/d %	39.33 #N/A	25.72 #N/A	9.35 #N/A	94.35 #N/A	76.11 #N/A	39.66 #N/A	19.93 #N/A						28.9 #N/A
Release 1 Demand Met Release 2 Demand Met	%	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A						#N/A #N/A
Outflow Demand Met Range Check - Mean Depth	%	#N/A -	#N/A -	#N/A 0.99	#N/A 1.23	#N/A -	#N/A -	#N/A -						#N/A 2
Range Check - Freq Depth < 10 cm Range Check - Flow/Width	1	1.06	1	0.65	2.25	1	1	1						0 3
Range Check - Inflow Conc Range Check - Outflow Conc	1			-	-	1	1	-						0
Water Balance Error Mass Balance Error	% %	0.00%	0.00%	0.00%	0.00%	0.00% 0.01%	0.00% 0.01%	0.00% 0.01%						0.01%
warning or Error Messages		Cell# 1 EDCE Fle Cell# 3 2 Depth o	ow/Width out of ca out of calib. range	Inb. range for EMG for SAV_3: 62 vs	_3: 222 vs. 26 - . 62 - 87 cm	210 m2/day								5
		Cell# 3 2 Flow/W Cell# 4 EDCW D	epth out of calib. r	ange for SAV_3: ange for EMG_3:	94 vs. 35 - 76 cm	m2/day								
		Com + EDCW F	our violation out of c	and: range for EMC	2_3. 471 VS. 26 -	2.10 m2/day								





# Table A.6 STA-1E: Cells 5-7: Case "5\_7 2006Base"

DWSTA2- Inputs & Outpu	uts		Project:	PROJECT_S	TATE							Cu	rrent Date:	08/03/05
Input Variable Design Case Name Input Series Name	Units -	Value 5_7 2006Base	Case Descrip Cells 5-7 of	tion: STA-1E with W	/est Distributio	n Cell	2 014 alua (	00% # CT		et E 240				
Starting Date for Simulation	'	05/01/65	Cell-to-cell s	eepage not co	nsidered in an	alysis	3-311 pius 2	20% 01 31	A-TE INNOW	at 3-319				
Starting Date for Simulation Starting Date for Output	-	05/01/65	Simulation Tu	oution Cell mod	Lincortainty A	ns in paralle								
Number of Iterations	- davs	0 7	Output Variat	ole (C (ppb)	Mean 19.5	Lower CL 21 7	Upper CL 18.0		Diagnostic H20 Balan	is ice Error Me	an & Max	0.0%	0.0%	
Inflow Conc Scale Factor Rainfall P Conc	- ppb	1	GM Outflow 0	C (ppb) on %	5.9 87%	7.6 85%	4.9 88%		Mass Bala Iterations	nce Error M & Converge	lean & Max	0.1%	0.2%	
Atmospheric P Load (Dry) Cell Number>	mg/m2-yr	20	Bypass Load 2	(%)	0.0%	5	6	7	Warning/E 8	rror Messa 9	ges 10	3	12	
Cell Label Vegetation Type	>	EMG_3	7 EMG_3	WDCE EMG_3	5 EMG_3	6 SAV_3								
Inflow Fraction Downstream Cell Number	1	0.4 2	5	0.6 4	5									
Surface Area Mean Width of Flow Path	km2 km	1.17 0.75	1.69 1.18	1.17 0.75	2.31 1.61	4.25 1.61								
Number of Tanks in Series Minimum Depth for Releases	- cm	0.5	3.0	0.5	3.0	3.0								
Release 1 Series Name Release 2 Series Name														
Depth Series Name		400	40	40	40									
Outflow Control Depth Outflow Weir Depth	cm	100	40	40	40	60								
Outflow Coefficient - Intercept	-	1	1	1	1	1								
Maximum Inflow Maximum Outflow	hm3/day													
Inflow Seepage Rate Inflow Seepage Control Elev	(cm/d) / cm cm		0.0054 69			0.0057 94								
Inflow Seepage Conc Outflow Seepage Rate	ppb (cm/d) / cm	0.01	20	0.01		20								
Outflow Seepage Control Elev Max Outflow Seepage Conc	cm ppb	-15 20		-76 20										
Seepage Recycle to Cell Number Seepage Recycle Fraction	1													
Seepage Discharge Fraction Initial Water Column Conc	- ppb	30	30	30	30	30								
Initial P Storage Per Unit Area Initial Water Column Depth	mg/m2 cm	500 200	500 200	500 200	500 200	500 200								
C0 = Conc at 0 g/m2 P Storage C1 = Conc at 1 g/m2 P storage	ppb	3 22	3 22	3 22	3 22	3 22								
K = Net Settling Rate at Steady State	ppb m/yr	300 16.8	300 16.8	300 16.8	300 16.8	300 52.5								
Z1 = Saturated Optake Depth Z2 = Lower Penalty Depth Z2 = Upper Penalty Depth	cm	40 100 200	40 100 200	40 100 200	40 100 200	40 100 200								
Output Variables	Units	1	200	3	4	5	6	7	8	9	10	11	12	Overall
Execution Time Run Date	sec/yr	5.37 08/03/05	5.86 08/03/05	6.17 08/03/05	6.66 08/03/05	7.17 08/03/05								7.17 08/03/05
Starting Date for Simulation Starting Date for Output	1	05/01/65	05/01/65	05/01/65	05/01/65	05/01/65								05/01/65
Ending Date Output Duration	- days	04/30/00 12784	04/30/00 12784	04/30/00 12784	04/30/00 12784	04/30/00 12784								04/30/00 12784
Cell Label Downstream Cell Label		WDCW 7	7 6	WDCE 5	5 6	6 Outflow								Total
Network Simulation Name Simulation Type	1	none Uncerta	none Uncerta	none Uncerta	none Uncerta	none Uncerta								none Uncerta
Surface Area Mean Rainfall	km2 cm/yr	1.17 142.9	1.69 142.9	1.17 142.9	2.31 142.9	4.25 142.9								10.58 142.9
Mean E I Cell Inflow Volume	cm/yr hm3/yr	129.7 20.6	129.7 16.1	129.2 30.8	129.4 25.8	129.7 43.3								129.5 51.4
Cell Inflow Load Cell Inflow Conc Tracted Outflow Volume	кg/yr ppb bm2/vr	2753 133.9	1437 89.4 17.2	4130 133.9 25.9	2486 96.4 26.1	47.0								133.9
Treated Outflow Load	kg/yr	1437	739	23.8 2486 96.4	1292	913 19.5								913 19.5
Upper Confidence Limit	ppb	96.8 81.8	51.3 36.3	102.3	58.6 41.5	21.7								21.7
Total Outflow Volume + Bypass Total Outflow Load + Bypass	hm3/yr kg/yr	16.1 1437	17.2 739	25.8 2486	26.1 1292	46.7 913								46.7 912.7
Total FWM Outflow Conc Bypass Load	ppb kg/yr	89.4 0	43.1 0	96.4 0	49.5 0	19.5 0								19.5 0.0
Bypass Load Maximum Inflow	% hm3/d	0.0 1.16	0.0 1.14	0.0 1.74	0.0 1.75	0.0 3.17								0.0 2.90
Maximum Outflow Surface Load Reduction	hm3/d kg/yr	1.14 1316	1.26 697	1.75 1644	1.91 1194	3.46 1119								3.46 5970
Load Trapped in Sediments Overall Load Reduction	kg/yr %	1031 48%	770 49%	1083 40%	1270 48%	1319 55%								5473 87%
Lower Confidence Limit Upper Confidence Limit	%	44% 52%	43%	36%	42%	58% 51%								85% 88%
Outflow Geo Mean - Composites	ppb	71.5	23.4 24.6	76.3	32.3	5.2								#N/A 5.9
Lower Confidence Limit	ppb	63.2	17.86	69.6 100%	40.5 23.0 100%	4.9								4.9
Frequency Outflow Conc > 20 ppb Frequency Outflow Conc > 20 ppb	%	100%	74%	100%	96%	4%								6% 4%
Freq Outflow Volume > 10 ppb 95th Percentile Outflow Conc	% ppb	100%	100%	100% 102	100% 48	42% 17								42% 17
Mean Biomass P Storage Storage Increase / Net Removal	mg/m2 %	2790 0%	1429 0%	2920 0%	1725 0%	311 0%								1359 0%
Net Storage Turnover Rate Unit Area P Removal	1/yr g/m2-yr	11.1 885	11.1 455	11.1 930	11.1 550	34.9 311								13.3 517
Mean Water Load Max Water Load	cm/d cm/d	4.8 99.5	2.6 67.4	7.2 149.3	3.1 75.6	2.8 74.7								1.3 27.4
Mean Depth Minimum Depth	cm cm	95 13	43 25	47 1	42 1	61 49								56 26
Maximum Depth Frequency Depth < 10 cm	cm %	115 0.0%	96 0.0%	115 2.7%	100 1.0%	117 0.0%								110 0.5%
How/Width HRT Days	m2/day days	75 19.6	37	113 6.6	44 13.9	74 21.9								65.7 42.4
Mean Velocity Seepage Outflow / Total Outflow	cm/sec %	0.09	0.10	0.27	0.12	0.14								0.14
Release 2 Outflow Volume	hm3/yr	0.0	0.0	0.0	0.0	0.0								0.0
95th Percentile Outflow Load Simulated / Specified Mean Depth	kg/d	12.95 #N/A	5.91 #N/A	20.98 #N/A	10.48 #N/A	6.00 #N/A								6.0 #N/A
Release 1 Demand Met Release 2 Demand Met	%	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A								#N/A #N/A
Outflow Demand Met Range Check - Mean Depth	%	#N/A 1.24	#N/A	#N/A	#N/A	#N/A 0.99								#N/A 2
Range Check - Freq Depth < 10 cm Range Check - Flow/Width	1	1	1	1	1	- 0.45								0
Range Check - Inflow Conc Range Check - Outflow Conc	-	1	1	1	1	1								0
Water Balance Error Mass Balance Error	% %	0.00%	0.00%	0.00%	0.00%	0.00% 0.18%								0.00% 0.14%
Warning or Error Messages		Cell# 1 WDCW E Cell# 5 6 Depth e	Depth out of calib. out of calib. range	range for EMG_3: for SAV_3: 61 vs	95 vs. 35 - 76 cr . 62 - 87 cm	n								3
		Cell# 5 6 Flow/W	nath out of calib. n	ange for SAV_3:	74 vs. 162 - 374 r	n2/day								

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Table A.7	STA-1E:	Cells 1-4:	Case "1	4 2006Mod"
1 4010 1107			Cube I	

DMSTA2- Inputs & Outpu	its		Project:	PROJECT_S	FA1E							Mode Cu	rrent Date:	7/1/2005 08/03/05
Input Variable Design Case Name	Units	Value	Case Descrip	tion: f STA-1E with	East Distributio	on Cell								
Input Series Name Starting Date for Simulation		TS_1_42006A 05/01/65	Inflows from Cell-to-cell s	C-51 West ad	justed for sam	e-day bypas alysis; inflow	s to S-155A	of L-8 run by 4% (ap	off through a prox. 6,400	S-5AE, ac-ft/yr) for	seepage re	cycle from v	vest flow pa	ith
Ending Date for Simulation Starting Date for Output	1	04/30/00 05/01/65	East Distribu	ition Cell mode	eled as two cel	s in parallel						-		
Integration Steps Per Day Number of Iterations	1	4 0	Simulation Typ Output Variab	be: ble	Uncertainty A Mean	nalysis Lower CL	Upper CL		Diagnostic	s				
Output Averaging Interval Inflow Conc Scale Factor	days -	7	FWM Outflow GM Outflow C	C (ppb) C (ppb)	24.3 20.5	31.2 27.3	18.4 14.8		H20 Balan Mass Bala	ce Error Me nce Error M	an & Max ean & Max	0.0%	0.0% 0.0%	
Rainfall P Conc Atmospheric P Load (Dry)	ppb mg/m2-yr	10 20	Bypass Load	on % (%)	80% 0.0%	74%	85%	-	Warning/E	rror Messag	nce jes	5	0.0%	
Cell Label	-	EDCE	2 1 EMG 3	2 SAV 3	EDCW	3 EMG 3	4N SAV 3	4S SAV 3	•	9			12	
Inflow Fraction Downstream Cell Number	÷	0.333	3	0,1120	0.707	6	7	0/11_0						
Surface Area Mean Width of Flow Path	km2 km	0.95 0.66	2.25 1.55	2.23 1.55	0.95	2.38 1.55	2.61 1.55	3.04 1.55						
Number of Tanks in Series Minimum Depth for Releases	- cm	0.5	3.0	3.0	0.5	3.0	3.0	3.0						
Release 1 Series Name Release 2 Series Name														
Outflow Series Name Depth Series Name Outflow Control Dopth		40	40	60	90	40	60	60						
Outflow Weir Depth Outflow Coefficient - Exponent	cm	40	40	4	4	40	4	4						
Outflow Coefficient - Intercept Bypass Depth	- cm	1	1	1	1	1	1	1						
Maximum Inflow Maximum Outflow	hm3/day hm3/day													
Inflow Seepage Rate Inflow Seepage Control Elev	(cm/d) / cm cm													
Outflow Seepage Conto Outflow Seepage Rate Outflow Seepage Control Elev	(cm/d) / cm	0.0095	0.0042	0.0042	0.0095			0.0054						
Max Outflow Seepage Conc Seepage Recycle to Cell Number	ppb -	20	20	20 1	20			20 7						
Seepage Recycle Fraction Seepage Discharge Fraction	1	1	1	1	1			1						
Initial Water Column Conc Initial P Storage Per Unit Area	ppb mg/m2	30 500	30 500	30 500	30 500	30 500	30 500	30 500				1		
C0 = Conc at 0 g/m2 P Storage	ppb	50 3	50 3	50 3 22	50 3	50 3	50 3	50 3						
C2 = Conc at Half-Max Uptake K = Net Settling Rate at Steady State	ppb m/vr	300 16.8	300 16.8	300 52.5	300 16.8	300 16.8	300 52.5	300 52.5						
Z1 = Saturated Uptake Depth Z2 = Lower Penalty Depth	cm	40 100	40 100	40 100	40 100	40 100	40 100	40 100						
Z3 = Upper Penalty Depth	cm	200	200	200	200	200	200	200	l	l				
Output Variables Execution Time	Units sec/yr	1 6.54	2 7.00	3 7.46	4 7.74	5 8.20	6 8.66	7 9.14	8	9	10	11	12	9.14
Run Date Starting Date for Simulation	-	05/01/65	08/03/05 05/01/65	08/03/05 05/01/65	08/03/05 05/01/65	08/03/05	08/03/05	08/03/05						08/03/05
Ending Date Output Duration	- - davs	04/30/00	04/30/00	04/30/00	04/30/00	04/30/00	04/30/00	04/30/00						04/30/00
Cell Label Downstream Cell Label	days	EDCE 1	1 2	2 Outflow	EDCW 3	3 4N	4N 4S	4S Outflow						Total
Network Simulation Name Simulation Type	1	none Uncerta	none Uncerta	none Uncerta	none Uncerta	none Uncerta	none Uncerta	none Uncerta						none Uncerta
Surface Area Mean Rainfall	km2 cm/yr	0.95 142.9	2.25 142.9	2.23 142.9	0.95	2.38 142.9	2.61 142.9	3.04 142.9						14.41 142.9
Cell Inflow Volume	hm3/yr	66.3 8012	78.6	72.2	129.7 140.7 17010	129.7 140.8 15277	129.7	129.7						207.0
Cell Inflow Conc Treated Outflow Volume	ppb hm3/vr	120.9	87.7 72.2	63.2 67.0	120.9	109.2	90.6 141.5	48.8						120.9
Treated Outflow Load Treated FWM Outflow Conc	kg/yr ppb	6889 87.7	4565 63.2	1607 24.0	15377 109.2	12780 90.6	6897 48.8	3469 24.5						5076 24.3
Upper Confidence Limit Lower Confidence Limit	ppb ppb	90.1 84.6	70.0 55.5	30.5 18.4	110.9 107.1	96.0 84.0	57.4 40.1	31.6 18.5						31.2 18.4
Total Outflow Volume + Bypass Total Outflow Load + Bypass	hm3/yr kg/yr	78.6 6889	72.2 4565	67.0 1607	140.8 15377	141.1 12780	141.5 6897	141.9 3469						208.9 5076.5
Bypass Load	kg/yr %	0	0	0	0	0	0	0						0.0
Maximum Inflow Maximum Outflow	hm3/d hm3/d	1.18 1.23	1.23 1.21	1.21 1.18	2.51 2.52	2.52 2.52	2.52 2.51	2.51 2.48						3.69 3.66
Surface Load Reduction Load Trapped in Sediments	kg/yr kg/yr	1122 969	2324 1951	2958 2864	1633 1162	2597 2678	5882 5971	3428 3478						19945 19073
Overall Load Reduction Lower Confidence Limit	%	14% 12%	34% 29%	65% 60%	10% 8%	17% 13%	46% 40%	50% 45%						80% 74%
Daily Geometric Mean Outflow Geo Mean - Composites	ppb ppb	80.6 80.8	40% 59.5 59.6	20.1	102.6 102.8	85.6 85.7	43.2 43.5	20.3						#N/A 20.5
Upper Confidence Limit Lower Confidence Limit	ppb	83.0 78.0	66.40 51.87	26.8 15.0	104.7 100.5	91.4 79.0	52.2 34.9	27.6 14.8						27.3 14.8
Frequency Outflow Conc > 10 ppb Frequency Outflow Conc > 20 ppb	%	100% 100%	100% 100%	100% 55%	100% 100%	100% 100%	100% 100%	99% 56%						99% 88%
Frequency Outflow Conc > 50 ppb Freq Outflow Volume > 10 ppb	%	100% 100%	97% 100%	0% 100%	100% 100%	100% 100%	26% 100%	0% 100%						56% 100%
Mean Biomass P Storage Storage Increase / Net Removal	mg/m2	104 3200 0%	2723 0%	30 1287 0%	131 3880 0%	99 3532 0%	60 2292 0%	32 1146 0%						31 2332 0%
Net Storage Turnover Rate Unit Area P Removal	1/yr g/m2-yr	11.1 1019	11.1 867	34.9 1284	11.0 1222	11.1 1125	34.9 2288	34.9 1144						19.9 1323
Mean Water Load Max Water Load	cm/d cm/d	19.1 124.3	9.6 54.6	8.9 54.3	40.5 264.0	16.2 105.7	14.8 96.7	12.7 82.6						3.9 25.6
Mean Depth Minimum Depth	cm cm	72 47	56 40	63 35	95 90	66 38	70 53	70 43						68 46
Maximum Depth Frequency Depth < 10 cm Flow/Midth	cm % m2/dov/	117 0.0% 275	94 0.0%	93 0.0%	139 0.0%	113 0.0%	113 0.0% 249	112 0.0% 250						109 0.0%
HRT Days Mean Velocity	days	3.8	5.9	7.1	2.4	4.0	4.7	5.5						17.2
Seepage Outflow / Total Outflow Release 1 Outflow Volume	% hm3/yr	0%	0%	0%	0%	0%	0%	0%						0%
Release 2 Outflow Volume 95th Percentile Outflow Volume	hm3/yr hm3/d	0.0 0.52	0.0 0.52	0.0 0.52	0.0 1.03	0.0 1.05	0.0 1.06	0.0 1.09						0.0 1.6
95th Percentile Outflow Load Simulated / Specified Mean Depth	kg/d %	50.95 #N/A	35.08 #N/A	14.58 #N/A	118.99 #N/A	99.99 #N/A	59.33 #N/A	31.83 #N/A						46.5 #N/A
Release 1 Demand Met Release 2 Demand Met	%	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A #N/A	#N/A #N/A #N/A						#N/A #N/A #N/A
Range Check - Mean Depth Range Check - Freg Depth < 10 cm	-	#N/A	#IN/A	-	1.25	+IN/A -	#N/A	-						#N/A 1 0
Range Check - Flow/Width Range Check - Inflow Conc	1	1.31	-	0.79	2.78	1.18	-							4
Range Check - Outflow Conc Water Balance Error	- %	0.00%	- 0.00%	- 0.00%	- 0.00%	- 0.00%	- 0.00%	- 0.00%						0.01%
Mass Balance Error Warning or Error Messages	%	0.00% Cell# 1 EDCE Fi	0.00% ow/Width out of ca	0.00% lib. range for EMG	0.00% _3: 275 vs. 26 -	0.01% 210 m2/day	0.01%	0.01%						0.01% 5
		Cell# 3 2 Flow/W Cell# 4 EDCW D	ridth out of calib. ra lepth out of calib. r	ange for SAV_3: ange for EMG_3:	128 vs. 162 - 374 95 vs. 35 - 76 cm	m2/day								
		Cell# 5 3 Flow/W	idth out of calib. ra	inge for EMG_3:	249 vs. 26 - 210 r	n2/day								

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# Table A.8 STA-1E: Cells 5-7: Case "5\_7 2006Mod"

DMSTA2- Inputs & Outpu	uts		Project:	PROJECT_S	FA1E							Mode Cu	el Release: rrent Date:	7/1/2005 08/07/05
Input Variable Design Case Name	Units -	Value 5_7 2006Mod	Case Descrip Cells 5-7 of	tion: STA-1E with W	est Distributio	n Cell								
Input Series Name Starting Date for Simulation	-	TS_ST1IDAII 05/01/65	Cell-to-cell s	ed to discharge eepage not co	s from G-311, nsidered in an	set at 30% c alysis	of total inflow	/s to STA-	1 I&D Works	s (S-5A Bas	sin and EBW	CD Runoff)		
Ending Date for Simulation Starting Date for Output	-	04/30/00 05/01/65	West Distrib	ution Cell mod	eled as two ce	lis in paralle								
Number of Iterations	- - davs	0	Output Variat	pe. <u>ple</u> (C (ppb)	Mean 27.2	Lower CL 34.9	Upper CL 21.5		Diagnostic: H20 Balan	s ce Error Me	an & Max	0.0%	0.0%	
Inflow Conc Scale Factor Rainfall P Conc	- ppb	1 10	GM Outflow 0 Load Reducti	C (ppb) on %	17.5 85%	24.5 81%	12.3 88%		Mass Bala Iterations 8	nce Error M Converge	lean & Max nce	0.1%	0.1% 0.0%	
Atmospheric P Load (Dry) Cell Number>	mg/m2-yr	20 1	Bypass Load 2	(%) <u>3</u>	0.0% 4	5	6	7	Warning/E 8	rror Messag 9	ges 10	3 11	12	
Cell Label Vegetation Type	>	EMG_3	7 EMG_3	EMG_3	5 EMG_3	6 SAV_3								
Inflow Fraction Downstream Cell Number	- -	0.12 2 1.17	5	0.18 4 1.17	5	4.25								
Mean Width of Flow Path Number of Tanks in Series	km	0.75	1.18	0.75	1.61	1.61								
Minimum Depth for Releases Release 1 Series Name	cm													
Release 2 Series Name Outflow Series Name														
Depth Series Name Outflow Control Depth	cm	100	40	40	40	60								
Outflow Coefficient - Exponent	-	4	4	4	4	4								
Bypass Depth Maximum Inflow	cm hm3/day	•	•	•										
Maximum Outflow Inflow Seepage Rate	hm3/day (cm/d) / cm		0.0054			0.0057								
Inflow Seepage Control Elev Inflow Seepage Conc	cm ppb	0.04	69 20			94 20								
Outflow Seepage Control Elev Max Outflow Seepage Conc	(cm/d) / cm cm	-15		-76										
Seepage Recycle to Cell Number	-	20		20										
Seepage Discharge Fraction Initial Water Column Conc	- ppb	30	30	30	30	30								
Initial P Storage Per Unit Area Initial Water Column Depth	mg/m2 cm	500 200	500 200	500 200	500 200	500 200								
C0 = Conc at 0 g/m2 P Storage C1 = Conc at 1 g/m2 P storage	ppb ppb	3 22	3 22	3 22	3 22	3 22								
C2 = Conc at Half-Max Uptake K = Net Settling Rate at Steady State	ppb m/yr	300	300 16.8	300 16.8	300 16.8	300 52.5								
Z1 = Saturated Optake Depth Z2 = Lower Penalty Depth Z3 = Upper Penalty Depth	cm	100	100	100	100	100								
Output Variables	Units	1	2	3	4	5	6	7	8	9	10	11	12	Overall
Execution Time Run Date	sec/yr -	4.57 08/07/05	5.03 08/07/05	5.31 08/07/05	5.83 08/07/05	6.37 08/07/05								6.37 08/07/05
Starting Date for Simulation Starting Date for Output	-	05/01/65 05/01/65	05/01/65 05/01/65	05/01/65 05/01/65	05/01/65 05/01/65	05/01/65 05/01/65								05/01/65 05/01/65
Ending Date Output Duration	- days	04/30/00 12784	04/30/00 12784	04/30/00 12784	04/30/00 12784	04/30/00 12784								04/30/00 12784
Downstream Cell Label		7	6 none	5 none	6 0000	Outflow								- -
Simulation Type Surface Area	- km2	Uncerta 1.17	Uncerta 1.69	Uncerta 1.17	Uncerta 2.31	Uncerta 4.25								Uncerta 10.58
Mean Rainfall Mean ET	cm/yr cm/yr	142.9 129.7	142.9 129.7	142.9 120.9	142.9 129.7	142.9 129.7								142.9 128.7
Cell Inflow Volume Cell Inflow Load	hm3/yr kg/yr	37.0 6456	32.9 4392 133.5	55.6 9683	50.9 6623	85.1 7105								92.6 16139 174.2
Treated Outflow Volume	hm3/yr kg/yr	32.9	33.9	50.9	51.2 4336	88.3 2405								88.3 2405
Treated FWM Outflow Conc Upper Confidence Limit	ppb	133.5 141.1	81.7 95.6	130.1 136.1	84.6 97.3	27.2 34.9								27.2 34.9
Lower Confidence Limit Total Outflow Volume + Bypass	ppb hm3/yr	124.7 32.9	67.6 33.9	123.4 50.9	71.6 51.2	21.5 88.3								21.5 88.3
Total Outflow Load + Bypass Total FWM Outflow Conc	kg/yr ppb	4392 133.5	2769 81.7	6623 130.1	4336 84.6	2405 27.2								2404.9 27.2
Bypass Load Bypass Load Maximum Inflow	kg/yr % bm3/d	0.0	0.0	0.0	0.0	0.0								0.0
Maximum Outflow Surface Load Reduction	hm3/d kg/yr	1.10	1.14	1.62	1.71	3.02 4700								3.02 13734
Load Trapped in Sediments Overall Load Reduction	kg/yr %	1581 32%	1694 37%	1288 32%	2365 35%	4894 66%								11823 85%
Lower Confidence Limit Upper Confidence Limit	%	28% 36%	30% 44%	28% 35%	28% 42%	63% 68%								81% 88%
Daily Geometric Mean Outflow Geo Mean - Composites	ppb ppb	119.9 127.1	67.4 70.0	149.1 116.5	76.7	15.4 17.5								#N/A 17.5
Lower Confidence Limit Frequency Outflow Conc > 10 ppb	ppb %	117.8	55.97 100%	109.6	60.7 100%	12.3 91%								24.5 12.3 91%
Frequency Outflow Conc > 20 ppb Frequency Outflow Conc > 50 ppb	%	100% 100%	100% 97%	100% 100%	100% 100%	35% 1%								65% 35%
Freq Outflow Volume > 10 ppb 95th Percentile Outflow Conc	% ppb	100% 146	100% 94	100% 157	100% 98	99% 34								99% 34
Mean Biomass P Storage Storage Increase / Net Removal	mg/m2 %	4272 0%	3143 0%	3471 0%	3213 0%	1155 0%								2521 0%
Unit Area P Removal Mean Water Load	g/m2-yr	1357	11.1 1001 5.3	11.1 1105 13.1	11.1 1023 6.0	34.9 1153 5.5								15.5 1118 2.4
Max Water Load Mean Depth	cm/d cm	95.0 86	65.0 48	142.6 45	70.1 47	67.1 65								26.2 58
Minimum Depth Maximum Depth	cm cm	5 112	27 97	1 119	7 100	50 116								27 109
Frequency Depth < 10 cm Flow/Width	% m2/day	0.1% 135	0.0% 76	13.4% 203	0.1% 87	0.0% 145								1.5% 126.4
HRT Days Mean Velocity	days cm/sec	9.9 0.18	8.9 0.19	3.4 0.52	7.8 0.21	11.8 0.26								24.3 0.26
Release 1 Outflow Volume Release 2 Outflow Volume	hm3/yr hm3/yr	0.0	0.0	0.0	0.0	0.0								0.0
95th Percentile Outflow Volume 95th Percentile Outflow Load	hm3/d kg/d	0.42 55.43	0.41 35.75	0.61 82.25	0.60 54.64	1.00 31.13								1.0 31.1
Simulated / Specified Mean Depth Release 1 Demand Met	%	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A								#N/A #N/A
Release 2 Demand Met Outflow Demand Met	% %	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A								#N/A #N/A
Range Check - Mean Depth Range Check - Freq Depth < 10 cm Range Check - Flow/Width	-	1.12	-	1.50	-									1
Range Check - Inflow Conc Range Check - Outflow Conc	-	-	-	-	-	-								0
Water Balance Error Mass Balance Error	% %	0.00% 0.01%	0.00% 0.05%	0.00% 0.10%	0.00%	0.00% 0.04%								0.00% 0.10%
Warning or Error Messages		Cell# 1 WDCW E Cell# 3 WDCE F	Depth out of calib. req Z < 10 cm out	range for EMG_3: of calib. range for	86 vs. 35 - 76 cr EMG_3: 13 vs. 0	n ) - 9 %								3
		Cell#56 Flow/W	ndth out of calib. ra	ange for SAV_3:	145 vs. 162 - 374	m2/day								

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# Table A.9 STA-2: Case "Exist"

DMSTA2- Inputs & Outpu	uts		Project:	PROJECT_S	FA2							Mod Cu	el Release: urrent Date:	7/1/2005 08/03/05
Input Variable Design Case Name	Units -	Value Exist	Case Descrip STA-2 Expa	nded to Include	e Cell 4								,	1
Input Series Name Starting Date for Simulation	-	TS_2006Base 05/01/65	Inflow time s Analysis for	series includes WY 1966-2000	allowance of 3 ) prior to additi	8 cfs (27,50 on of Cell 4	10 ac-ft/yr) s	eepage fro	m WCA-2A	to Supply C	anal			
Ending Date for Simulation Starting Date for Output	1	04/30/00 05/01/65	0. L.C. T											
Number of Iterations	- - dave	4 0 7	Output Variat	pe: <u>ble</u> (C (ppb)	Mean 27.8	Lower CL 33.8	Upper CL 22.5		Diagnostic H20 Balan	s e Error Me	an & Max	0.0%	0.0%	
Inflow Conc Scale Factor Rainfall P Conc	- ppb	1 10	GM Outflow C	C (ppb) C (ppb) on %	21.0 21.1 73%	27.2	15.9 78%		Mass Bala Iterations 8	nce Error M Converger	ean & Max	0.1%	0.2%	
Atmospheric P Load (Dry) Cell Number>	mg/m2-yr	20	Bypass Load 2	(%) 3	0.0% 4	5	6	7	Warning/E 8	rror Messag 9	jes 10	0 11	12	
Cell Label Vegetation Type	- >	1 PEW_3	2 PEW_3	3 SAV_3										
Inflow Fraction Downstream Cell Number		0.23	0.29	0.48										
Surface Area Mean Width of Flow Path	km2 km	7.28	9.19 2.00	9.19 2.00										
Minimum Depth for Releases Release 1 Series Name	cm	3.0	3.0	0.0										
Release 2 Series Name Outflow Series Name														
Depth Series Name Outflow Control Depth	cm	40	40	60										
Outflow Weir Depth Outflow Coefficient - Exponent	cm -	4	4	4										
Bypass Depth Maximum Inflow	- cm bm2/day	1	1	1										
Maximum Innow Maximum Outflow Inflow Seepage Rate	hm3/day (cm/d) / cm	0.008												
Inflow Seepage Control Elev Inflow Seepage Conc	cm	76 20												
Outflow Seepage Rate Outflow Seepage Control Elev	(cm/d) / cm cm	0.004 -61	0.006 -61	0.01 -30										
Max Outflow Seepage Conc Seepage Recycle to Cell Number	ppb -	20 1	20 2	20 3										
Seepage Recycle Fraction Seepage Discharge Fraction Initial Water Column Conc	-	1	30	1	20									
Initial P Storage Per Unit Area Initial Water Column Depth	mg/m2 cm	500 200	500 200	500 200	500 200									
C0 = Conc at 0 g/m2 P Storage C1 = Conc at 1 g/m2 P storage	ppb ppb	3 22	3 22	3 22	3 22									
C2 = Conc at Half-Max Uptake K = Net Settling Rate at Steady State	ppb m/yr	300 34.9	300 34.9	300 52.5	300 52.5									
Z1 = Saturated Uptake Depth Z2 = Lower Penalty Depth	cm cm	40 100	40 100	40 100	40 100									
23 = Upper Penalty Depth	cm	200	200	200	200	5		7			10	11	12	Overall
Execution Time Run Date	sec/yr	4.29 08/03/05	4.74 08/03/05	5.49 08/03/05						3				5.49 08/03/05
Starting Date for Simulation Starting Date for Output	1	05/01/65 05/01/65	05/01/65 05/01/65	05/01/65 05/01/65										05/01/65 05/01/65
Ending Date Output Duration	- days	04/30/00 12784	04/30/00 12784	04/30/00 12784										04/30/00 12784
Cell Label Downstream Cell Label		1 Outflow	2 Outflow	3 Outflow										Total
Network Simulation Name Simulation Type Surface Area	- -	Uncerta	Uncerta	Uncerta										Uncerta
Mean Rainfall Mean ET	cm/yr cm/yr	128.6 130.3	128.6 130.3	128.6 130.3										128.6 130.3
Cell Inflow Volume Cell Inflow Load	hm3/yr kg/yr	97.5 9964	122.9 12564	203.4 20795										423.8 43323
Cell Inflow Conc Treated Outflow Volume	ppb hm3/yr	102.2 101.6	102.2 122.7	102.2 203.3										102.2 427.6
Treated Outflow Load Treated FWM Outflow Conc	kg/yr ppb	2820 27.7	3426 27.9	5627 27.7										11873 27.8
Upper Confidence Limit Lower Confidence Limit Total Outflow Volume + Bypass	ppb ppb	34.5 22.0	34.b 22.2 122.7	23.0 23.0 203.3										22.5 427.6
Total Outflow Load + Bypass Total FWM Outflow Conc	kg/yr ppb	2820	3426 27.9	5627 27.7										11873.3 27.8
Bypass Load Bypass Load	kg/yr %	0 0.0	0 0.0	0 0.0										0.0 0.0
Maximum Inflow Maximum Outflow	hm3/d hm3/d	1.91 1.95	2.41 2.46	3.98 4.09										8.30 8.50
Surface Load Reduction Load Trapped in Sediments	kg/yr kg/yr	7144 7253 729/	9138 9043 729/	15168 14758										31450 31054
Lower Confidence Limit	76 %	65% 78%	66% 78%	68% 78%										67% 78%
Daily Geometric Mean Outflow Geo Mean - Composites	ppb ppb	21.3 21.8	21.1 21.9	19.3 20.1										#N/A 21.1
Upper Confidence Limit Lower Confidence Limit	ppb ppb	28.4 16.2	28.49 16.26	25.4 15.5										27.2 15.9
Frequency Outflow Conc > 10 ppb Frequency Outflow Conc > 20 ppb	%	100% 62%	100% 63%	99% 49%										100% 90%
Frequency Outflow Conc > 50 ppb Freq Outflow Volume > 10 ppb 95th Percentile Outflow Conc	% 000	100% 34	100% 34	100% 34										56% 100% 34
Mean Biomass P Storage Storage Increase / Net Removal	mg/m2 %	1501 0%	1482 0%	1609 0%										1533 0%
Net Storage Turnover Rate Unit Area P Removal	1/yr g/m2-yr	23.2 996	23.2 984	34.9 1606										27.6 1210
Mean Water Load Max Water Load	cm/d cm/d	3.7 26.2	3.7 26.2	6.1 43.3										4.5 32.3
Minimum Depth Maximum Depth	cm	58 40 105	57 33 105	70 59										62 44
Frequency Depth < 10 cm Flow/Width	% m2/day	0.0%	0.0%	0.0%										0.0%
HRT Days Mean Velocity	days cm/sec	15.8 0.34	15.5 0.34	11.6 0.46										13.7 0.38
Seepage Outflow / Total Outflow Release 1 Outflow Volume	% hm3/yr	0% 0.0	0% 0.0	0% 0.0										0% 0.0
Release 2 Outflow Volume 95th Percentile Outflow Volume	hm3/yr hm3/d	0.0	0.0	0.0 2.18										0.0
Sim Percentile Outflow Load Simulated / Specified Mean Depth Release 1 Demand Met	кg/d %	33.24 #N/A #N/A	41.53 #N/A #N/A	72.33 #N/A #N/A										146.5 #N/A #N/A
Release 2 Demand Met Outflow Demand Met	%	#N/A #N/A	#N/A #N/A	#N/A #N/A										#N/A #N/A
Range Check - Mean Depth Range Check - Freq Depth < 10 cm	-	-	-	-										0
Range Check - Flow/Width Range Check - Inflow Conc	1	1	÷	1										0 0
Range Check - Outflow Conc Water Balance Error	- %	0.00%	0.00%	0.00%										0.00%
Mass Balance Error Warning or Error Messages	%	0.02%	0.02%	0.16%										0.09%

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#### Table A.10 STA-2: Case "2006 Base"

DMSTA2- Inputs & Outpu	uts		Project:	PROJECT_ST	TA2							Mode	el Release: rrent Date:	7/1/2005
Input Variable	Units	Value 2006 Base	Case Descrip	ntion:	e Cell 4							ea	nent Date.	00/03/03
Input Series Name Starting Date for Simulation	-	TS_2006Base 05/01/65	Inflow time s Analysis for	series includes WY 1966-2000	allowance of 3 following add	8 cfs (27,50 ition of Cell	00 ac-ft/yr) s 4	eepage fro	m WCA-2A	to Supply C	anal			
Ending Date for Simulation Starting Date for Output	1	04/30/00 05/01/65	Cell 4 data t	aken from May	2005 BODR b	y Brown & (	Caldwell; ne	t cell area	1,900 acres					
Integration Steps Per Day Number of Iterations	1	4 0	Simulation Ty Output Variat	pe: <u>ple</u>	Uncertainty Ar Mean	halysis Lower CL	Upper CL		Diagnostic	<u>s</u>				-
Output Averaging Interval Inflow Conc Scale Factor	days -	7 1	FWM Outflow GM Outflow 0	v C (ppb) C (ppb)	21.0 15.0	25.7 19.7	17.1 11.2		H20 Balan Mass Bala	ce Error Me nce Error M	an & Max ean & Max	0.0% 0.1%	0.0% 0.1%	
Rainfall P Conc Atmospheric P Load (Dry)	ppb mg/m2-yr	10 20	Load Reducti Bypass Load	ion % (%)	79% 0.0%	75%	83%	_	Iterations & Warning/E	& Converger rror Messag	nce jes	2	0.6%	
Cell Label		1 1 PEW 3	2 2 PEW 3	3 3 SAV 3	4 4 SAV 3	5	6	- 1	8	9	10	11	12	
Inflow Fraction		0.165	0.21	0.365	0.26									
Surface Area Mean Width of Flow Path	km2 km	7.28 1.58	9.19 2.00	9.19 2.00	7.70 2.50									
Number of Tanks in Series Minimum Depth for Releases	- cm	3.0	3.0	6.0	3.0									
Release 1 Series Name Release 2 Series Name														
Outflow Series Name Depth Series Name		40	40		40			-						
Outflow Control Depth Outflow Weir Depth Outflow Coefficient - Exponent	cm	40	40	4	42									
Outflow Coefficient - Intercept Bypass Depth	- cm	1	1	1	1									
Maximum Inflow Maximum Outflow	hm3/day hm3/day													
Inflow Seepage Rate Inflow Seepage Control Elev	(cm/d) / cm cm	0.008 76			0.004 67									
Inflow Seepage Conc Outflow Seepage Rate	ppb (cm/d) / cm	20 0.004	0.006	0.00337	20 0.0037									
Max Outflow Seepage Control Elev Max Outflow Seepage Conc Seepage Registe to Cell Number	ppb	-61 20	20	-30	20									
Seepage Recycle to Cell Number Seepage Recycle Fraction Seepage Discharge Fraction	-	1	1	1	0.78									
Initial Water Column Conc Initial P Storage Per Unit Area	ppb mg/m2	30 500	30 500	30 500	30 500									
Initial Water Column Depth C0 = Conc at 0 g/m2 P Storage	cm ppb	200	200 3	200 3	200 3									
C1 = Conc at 1 g/m2 P storage C2 = Conc at Half-Max Uptake	ppb ppb	22 300	22 300	22 300	22 300									
K = Net Settling Rate at Steady State Z1 = Saturated Uptake Depth	m/yr cm	34.9 40	34.9 40	52.5 40	52.5 40									
22 = Lower Penalty Depth 23 = Upper Penalty Depth	cm cm	100 200	100 200	100 200	100 200									
Output Variables	Units	1	2 3.80	3	4	5	6	7	8	9	10	11	12	Overall 5.09
Run Date Starting Date for Simulation	-	08/03/05	08/03/05	08/03/05	08/03/05 05/01/65									08/03/05 05/01/65
Starting Date for Output Ending Date	1	05/01/65 04/30/00	05/01/65 04/30/00	05/01/65 04/30/00	05/01/65 04/30/00									05/01/65 04/30/00
Output Duration Cell Label	days	12784 1	12784 2	12784 3	12784 4									12784 Total
Downstream Cell Label Network Simulation Name	-	Outflow	Outflow	Outflow	Outflow									- none
Simulation Type Surface Area	- km2	7.28	9.19	Uncerta 9.19	Uncerta 7.70									Uncerta 33.36
Mean ET Cell Inflow Volume	cm/yr cm/yr	130.3	130.3	130.3	130.3									130.3
Cell Inflow Load Cell Inflow Conc	kg/yr ppb	7148	9098 102.2	15813 102.2	11264 102.2									43323 102.2
Treated Outflow Volume Treated Outflow Load	hm3/yr kg/yr	74.5 1527	88.8 1860	154.5 3257	110.8 2341									428.7 8985
Treated FWM Outflow Conc Upper Confidence Limit	ppb ppb	20.5 26.0	20.9 26.4	21.1 25.7	21.1 25.1									21.0 25.7
Lower Confidence Limit Total Outflow Volume + Bypass	ppb hm3/yr	16.2 74.5	16.6 88.8	17.4 154.5	17.9 110.8									17.1 428.7
Total FWM Outflow Conc Bypass Load	ppb kg/yr	20.5	20.9	21.1	2341									21.0
Bypass Load Maximum Inflow	% hm3/d	0.0	0.0 1.74	0.0 3.03	0.0 2.16									0.0 8.30
Maximum Outflow Surface Load Reduction	hm3/d kg/yr	1.39 5621	1.76 7238	3.13 12556	2.23 8923									8.52 34338
Load Trapped in Sediments Overall Load Reduction	kg/yr %	5817 79%	7283 80%	12663 79%	9135 79%									34898 79%
Lower Confidence Limit Upper Confidence Limit	%	73% 83%	74% 84%	75% 83%	75% 82%									75% 83%
Outflow Geo Mean - Composites	ppb ppb	15.2	15.2 16.0	13.4 14.1	13.7									#N/A 15.0
Lower Confidence Limit Frequency Outflow Conc > 10 ppb	ppb %	11.4	11.79	10.5	11.4									11.2
Frequency Outflow Conc > 20 ppb Frequency Outflow Conc > 50 ppb	%	17% 0%	20% 0%	18% 0%	18% 0%									45% 17%
Freq Outflow Volume > 10 ppb 95th Percentile Outflow Conc	% ppb	99% 25	99% 26	97% 27	97% 27									98% 26
Mean Biomass P Storage Storage Increase / Net Removal	mg/m2 %	1204 0%	1194 0%	1380 0%	1189 0%									1246 0%
Net Storage Turnover Rate Unit Area P Removal	1/yr g/m2-yr	23.2 799	23.2 793	34.9 1378	34.9 1187									29.4 1046
Max Water Load Max Water Load Mean Depth	cm/d	18.8 55	19.0 53	4.6 33.0 68	28.0 54									24.9 58
Minimum Depth Maximum Depth	cm	40 97	27 97	57 112	40 97									41 101
Frequency Depth < 10 cm Flow/Width	% m2/day	0.0% 121	0.0% 122	0.0% 212	0.0% 121									0.0% 146.2
HRT Days Mean Velocity	days cm/sec	20.7 0.26	20.2 0.26	14.8 0.36	13.8 0.26									16.6 0.29
Seepage Outflow / Total Outflow Release 1 Outflow Volume	% hm3/yr	0%	0%	0%	1% 0.0									0%
Release 2 Outflow Volume 95th Percentile Outflow Volume	hm3/yr hm3/d	0.0	0.0	0.0	0.0									0.0 4.4
Simulated / Specified Mean Depth	кg/а %	17.83 #N/A #N/A	22.51 #N/A #N/A	42.69 #N/A #N/A	30.29 #N/A #N/A									#N/A #N/A
Release 2 Demand Met Outflow Demand Met	%	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A									#N/A #N/A
Range Check - Mean Depth Range Check - Freq Depth < 10 cm	-	1	-	-	0.87									1
Range Check - Flow/Width Range Check - Inflow Conc	1	1	1	1	0.75									1 0
Range Check - Outflow Conc Water Balance Error	~	0.00%	0.00%	0.00%	0.00%									0.00%
Mass Balance Error Warning or Error Messages	%	0.02% Cell# 4 4 Depth o	0.02% out of calib. range	0.12% for SAV_3: 54 vs	0.03%						-			2
		Cell# 4 4 Flow/W	run out of calib. n	ange for SAV_3:	121 VS. 162 - 374	m2/day								

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DMSTA2- Inputs & Outpu	lts	Value	Project:	PROJECT_S	FA34							Mode	el Release: rrent Date:	7/1/2005 08/03/05
Input Variable Design Case Name Input Series Name	- -	Value ST3_06Base TS_ST306Base	STA-3/4; Ce Inflows limit	ells 1A and 1B ed to those from	only n North New F	River Canal a	at G-370							
Starting Date for Simulation Ending Date for Simulation	1	05/01/65 04/30/00	Includes S-2 Lake water	2/S-7 Basin run supply releases	off and Lake fl to low WCA e	ow-through	releases at m treatment	S-351 t area inflo	ws.					
Starting Date for Output Integration Steps Per Day	1	05/01/65 4	Simulation Ty	pe:	Uncertainty A	nalysis								I
Number of Iterations Output Averaging Interval	- days	0 7	Output Varial FWM Outflow	ole v C (ppb)	Mean 25.3	Lower CL 31.4	Upper CL 19.9		Diagnostic H20 Balan	s ce Error Me	an & Max	0.0%	0.0%	
Rainfall P Conc Atmospheric B Lood (Dp)	ppb	10	Load Reducti	(ppb) on %	69% 0.0%	61%	75%		Iterations &	Converge	nce	0.1% 3	0.1%	
Cell Number> Cell Label	-	1 1A	2 1B	3	4	5	6	7	8	9 9	10	11	12	1
Vegetation Type Inflow Fraction	>	EMG_3 1	PEW_3											
Downstream Cell Number Surface Area	- km2	2 12.30	14.12											
Mean Width of Flow Path Number of Tanks in Series	km -	3.42 6.0	4.50 3.0											
Minimum Depth for Releases Release 1 Series Name Release 2 Series Name	cm													
Outflow Series Name Depth Series Name														
Outflow Control Depth Outflow Weir Depth	cm cm	60	60											
Outflow Coefficient - Exponent Outflow Coefficient - Intercept	1	4 1	4 1											
Bypass Depth Maximum Inflow	cm hm3/day													
Inflow Seepage Rate	(cm/d) / cm													
Inflow Seepage Conc Outflow Seepage Rate	ppb (cm/d) / cm	0.0058	0.0029											
Outflow Seepage Control Elev Max Outflow Seepage Conc	cm ppb	16 20	40 20											
Seepage Recycle to Cell Number Seepage Recycle Fraction	-	1 0.5	1 0.5											
Seepage Discharge Fraction Initial Water Column Conc	- ppb	30	30											
Initial Water Column Depth	mg/m2 cm	200	200											
C1 = Conc at 1 g/m2 P storage C2 = Conc at 1 g/m2 P storage	ppb	22 300	22 300											
K = Net Settling Rate at Steady State Z1 = Saturated Uptake Depth	m/yr cm	16.8 40	34.9 40											
Z2 = Lower Penalty Depth Z3 = Upper Penalty Depth	cm	100 200	100 200											
Output Variables	Units	1	2	3	4	5	6	7	8	9	10	11	12	Overall
Execution Time Run Date	sec/yr -	3.60 08/03/05	4.06 08/03/05											4.06 08/03/05
Starting Date for Simulation Starting Date for Output	-	05/01/65	05/01/65											05/01/65
Output Duration Cell Label	days	12784 1A	12784 1B											12784 Total
Downstream Cell Label Network Simulation Name	-	1B none	Outflow none											- none
Simulation Type Surface Area	- km2	Uncerta 12.30	Uncerta 14.12											Uncerta 26.41
Mean Rainfall Mean ET	cm/yr cm/yr	130.0 134.9	130.0 134.9											130.0 134.9
Cell Inflow Conc	kg/yr	27893	18656 53.4											27893 78.6
Treated Outflow Volume Treated Outflow Load	hm3/yr kg/yr	349.5 18656	345.0 8722											345.0 8722
Treated FWM Outflow Conc Upper Confidence Limit	ppb ppb	53.4 57.9	25.3 31.4											25.3 31.4
Lower Confidence Limit Total Outflow Volume + Bypass	ppb hm3/yr	48.2 349.5	19.9 345.0											19.9 345.0
Total Outflow Load + Bypass Total FWM Outflow Conc	kg/yr ppb	18656 53.4	25.3											8721.9 25.3
Bypass Load Bypass Load Maximum Inflow	% ////////////////////////////////////	0.0	0.0											0.0
Maximum Outflow Surface Load Reduction	hm3/d kg/yr	4.90 9237	5.27 9934											5.27 19171
Load Trapped in Sediments Overall Load Reduction	kg/yr %	8996 33%	10284 53%											19280 69%
Lower Confidence Limit Upper Confidence Limit	%	27% 40%	47% 59%											61% 75%
Outflow Geo Mean - Composites	ppb	40.1	21.2											21.2 27.3
Lower Confidence Limit Frequency Outflow Conc > 10 ppb	ppb %	41.3 100%	16.01											16.0 99%
Frequency Outflow Conc > 20 ppb Frequency Outflow Conc > 50 ppb	% %	100% 37%	62% 0%											89% 62%
Freq Outflow Volume > 10 ppb 95th Percentile Outflow Conc	% ppb	100% 60	100% 32											100% 32
Mean Biomass P Storage Storage Increase / Net Removal	mg/m2 %	2297 0%	1098											1656 0%
Unit Area P Removal Mean Water Load	g/m2-yr	732	729 6.8											730
Max Water Load Mean Depth	cm/d cm	40.3 68	34.7 65											18.8 67
Minimum Depth Maximum Depth	cm cm	27 109	21 103											24 106
Frequency Depth < 10 cm Flow/Width	% m2/day	0.0% 284	0.0% 213											0.0% 246.0
HRT Days Mean Velocity	days cm/sec	8.6 0.48	9.6 0.38											18.1 0.43
Release 1 Outflow Volume	hm3/yr	0.0	0.0											0.0
95th Percentile Outflow Volume 95th Percentile Outflow Load	hm3/d	3.26 186.57	3.18											3.2 94.2
Simulated / Specified Mean Depth Release 1 Demand Met	%	#N/A #N/A	#N/A #N/A											#N/A #N/A
Release 2 Demand Met Outflow Demand Met	%	#N/A #N/A	#N/A #N/A											#N/A #N/A
Range Check - Mean Depth Range Check - Freq Depth < 10 cm	1	1	1											0
Range Check - Flow/Width Range Check - Inflow Conc Range Check - Outflow Conc	1	1.35	-											1
Water Balance Error Mass Balance Error	- % %	0.00%	0.00%											0.00%
Warning or Error Messages	70	Cell# 1 1A Flow/	Width out of calib.	range for EMG_3	284 vs. 26 - 210	) m2/day								1

# Table A.11: STA-3/4: Cells 1A and 1B: Case "ST3\_06Base"

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# Table A.12 STA-3/4: Cells 2 &4: Case "ST4\_06Base"

DMSTA2- Inputs & Outpu	uts		Project:	PROJECT_S	TA34							Mode Cu	rrent Date:	7/1/2005 08/03/05
Input Variable Design Case Name Input Series Name	- -	Value ST4_06Base	STA-3/4; Ce	tion: Ils 1A and 1B and to those from	excluded	at G-372								1
Starting Date for Simulation	1	05/01/65	Includes S-3	3/S-8, SSDD, S supply releases	FCD and G136 s to low WCA a	SO Basin nd to Big C	runoff and L vpress Rese	ake flow-ti ervation ex	nrough relea	ses at S-35 treatment a	4 area inflows.			
Starting Date for Output Integration Steps Per Day	1	05/01/65 4	Simulation Ty	pe:	Uncertainty Ar	nalysis	,							
Number of Iterations Output Averaging Interval	- days	0 7	Output Variat FWM Outflow	<u>ole</u> / C (ppb)	Mean 18.2	Lower CL 22.1	Upper CL 15.1		Diagnostic: H20 Balan	<u>s</u> ce Error Me	an & Max	0.0%	0.0%	
Inflow Conc Scale Factor Rainfall P Conc	ppb	1 10	GM Outflow C Load Reducti	C (ppb) on %	13.2 79%	16.8 75%	10.3 83%		Mass Bala Iterations &	nce Error M Converge	lean & Max nce	0.1%	0.1% 0.0%	
Atmospheric P Load (Dry) Cell Number>	mg/m2-yr	20	Bypass Load 2	(%) 3	0.0% 4	5	6	7	Warning/E 8	rror Messag 9	jes 10	4	12	
Vegetation Type	>	EMG_3 0.54	SAV_3	3A EMG_3 0.46	3B SAV_3									
Downstream Cell Number Surface Area	- km2	2 10.29	11.71	4 9.61	8.92									
Mean Width of Flow Path Number of Tanks in Series	km -	2.89 6.0	4.02 3.0	4.88 4.0	4.88 4.0									
Minimum Depth for Releases Release 1 Series Name	cm													
Release 2 Series Name Outflow Series Name														
Outflow Control Depth Outflow Weir Depth	cm	60	60	60	60									
Outflow Coefficient - Exponent Outflow Coefficient - Intercept	-	4	4	4	4									
Bypass Depth Maximum Inflow	cm hm3/day													
Maximum Outflow Inflow Seepage Rate	hm3/day (cm/d) / cm													
Inflow Seepage Control Elev Inflow Seepage Conc	ppb	0.0014		0.0028										
Outflow Seepage Control Elev Max Outflow Seepage Conc	cm ppb	-67 20		-64 20										
Seepage Recycle to Cell Number Seepage Recycle Fraction	-	1 0.5		3 0.5										
Seepage Discharge Fraction Initial Water Column Conc	- ppb	30	30	30	30				+					
Initial P Storage Per Unit Area Initial Water Column Depth	mg/m2 cm	500 200	500 200	500 200	500 200									
CU = Conc at 0 g/m2 P Storage C1 = Conc at 1 g/m2 P storage C2 = Conc at Half-Max Untako	ppb ppb	3 22 300	3 22 300	3 22 300	3 22 300									
K = Net Settling Rate at Steady State	m/yr	16.8	52.5 40	16.8	52.5 40									
Z2 = Lower Penalty Depth Z3 = Upper Penalty Depth	cm	100 200	100 200	100 200	100 200									
Output Variables	Units	1	2	3	4	5	6	7	. 8	9	10	11	12	Overall
Execution Time Run Date	sec/yr -	6.20 08/03/05	6.71 08/03/05	7.31 08/03/05	7.89 08/03/05									7.89 08/03/05
Starting Date for Simulation Starting Date for Output	÷	05/01/65	05/01/65	05/01/65	05/01/65									05/01/65
Output Duration Cell Label	days	12784 2A	12784 2B	12784 3A	12784 3B									12784 Total
Downstream Cell Label Network Simulation Name	-	2B none	Outflow none	3B none	Outflow none									- none
Simulation Type Surface Area	km2	Uncerta 10.29	Uncerta 11.71	Uncerta 9.61	Uncerta 8.92									Uncerta 40.53
Mean Rainfall Mean ET Coll Inflow Volumo	cm/yr cm/yr	130.0 134.9	130.0 134.9	130.0 134.9 201.6	130.0 134.9									130.0 134.9
Cell Inflow Load Cell Inflow Load	kg/yr	20004	12295 52.9	17040	9847 51.1									436.2 37044 84.5
Treated Outflow Volume Treated Outflow Load	hm3/yr kg/yr	232.6 12295	232.1 4179	192.9 9847	192.4 3563									424.5 7741
Treated FWM Outflow Conc Upper Confidence Limit	ppb ppb	52.9 58.4	18.0 21.8	51.1 56.5	18.5 22.4									18.2 22.1
Lower Confidence Limit Total Outflow Volume + Bypass	ppb hm3/yr	46.8 232.6	14.9 232.1	45.2 192.9	15.4 192.4									15.1 424.5
Total Outflow Load + Bypass Total FWM Outflow Conc Bypass Load	kg/yr ppb kg/yr	52.9	4179 18.0	9847 51.1	18.5									18.2 0.0
Bypass Load Maximum Inflow	% hm3/d	0.0 5.02	0.0 4.95	0.0 4.28	0.0 4.22									0.0 9.31
Maximum Outflow Surface Load Reduction	hm3/d kg/yr	4.95 7709	4.97 8116	4.22 7194	4.29 6284									9.26 29303
Load Trapped in Sediments Overall Load Reduction	kg/yr %	7690 39%	8500 66%	6757 42%	6570 64%									29517 79%
Lower Confidence Limit Upper Confidence Limit	%	32%	63% 68%	36% 49%	60% 66%									75% 83%
Outflow Geo Mean - Composites	ppb	44.2 45.2 51.4	13.1	42.0 46.1 51.9	13.5									13.2 16.8
Lower Confidence Limit Frequency Outflow Conc > 10 ppb	ppb %	38.6 100%	10.25 74%	39.9 100%	10.6 76%									10.3 74%
Frequency Outflow Conc > 20 ppb Frequency Outflow Conc > 50 ppb	%	100% 27%	13% 0%	100% 31%	16% 0%									41% 14%
Freq Outflow Volume > 10 ppb 95th Percentile Outflow Conc	% ppb	100% 59	92% 23	100% 59	92% 24									92% 24
Storage Increase / Net Removal	mg/m∠ % 1/vr	2347 0%	0%	2207 0%	738 0% 34.9									1491 0% 17.1
Unit Area P Removal Mean Water Load	g/m2-yr cm/d	748	726	703	737									728
Max Water Load Mean Depth	cm/d cm	48.8 66	42.2 63	44.5 59	47.3 60									23.0 62
Minimum Depth Maximum Depth	cm cm	27 114	7 105	9 96	2 97									12 103
Frequency Depth < 10 cm Flow/Width	% m2/day	0.0%	0.1%	0.1%	0.2%									0.1% 153.3
Mean Velocity Seepage Outflow / Total Outflow	cm/sec	0.39	0.29	0.22	0.21									0.28
Release 1 Outflow Volume Release 2 Outflow Volume	hm3/yr hm3/yr	0.0	0.0	0.0	0.0									0.0
95th Percentile Outflow Volume 95th Percentile Outflow Load	hm3/d kg/d	2.30 128.67	2.27 47.27	1.95 107.14	1.97 42.03									4.2 89.5
Simulated / Specified Mean Depth Release 1 Demand Met	% %	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A									#N/A #N/A
Release 2 Demand Met Outflow Demand Met	%	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A									#N/A #N/A
Range Check - Mean Depth Range Check - Freq Depth < 10 cm Range Check - Flow/Width	÷	- 1.07	-	-	0.97									0
Range Check - Inflow Conc Range Check - Outflow Conc	-	-	-	-	-									0
Water Balance Error Mass Balance Error	%	0.00% 0.12%	0.00% 0.02%	0.00% 0.06%	0.00% 0.08%									0.00% 0.12%
Warning or Error Messages		Cell# 1 2A Flow/ Cell# 2 2B Flow/	Width out of calib. Width out of calib.	range for EMG_3: range for SAV_3:	224 vs. 26 - 210 158 vs. 162 - 374	m2/day Im2/day								4
		Cell# 4 3B Depth Cell# 4 3B Flow/	out of calib. range Width out of calib.	a for SAV_3: 60 range for SAV_3:	/s. 62 - 87 cm 108 vs. 162 - 374	l m2/day								

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# Table A.13 STA-3/4: Case "2006 Base"

DMSTA2- Inputs & Outpu	uts		Project:	PROJECT_ST	FA34							Mode Cui	I Release: rent Date:	7/1/2005 08/03/05
Input Variable Design Case Name Input Series Name	- -	2006 Base TS All06Base	Case Descrip STA-3/4, all All inflows	tion: cells										
Starting Date for Simulation Ending Date for Simulation	-	05/01/65 04/30/00	Lake water	supply releases	to low WCA a	and to Big C	ypress Rese	ervation ex	cluded from	treatment a	area inflows.			
Starting Date for Output Integration Steps Per Day	1	05/01/65 4	Simulation Ty	pe:	Uncertainty A	nalysis								
Number of Iterations Output Averaging Interval Inflow Conc Scale Factor	days	0 7	FWM Outflow	C (ppb)	20.1 15.6	24.8 20.1	16.2 11 9		H20 Balan Mass Balan	s ce Error Me	an & Max	0.0%	0.0%	
Rainfall P Conc Atmospheric P Load (Dry)	ppb mg/m2-yr	10 20	Load Reducti Bypass Load	on % (%)	76%	71%	81%		Iterations & Warning/E	Converge	nce les	3	0.0%	
Cell Number> Cell Label	-	1 1A	2 1B	3 2A	<b>4</b> 2B	5 3A	6 3B	7	8	9	10	11	12	1
Vegetation Type Inflow Fraction Downstroom Coll Number	>	EMG_3 0.34	PEW_3	EMG_3 0.36	SAV_3	EMG_3 0.3	SAV_3							
Surface Area Mean Width of Flow Path	km2 km	12.30 3.42	14.12 4.50	10.29 2.89	11.71 4.02	9.61 4.88	8.92 4.88							
Number of Tanks in Series Minimum Depth for Releases	- cm	6.0	3.0	6.0	3.0	4.0	4.0							
Release 1 Series Name Release 2 Series Name Outflow Series Name														
Depth Series Name Outflow Control Depth	cm	60	60	60	60	60	60	-						
Outflow Weir Depth Outflow Coefficient - Exponent	cm -	4	4	4	4	4	4							
Bypass Depth Maximum Inflow	- cm hm3/day	1	1	1	1	1	1							
Maximum Outflow Inflow Seepage Rate	hm3/day (cm/d) / cm													
Inflow Seepage Control Elev Inflow Seepage Conc	cm ppb	0.0058	0.0020	0.0014		0.0020								
Outflow Seepage Control Elev Max Outflow Seepage Conc	cm ppb	16 20	40 20	-67 20		-64 20								
Seepage Recycle to Cell Number Seepage Recycle Fraction	-	1 0.5	1 0.5	3 0.5		3 0.5								
Seepage Discharge Fraction Initial Water Column Conc	- ppb	30	30	30	30	30	30 500							
Initial Water Column Depth C0 = Conc at 0 g/m2 P Storage	cm ppb	200	200	200	200	200	200							
C1 = Conc at 1 g/m2 P storage C2 = Conc at Half-Max Uptake	ppb ppb	22 300	22 300	22 300	22 300	22 300	22 300							
<ul> <li>Net Settling Rate at Steady State</li> <li>Z1 = Saturated Uptake Depth</li> <li>Z2 = Lower Penalty Depth</li> </ul>	m/yr cm cm	16.8 40 100	34.9 40 100	16.8 40 100	52.5 40 100	16.8 40 100	52.5 40 100							
Z3 = Upper Penalty Depth	cm	200	200	200	200	200	200							
Output Variables Execution Time	Units sec/yr	1 8.31	2 8.80	3 9.60	4 10.09	5 10.69	6 11.29	7	8	9	10	11	12	Overall 11.29
Starting Date for Simulation Starting Date for Output	-	05/01/65	05/01/65	05/01/65	05/01/65	05/01/65	05/01/65							05/01/65
Ending Date Output Duration	- days	04/30/00 12784	04/30/00 12784	04/30/00 12784	04/30/00 12784	04/30/00 12784	04/30/00 12784							04/30/00 12784
Cell Label Downstream Cell Label Network Simulation Name		1A 1B	1B Outflow	2A 2B	2B Outflow	3A 3B	3B Outflow							Total -
Simulation Type Surface Area	- - km2	Uncerta 12.30	Uncerta 14.12	Uncerta 10.29	Uncerta 11.71	Uncerta 9.61	Uncerta 8.92							Uncerta 66.94
Mean Rainfall Mean ET	cm/yr cm/yr	130.0 134.9	130.0 134.9	130.0 134.9	130.0 134.9	130.0 134.8	130.0 134.9							130.0 134.9
Cell Inflow Load Cell Inflow Load Cell Inflow Conc	kg/yr	269.7 22079 81.9	264.3 13076 49.5	285.6 23377 81.9	289.6 15734 54.3	238.0 19481 81.9	221.2 11734 53.0							793.2 64937 81.9
Treated Outflow Volume Treated Outflow Load	hm3/yr kg/yr	264.3 13076	260.2 5266	289.6 15734	289.0 5783	221.2 11734	220.8 4412							770.0 15460
Treated FWM Outflow Conc Upper Confidence Limit	ppb ppb	49.5 55.0	20.2 25.8	54.3 59.1	20.0 24.3	53.0 58.1	20.0 24.3							20.1 24.8
Total Outflow Volume + Bypass Total Outflow Load + Bypass	hm3/yr kg/yr	43.4 264.3 13076	260.2 5266	48.9 289.6 15734	289.0 5783	221.2 11734	220.8 4412							770.0
Total FWM Outflow Conc Bypass Load	ppb kg/yr	49.5 0	20.2 0	54.3 0	20.0 0	53.0 0	20.0 0							20.1 0.0
Bypass Load Maximum Inflow Maximum Outflow	% hm3/d hm3/d	0.0 4.34 4.14	0.0 4.14 4.25	0.0 4.60 4.38	0.0 4.38 4.56	0.0 3.83 3.63	0.0 3.63 3.76							0.0 12.77 12.58
Surface Load Reduction Load Trapped in Sediments	kg/yr kg/yr	9003 8821	7810 8191	7644 7774	9951 10334	7748 6951	7322 7608							49477 49680
Overall Load Reduction Lower Confidence Limit	%	41% 34%	60% 54%	33% 27%	63% 59%	40% 34%	62% 58%							76% 71%
Daily Geometric Mean Outflow Geo Mean - Composites	ppb ppb	48% 42.2 43.0	14.0 16.4	47.1 48.0	13.1 15.6	48.4 49.0	12.0 15.5							#N/A 15.6
Upper Confidence Limit Lower Confidence Limit	ppb ppb	49.1 36.5	21.88 12.09	53.3 42.0	19.7 12.2	54.5 43.1	19.7 12.2							20.1 11.9
Frequency Outflow Conc > 10 ppb Frequency Outflow Conc > 20 ppb Frequency Outflow Conc > 50 ppb	%	100% 100%	93% 28% 0%	100% 100% 39%	89% 27% 0%	100% 100% 46%	87% 28% 0%							89% 58% 26%
Freq Outflow Volume > 10 ppb 95th Percentile Outflow Conc	% ppb	100% 55	98% 26	100% 60	97% 26	100% 60	95% 26							97% 26
Mean Biomass P Storage Storage Increase / Net Removal	mg/m2 %	2252 0%	874 0%	2372 0%	884 0%	2271 0%	854 0%							1557 0%
Net Storage Furnover Rate Unit Area P Removal Mean Water Load	g/m2-yr cm/d	717 6.0	23.2 580 5.1	755 7.6	34.9 883 6.8	11.1 723 6.8	34.9 853 6.8							742 3.2
Max Water Load Mean Depth	cm/d cm	35.3 66	29.3 63	44.7 70	37.4 65	39.8 58	40.7 61							19.1 64
Minimum Depth Maximum Depth Froquency Depth = 10 cm	cm cm	29 105	13 99	45 111	23 103	1 93 1.2%	5 94 0.1%							20 101 0.2%
Flow/Width HRT Days	70 m2/day days	216 11.0	161 12.2	271 9.2	197 9.7	134 8.6	124 9.0							185.4 19.8
Mean Velocity Seepage Outflow / Total Outflow	cm/sec %	0.38 2%	0.30 1%	0.45 1%	0.35 0%	0.26 4%	0.23 0%							0.33 3%
Release 1 Outflow Volume Release 2 Outflow Volume	hm3/yr hm3/yr	0.0	0.0	0.0	0.0	0.0	0.0 0.0							0.0 0.0 7.2
95th Percentile Outflow Volume 95th Percentile Outflow Load Simulated / Specified Mean Depth	kg/d %	129.98 #N/A	59.38 #N/A	153.31 #N/A	64.61 #N/A	120.40 #N/A	52.00 #N/A							176.1 #N/A
Release 1 Demand Met Release 2 Demand Met	%	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A							#N/A #N/A
Outflow Demand Met Range Check - Mean Depth Range Check - Free Depth < 10 cm	-	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A 0.99							#N/A
Range Check - Flow/Width Range Check - Flow/Width Range Check - Inflow Conc	-	1.03	-	1.29		-	0.77							3
Range Check - Outflow Conc Water Balance Error	- %	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%							0
Mass Balance Error Warning or Error Messages	%	Cell# 1 1A Flow/ Cell# 3 2A Flow/	U.U2% Width out of calib.	0.11% range for EMG_3: range for EMG_3:	0.02% 216 vs. 26 - 210 271 vs. 26 - 210	0.02% m2/day m2/day	0.06%							4
		Cell# 6 3B Depth Cell# 6 3B Flow/	out of calib. range Width out of calib.	for SAV_3: 61 v range for SAV_3:	s. 62 - 87 cm 124 vs. 162 - 37	4 m2/day								

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### Table A.14 STA-5: Case "Exist"

Input Variable	Units	Value	Case Descrip	tion:	2006							Cu	rrent Date:	08/04/0
Design Case Name Input Series Name Starting Date for Simulation	-	Exist TS_Exist	Existing STA Used Inflow	A-5, WY 2002-3 Volumes as fo	2005 r C-139 Basin	total to L-3,	reduced by	measured	bypas at G-	406	od by 10% f	or BMBe)		
Ending Date for Simulation Starting Date for Output	-	04/30/05	Used Histori	C ITHIOW CONCE	matauons (Incre	aseu CONCE	anuauons in	uata file b	y 1176; nad l	Seen (educ)	a by 10% h	u DiviPS)		
Integration Steps Per Day Number of Iterations	-	4	Simulation Typ Output Variab	oe: ole	Uncertainty An Mean	nalysis Lower CL	Upper CL		Diagnostic	5				•
Output Averaging Interval Inflow Conc Scale Factor	days -	7	FWM Outflow GM Outflow 0	C (ppb) (ppb)	66.5 62.3	82.1 77.5	51.6 47.7		H20 Balan Mass Balar	ce Error Me nce Error M	an & Max lean & Max	0.1% 0.0%	0.0% 0.0%	
Rainfall P Conc Atmospheric P Load (Dry)	ppb mg/m2-yr	10 20	Load Reducti Bypass Load	on % (%)	73% 0.0%	66%	79%		Iterations & Warning/E	k Converge	nce Jes	3	0.7%	
Cell Number> Cell Label	-	1 1A	2 1B	3 2A	4 2B	5	6	7	8	9	10	11	12	1
Vegetation Type Inflow Fraction	>	0.5	SAV_3	EMG_3 0.5	EMG_3									
Surface Area Mean Width of Flow Path	km2	3.38	4.94	3.38 1.56	4.94									
Number of Tanks in Series Minimum Depth for Releases	- cm	3.0	3.0	3.0	3.0									
Release 1 Series Name Release 2 Series Name														
Outflow Series Name Depth Series Name														
Outflow Control Depth Outflow Weir Depth	cm cm	40	60	40	60									
Outflow Coefficient - Exponent Outflow Coefficient - Intercept	-	4	4 1	4 1	4									
Bypass Depth Maximum Inflow Maximum Outflow	cm hm3/day bm3/day													
Inflow Seepage Rate	(cm/d) / cm cm													
Inflow Seepage Conc Outflow Seepage Rate	ppb (cm/d) / cm	0.0075	0.0075	0.0075	0.0075									
Outflow Seepage Control Elev Max Outflow Seepage Conc	cm ppb	-46 20	-38 20	-46 20	-38 20									
Seepage Recycle to Cell Number Seepage Recycle Fraction	1	1 1	1 1	3 1	3 1				1					
Seepage Discharge Fraction Initial Water Column Conc	ppb	30	30	30	30				1					
Initial P Storage Per Unit Area Initial Water Column Depth C0 = Conc at 0 g/m2 P Storage	cm	200	200	200	200	3	3							
C1 = Conc at 1 g/m2 P storage C2 = Conc at Half-Max Untake	ppb ppb	22 300	22 300	22 300	22 300	22 300	22 300							
K = Net Settling Rate at Steady State Z1 = Saturated Uptake Depth	m/yr cm	16.8 40	52.5 40	16.8 40	16.8 40	16.8 40	52.5 40							
Z2 = Lower Penalty Depth Z3 = Upper Penalty Depth	cm cm	100 200	100 200	100 200	100 200	100 200	100 200							
Output Variables	Units	1	2	3	4	5	6	7	8	9	10	11	12	Overall
Execution Time Run Date	sec/yr -	11.50 08/04/05	12.50 08/04/05	13.25 08/04/05	14.25 08/04/05									14.25 08/04/05
Starting Date for Simulation Starting Date for Output	1	05/01/01 05/01/01	05/01/01 05/01/01	05/01/01 05/01/01	05/01/01 05/01/01									05/01/01 05/01/01
Output Duration	days	1461 14	1461 18	1461	1461 2B									1461 Total
Downstream Cell Label		1B none	Outflow	2B Done	Outflow									-
Simulation Type Surface Area	- km2	Uncerta 3.38	Uncerta 4.94	Uncerta 3.38	Uncerta 4.94									Uncerta 16.63
Mean Rainfall Mean ET	cm/yr cm/yr	0.0 0.0	0.0 0.0	0.0 0.0	0.0 0.0									0.0 0.0
Cell Inflow Volume Cell Inflow Load	hm3/yr kg/yr	92.9 22545	107.0 16408	92.9 22545	107.0 16408									185.8 45089
Cell Inflow Conc Treated Outflow Volume	ppb hm3/yr	242.6	153.4 92.8	242.6	153.4 92.8									242.6 185.7
Treated Outflow Edad Treated FWM Outflow Conc	ppb	153.4	38.6	153.4	94.3 113.1									66.5 82.1
Lower Confidence Limit Total Outflow Volume + Bypass	ppb ppb hm3/vr	141.6	28.3	141.6	74.9									51.6
Total Outflow Load + Bypass Total FWM Outflow Conc	kg/yr ppb	16408 153.4	3580 38.6	16408 153.4	8758 94.3									12337.9 66.5
Bypass Load Bypass Load	kg/yr %	0 0.0	0 0.0	0 0.0	0 0.0									0.0 0.0
Maximum Inflow Maximum Outflow	hm3/d hm3/d	0.96	1.02 0.94	0.96	1.02 0.94									1.93 1.89
Surface Load Reduction Load Trapped in Sediments	kg/yr kg/yr	6137 5249 27%	12828 12051	6137 5249 27%	7650 6242									32751 28791 72%
Lower Confidence Limit	76 % %	23%	73%	23%	40%									66% 79%
Daily Geometric Mean Outflow Geo Mean - Composites	ppb ppb	139.5 139.6	33.5 34.5	139.5 139.6	88.8 89.8									#N/A 62.3
Upper Confidence Limit Lower Confidence Limit	ppb ppb	147.5 129.3	46.90 24.42	147.5 129.3	108.0 70.8									77.5 47.7
Frequency Outflow Conc > 10 ppb Frequency Outflow Conc > 20 ppb	% %	100% 100%	100% 100%	100% 100%	100% 100%									100% 100%
Frequency Outflow Conc > 50 ppb Freq Outflow Volume > 10 ppb	%	100% 100%	4% 100%	100% 100%	100% 100%									100%
95th Percentile Outflow Conc Mean Biomass P Storage Storage Increase (Not Removal	ppb mg/m2	184 4875 0%	48 2444 0%	184 4875 0%	106 3969 0%									3885
Net Storage Turnover Rate Unit Area P Removal	1/yr g/m2-yr	1.3 1553	4.0	1.3 1553	1.3 1264									1.8 1731
Mean Water Load Max Water Load	cm/d cm/d	7.5 28.5	5.9 20.6	7.5 28.5	5.9 20.6									3.1 11.6
Mean Depth Minimum Depth	cm cm	60 39	66 55	60 39	66 55									64 49
Maximum Depth Frequency Depth < 10 cm	cm %	90 0.0%	88 0.0%	90 0.0%	88 0.0%									89 0.0%
Flow/Width HRT Days	m2/day days	163 8.0	188 11.2	163 8.0	188 11.2									177.7 20.9
Mean Velocity Seepage Outflow / Total Outflow	cm/sec %	0.32	0.33	0.32	0.33									0.32
Release 2 Outflow Volume 95th Percentile Outflow Volume	hm3/yr hm3/d	0.0	0.0	0.0	0.0									0.0
95th Percentile Outflow Load Simulated / Specified Mean Depth	kg/d %	139.19 #N/A	31.58 #N/A	139.19 #N/A	71.65 #N/A									101.8 #N/A
Release 1 Demand Met Release 2 Demand Met	%	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A									#N/A #N/A
Outflow Demand Met Range Check - Mean Depth	%	#N/A	#N/A -	#N/A	#N/A									#N/A 0
Range Check - Freq Depth < 10 cm Range Check - Flow/Width	1	1	1	1	1									0 0
Range Check - Inflow Conc Range Check - Outflow Conc	-	-	1.00	-	-									1
Water Balance Error Mass Balance Error Water December 1	%	0.00%	0.00%	0.00%	0.00%	nob								0.09% 0.01%
warming or Error wessages		Cell# 2 1B Inflow	conc out of calib.	range for SAV_3:	153 vs. 15 - 153	add								1

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# Table A.15 STA-5: Case "Exist All Emerg"

DMSTA2- Inputs & Outpu	ıts		Project:	PROJECT_S	FA5							Mod	el Release:	7/1/2005
Input Variable	Units	Value	Case Descrip	tion:	2005							CL	Irrent Date:	08/04/05
Design Case Name Input Series Name Starting Date for Simulation	-	TS_Exist 06/01/01	Used Inflow	Volumes as fo	r C-139 Basin	total to L-3,	reduced by	recorded t	oypass at G-	406	ad by 10% fr	PMDc)		
Ending Date for Simulation	-	04/30/05	Used Histon	ic milow Conce	intrations (incre	Jased Conce	antrations in	data me b	y 11%, nau	been reduci	3d Dy 10% it	Di Divirs)		
Integration Steps Per Day	-	4	Simulation Ty	pe:	Uncertainty A	nalysis	Linner Cl		Disessetia	-				
Output Averaging Interval	days	7	FWM Outflow	v C (ppb)	94.3	113.1	74.9		H20 Balan	s ce Error Me	an & Max	0.1%	0.0%	
Rainfall P Conc	ppb	10	Load Reducti	ion %	61%	53%	69%		Iterations &	& Converge	nce	3	0.6%	
Cell Number>	mg/mz-yr	1	2 1 D	( <sup>76</sup> ) 3	4 2P	5	6	7	waning/≞ 8	9 9	10 10	11	12	
Vegetation Type	>	EMG_3	EMG_3	EMG_3	EMG_3									
Downstream Cell Number	-	2	4.94	4	4.94									
Mean Width of Flow Path	km	1.56	1.56	1.56	1.56									
Minimum Depth for Releases Release 1 Series Name	cm	0.0	0.0	0.0	0.0									
Release 2 Series Name														
Depth Series Name Outflow Control Depth	cm	40	60	40	60									
Outflow Weir Depth Outflow Coefficient - Exponent	cm	4	4	4	4									
Outflow Coefficient - Intercept Bypass Depth	- cm	1	1	1	1									
Maximum Inflow Maximum Outflow	hm3/day hm3/day													
Inflow Seepage Rate Inflow Seepage Control Elev	(cm/d) / cm cm													
Inflow Seepage Conc Outflow Seepage Rate	ppb (cm/d) / cm	0.0075	0.0075	0.0075	0.0075									
Outflow Seepage Control Elev Max Outflow Seepage Conc	cm ppb	-46 20	-38 20	-46 20	-38 20									
Seepage Recycle to Cell Number Seepage Recycle Fraction	1	1 1	1 1	3 1	3 1									
Seepage Discharge Fraction Initial Water Column Conc	ppb	30	30	30	30									
Initial P Storage Per Unit Area Initial Water Column Depth	mg/m2 cm	500 200	500 200	500 200	500 200									
C0 = Conc at 0 g/m2 P Storage C1 = Conc at 1 g/m2 P storage	ppb ppb	3 22	3 22	3 22	3 22	3 22	3 22							
C2 = Conc at Half-Max Uptake K = Net Settling Rate at Steady State	ppb m/yr	300 16.8	300 16.8	300 16.8	300 16.8	300 16.8	300 52.5							
Z1 = Saturated Uptake Depth Z2 = Lower Penalty Depth	cm cm	40 100	40 100	40 100	40 100	40 100	40 100							
Z3 = Upper Penalty Depth	cm	200	200	200	200	200	200			·				
Execution Time	Sec/yr	12.25	2 13.25	3 14.25	4	5	6	7	8	9	10	11	12	15.50
Starting Date for Simulation	-	05/01/01	05/01/01	05/01/01	05/01/01									05/01/01
Ending Date	-	04/30/05	04/30/05	05/01/01 04/30/05	04/30/05									05/01/01 04/30/05
Cell Label	days	1461 1A	1461 1B	1461 2A	1461 2B									Total
Network Simulation Name	-	none	none	none	none									none
Surface Area Moan Bainfall	km2	3.38	4.94	3.38	4.94									16.63
Mean ET Cell Inflow Volume	cm/yr	0.0	0.0	0.0	0.0									0.0
Cell Inflow Load	kg/yr	22545	16408	22545	16408									45089
Treated Outflow Volume Treated Outflow Load	hm3/yr kg/yr	107.0 16408	92.8 8758	107.0 16408	92.8 8758									185.7 17517
Treated FWM Outflow Conc Upper Confidence Limit	ppb	153.4 163.3	94.3 113.1	153.4 163.3	94.3 113.1									94.3 113.1
Lower Confidence Limit Total Outflow Volume + Bypass	ppb hm3/yr	141.6 107.0	74.9 92.8	141.6 107.0	74.9 92.8									74.9 185.7
Total Outflow Load + Bypass Total FWM Outflow Conc	kg/yr ppb	16408 153.4	8758 94.3	16408 153.4	8758 94.3									17516.6 94.3
Bypass Load Bypass Load	kg/yr %	0 0.0	0 0.0	0 0.0	0 0.0									0.0 0.0
Maximum Inflow Maximum Outflow	hm3/d hm3/d	0.96 1.02	1.02 0.94	0.96 1.02	1.02 0.94									1.93 1.89
Surface Load Reduction Load Trapped in Sediments	kg/yr kg/yr	6137 5249	7650 6242	6137 5249	7650 6242									27573 22982
Overall Load Reduction Lower Confidence Limit	% %	27% 23%	47% 40%	27% 23%	47% 40%									61% 53%
Upper Confidence Limit Daily Geometric Mean	% ppb	33% 139.5	54% 88.8	33% 139.5	54% 88.8									69% #N/A
Outflow Geo Mean - Composites Upper Confidence Limit	ppb ppb	139.6 147.5	89.8 107.98	139.6 147.5	89.8 108.0									89.8 108.0
Lower Contidence Limit Frequency Outflow Conc > 10 ppb	ppb %	129.3 100%	70.80	129.3 100%	70.8									70.8
Frequency Outflow Conc > 20 ppb Frequency Outflow Conc > 50 ppb	%	100%	100%	100%	100%									100%
95th Percentile Outflow Conc	ppb	184	106	184	106									106
Storage Increase / Net Removal Net Storage Turnover Rete	% 1/vr	0%	0%	0%	0%									0%
Unit Area P Removal Mean Water Load	g/m2-yr cm/d	1553	1264	1553	1264									1382 3.1
Max Water Load Mean Depth	cm/d cm	28.5	20.6	28.5 60	20.6									11.6 64
Minimum Depth Maximum Depth	cm	39 90	55 88	39 90	55 88									49 89
Frequency Depth < 10 cm Flow/Width	% m2/dav	0.0% 163	0.0% 188	0.0% 163	0.0% 188									0.0%
HRT Days Mean Velocity	days	8.0	11.2	8.0	11.2									20.9
Seepage Outflow / Total Outflow Release 1 Outflow Volume	% hm3/vr	0%	0%	0%	0%									0%
Release 2 Outflow Volume 95th Percentile Outflow Volume	hm3/yr hm3/d	0.0	0.0	0.0	0.0									0.0
95th Percentile Outflow Load Simulated / Specified Mean Depth	kg/d %	139.19 #N/A	71.65 #N/A	139.19 #N/A	71.65 #N/A									143.3 #N/A
Release 1 Demand Met Release 2 Demand Met	%	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A									#N/A #N/A
Outflow Demand Met Range Check - Mean Depth	%	#N/A -	#N/A -	#N/A -	#N/A -									#N/A 0
Range Check - Freq Depth < 10 cm Range Check - Flow/Width	1	-	1	1	1									0
Range Check - Inflow Conc Range Check - Outflow Conc	1	-	1	1	1									0
Water Balance Error Mass Balance Error	%	0.00%	0.00%	0.00%	0.00%									0.09% 0.01%
Warning or Error Messages														0
													1 22	

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#### Table A.16 STA-5: Case "2006 Base"

DMSTA2- Inputs & Outputs Project: PROJECT_STA5 Model Release: 7/												7/1/2005		
Input Variable	Units	Value	Case Descrip	tion:	EL 14/- 0		074.04		4.575				nem Date.	08/04/05
Design Case Name Input Series Name	-	2006 Base TS_Base	2006-2009;	ded to include	Flow-Way 3; ells considered	as SAV_3	o SIA-6 (ma	ax. daily inf	low 1,575 cl	s during pe	nod WY 199	95-2005)		
Ending Date for Simulation	-	04/30/05	Flow-Way 3	generally base	ns Reduced b d on April 200	5 Draft BOE	ngoing BiviP R; bypass o	directed to	STA-6	sin				
Integration Steps Per Day	-	4	Simulation Typ	De:	Uncertainty A	nalysis								
Output Averaging Interval	days	7	FWM Outflow	C (ppb)	22.1	29.6	16.7		H20 Balan	s ce Error Me	an & Max	0.0%	0.0%	
Rainfall P Conc	ppb	10	Load Reducti	on %	90%	23.2 86%	92%		Iterations &	Converge	ean & Max	0.0%	0.0%	
Cell Number>	mg/m2-yr	1	Bypass Load	(%) 3	0.0% 4	5	6	7	vvarning/E 8	9 9	10	4 11	12	
Vegetation Type	>	EMG_3	SAV_3	EMG_3	SAV_3	EMG_3	SAV_3							
Downstream Cell Number	-	2	4.04	4	4.04	6	0.74							
Mean Width of Flow Path	km	1.56	1.56	1.56	1.56	1.56	1.56							
Minimum Depth for Releases	cm	5.0	5.0	5.0	5.0	3.0	3.0							
Release 2 Series Name														
Depth Series Name Outflow Control Depth		40	60	40	60	40	60							
Outflow Weir Depth	cm	40	4	40	4	40	4							
Outflow Coefficient - Intercept Bypass Depth	- cm	1	1	1	1	1	1							
Maximum Inflow Maximum Outflow	hm3/day													
Inflow Seepage Rate Inflow Seepage Control Elev	(cm/d) / cm cm													
Inflow Seepage Conc Outflow Seepage Rate	ppb (cm/d) / cm	0.0075	0.0075			0.0075	0.0075							
Outflow Seepage Control Elev Max Outflow Seepage Conc	cm	-46 20	-38 20			20	20							
Seepage Recycle to Cell Number Seepage Recycle Fraction		1	2			5	6							
Seepage Discharge Fraction Initial Water Column Conc	- ppb	30	30	30	30	30	30							
Initial P Storage Per Unit Area Initial Water Column Depth	mg/m2 cm	500 200	500 200	500 200	500 200	500 200	500 200							
C0 = Conc at 0 g/m2 P Storage C1 = Conc at 1 g/m2 P storage	ppb ppb	3 22	3 22	3 22	3 22	3 22	3 22							
C2 = Conc at Half-Max Uptake K = Net Settling Rate at Steady State	ppb m/yr	300 16.8	300 52.5	300 16.8	300 52.5	300 16.8	300 52.5							
Z1 = Saturated Uptake Depth Z2 = Lower Penalty Depth	cm cm	40 100	40 100	40 100	40 100	40 100	40 100							
Z3 = Upper Penalty Depth	cm	200	200	200	200	200	200		<u> </u>					
Output Variables Execution Time	Units sec/yr	1 8.91	2 9.54	3 10.09	<b>4</b> 10.64	5 11.27	6 11.82	7	8	9	10	11	12	Overall 11.82
Run Date Starting Date for Simulation	1	08/04/05 05/01/94	08/04/05 05/01/94	08/04/05 05/01/94	08/04/05 05/01/94	08/04/05 05/01/94	08/04/05 05/01/94							08/04/05 05/01/94
Starting Date for Output Ending Date	1	05/01/94 04/30/05	05/01/94 04/30/05	05/01/94 04/30/05	05/01/94 04/30/05	05/01/94 04/30/05	05/01/94 04/30/05							05/01/94 04/30/05
Output Duration Cell Label	days	4018 1A	4018 1B	4018 2A	4018 2B	4018 3A	4018 3B							4018 Total
Downstream Cell Label Network Simulation Name	-	1B none	Outflow	2B none	Outflow	3B none	Outflow							- none
Simulation Type Surface Area	km2	3.38	4.94	3.38	Uncerta 4.94	4.61	3.71							24.95
Mean Raintail Mean ET	cm/yr cm/yr	82.1 82.0	82.1 82.0	82.1 82.0	82.1 82.0	82.1 82.0	82.1 82.0							82.1 82.0
Cell Inflow Load	kg/yr	13047	7696	13048	8283 126.6	13047	6259 105 8							39143
Treated Outflow Volume	hm3/yr	65.4	65.4	65.4	65.4	59.2	53.7							184.6
Treated FWM Outflow Conc	ppb	117.6	20.8	126.6	22.0	105.8	23.8							22.1
Lower Confidence Limit	ppb ppb	103.9	15.9	110.9	16.5	88.5	17.8							16.7
Total Outflow Load + Bypass	kg/yr	7696	1364	8283	1437	6259	1275							4075.4
Bypass Load	kg/yr	0	0	0	0	0	0							0.0
Maximum Inflow Maximum Outflow	hm3/d	1.11	1.03	1.11	1.03	1.11	0.99							3.32
Surface Load Reduction	kg/yr kg/yr	5351 4524	6332 6286	4765	6847 6986	6789 6096	4984 4892							35067 33644
Overall Load Reduction Lower Confidence Limit	%	41% 35%	82% 79%	37% 30%	83% 79%	52% 45%	80% 76%							90% 86%
Upper Confidence Limit Daily Geometric Mean	% ppb	48% 103.7	85% 14.2	44% 112.0	85% 14.8	60% 102.9	82% 16.4							92% #N/A
Outflow Geo Mean - Composites Upper Confidence Limit	ppb ppb	103.4 114.5	15.7 22.00	111.7 124.7	16.4 23.7	93.9 109.3	17.5 25.3							16.2 23.2
Lower Confidence Limit Frequency Outflow Conc > 10 ppb	ppb %	90.6 100%	11.07 90%	96.9 100%	11.3 91%	77.5 100%	12.1 93%							11.2 90%
Frequency Outflow Conc > 20 ppb Frequency Outflow Conc > 50 ppb	%	100% 100%	25% 0%	100% 100%	30% 0%	100% 100%	37% 0%							60% 29%
Freq Outflow Volume > 10 ppb 95th Percentile Outflow Conc	% ppb	100% 156	96% 29	100% 169	96% 31	100% 140	98% 34							97% 30
Mean Biomass P Storage Storage Increase / Net Removal	mg/m2 %	4203 0%	1275 0%	4515 0%	1417 0%	4152 0%	1321 0%							2677 0%
Unit Area P Removal	g/m2-yr	3.5 1339	11.0	3.5 1438	11.0	3.5 1322	1319							5.5 1348
Max Water Load Max Dooth	cm/d	32.7	20.9	32.7	20.9	24.0	4.4 26.6							13.3
Minimum Depth Maximum Depth	cm	24	44	24	44	10	8							27
Frequency Depth < 10 cm	%	0.0%	0.0%	0.0%	0.0%	0.2%	0.7%							0.1%
HRT Days	days	9.9	17.3	9.9	17.3	12.8	12.4							26.2
Seepage Outflow / Total Outflow	6m26r	0.25	0%	0.25	0.21	10%	9%							6% 0.0
Release 2 Outflow Volume	hm3/yr	0.0	0.0	0.0	0.0	0.0	0.0							0.0
95th Percentile Outflow Load	kg/d	91.25 #NI/A	16.91 #N/A	96.31 #N/A	17.77	74.04	17.33							51.7 #N/A
Release 1 Demand Met	%	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A							#N/A #N/A
Outflow Demand Met Range Check - Mean Depth	%	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A							#N/A
Range Check - Freq Depth < 10 cm Range Check - Flow/Width	-	1	- 0.71	-	- 0.71	1	- 0.64							0
Range Check - Inflow Conc Range Check - Outflow Conc	1	-	-	:	-	1								0
Water Balance Error Mass Balance Error	%	0.00%	0.00%	0.00%	0.00%	0.00% 0.00%	0.00%							0.00%
Warning or Error Messages		Cell# 2 1B Flow/ Cell# 4 2B Flow/	Width out of calib. Width out of calib.	range for SAV_3: range for SAV_3:	115 vs. 162 - 37 115 vs. 162 - 37	4 m2/day 4 m2/day								4
		Cell# 6 3B Depth Cell# 6 3B Flow/	out of calib. range Width out of calib.	for SAV_3: 54 v range for SAV_3:	rs. 62 - 87 cm 104 vs. 162 - 37	4 m2/day								

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# Table A.17 STA-5: Case "2006 Base Emg"

nput Variable Design Case Name	Units -	Value 2006 Base Em	Case Descrip STA-5 expa	nded to include	Flow-Way 3;	no bypass t	o STA-6 (ma	ax. daily in	flow 1,575 cl	s during pe	riod WY 199	95-2005)		00/0
nput Series Name Starting Date for Simulation	-	TS_Base 05/01/94	2006-2009; Historic Inflo	downstream ce ow Concentratio	ells considered	as SAV_3 y 10% for o	ngoing BMP	· implemer	ntation in bas	sin				
Ending Date for Simulation Starting Date for Output	1	04/30/05 05/01/94	Flow-Way 3	generally base	d on April 200	5 Draft BOD	OR; bypass o	directed to	STA-6					
Number of Iterations	-	4	Output Variat	pe: <u>ple</u>	Mean F7.0	Lower CL	Upper CL		Diagnostic	<u>s</u>		0.0%	0.0%	
nflow Conc Scale Factor	days -	1	GM Outflow C	C (ppb) C (ppb)	50.5	74.3 66.7	41.9 35.9		Mass Bala	nce Error Me	lean & Max	0.0%	0.0%	
Atmospheric P Load (Dry)	mg/m2-yr	20	Bypass Load	(%) 3	0.0%	5	60%	7	Warning/E	rror Messa	jes 10	0	12	
Cell Label /egetation Type	>	1A EMG 3	1B EMG 3	2A EMG 3	2B EMG 3	3A EMG 3	3B EMG 3			<i>,</i>			12	
nflow Fraction Downstream Cell Number	-	0.33333		0.33334		0.33333								
Surface Area Mean Width of Flow Path	km2 km	3.38 1.56	4.94 1.56	3.38 1.56	4.94 1.56	4.61 1.56	3.71 1.56							
lumber of Tanks in Series /inimum Depth for Releases	- cm	3.0	3.0	3.0	3.0	3.0	3.0							
Release 1 Series Name Release 2 Series Name														
Outflow Series Name Depth Series Name														
Dutflow Control Depth Dutflow Weir Depth	cm cm	40	60	40	60	40	60							
Dutflow Coefficient - Exponent Dutflow Coefficient - Intercept	-	4 1	4 1	4 1	4 1	4	4 1							
Aximum Inflow	cm hm3/day													
nflow Seepage Rate	(cm/d) / cm													
nflow Seepage Conc Nutflow Seepage Rate	ppb (cm/d) / cm	0.0075	0.0075			0.0075	0.0075							
Outflow Seepage Control Elev	cm	-46	-38			20	20							
Seepage Recycle to Cell Number	-	1	2			5	6							
Seepage Discharge Fraction	- ppb	30	30	30	30	30	30							
nitial P Storage Per Unit Area nitial Water Column Depth	mg/m2 cm	500 200	500 200	500 200	500 200	500 200	500 200							
C0 = Conc at 0 g/m2 P Storage C1 = Conc at 1 g/m2 P storage	ppb ppb	3 22	3 22	3 22	3 22	3 22	3 22							
C2 = Conc at Half-Max Uptake K = Net Settling Rate at Steady State	ppb m/yr	300 16.8	300 16.8	300 16.8	300 16.8	300 16.8	300 16.8							
21 = Saturated Uptake Depth 22 = Lower Penalty Depth	cm cm	40 100	40 100	40 100	40 100	40 100	40 100							
23 = Upper Penalty Depth	cm	200	200	200	200	200	200			<u> </u>				l
Execution Time	Sec/yr	1 9.18	2 9.73	3 10.27	4 10.91	5 11.45	6 12.09	7	8	9	10	11	12	12.0
Starting Date for Simulation	-	05/01/94	05/01/94	05/01/94	05/01/94	09/02/05	09/02/05							09/02
Inding Date	- -	04/30/05	04/30/05	04/30/05	04/30/05	04/30/05	04/30/05							04/30
Cell Label	uuyo	1A 1B	1B Outflow	2A 2B	2B Outflow	3A 3B	3B Outflow							Tota
Network Simulation Name	1	none Uncerta	none Uncerta	none Uncerta	none Uncerta	none Uncerta	none Uncerta							non Unce
Surface Area Mean Rainfall	km2 cm/yr	3.38 82.1	4.94 82.1	3.38 82.1	4.94 82.1	4.61 82.1	3.71 82.1							24.9 82.1
Mean ET Cell Inflow Volume	cm/yr hm3/yr	82.0 65.4	82.0 65.4	82.0 65.4	82.0 65.4	82.0 65.4	82.0 59.2							82.0 196.
Cell Inflow Load Cell Inflow Conc	kg/yr ppb	13047 199.4	7696 117.6	13048 199.4	8283 126.6	13047 199.4	6259 105.8							3914 199.
Freated Outflow Volume Freated Outflow Load	hm3/yr kg/yr	65.4 7696	65.4 3496	65.4 8283	65.4 4006	59.2 6259	53.7 3061							184. 1056
Freated FWM Outflow Conc Jpper Confidence Limit	ppb	117.6 129.8	53.4 67.9	126.6 140.5	61.2 79.9	105.8 122.0	57.0 75.4							57. 74.
Total Outflow Volume + Bypass	ppb hm3/yr	103.9 65.4	40.1	110.9 65.4	44.4 65.4	88.5 59.2	41.0 53.7							41.9
Total Outflow Load + Bypass	ppb	117.6	53.4	126.6	61.2	105.8	57.0							57.2
Sypass Load Sypass Load	kg/yi % bm3/d	0.0	0.0	0.0	0.0	0.0	0.0							0.0
Maximum Outflow	hm3/d	1.03	1.00	1.03	1.00	0.99	0.95							2.9
oad Trapped in Sediments	kg/yr %	4524 41%	3795	4860 37%	4416 52%	6096 52%	2933 51%							2662 739
ower Confidence Limit Jpper Confidence Limit	% %	35% 48%	48% 61%	30% 44%	43% 60%	45% 60%	44% 58%							65% 80%
Daily Geometric Mean Dutflow Geo Mean - Composites	ppb ppb	103.7 103.4	45.3 47.3	112.0 111.7	52.2 54.6	102.9 93.9	52.4 49.8							#N/. 50.5
Jpper Confidence Limit ower Confidence Limit	ppb ppb	114.5 90.6	61.09 34.48	124.7 96.9	72.6 38.3	109.3 77.5	67.3 34.5							66. 35.
requency Outflow Conc > 10 ppb requency Outflow Conc > 20 ppb	%	100% 100%	100% 100%	100% 100%	100% 100%	100% 100%	100% 100%							100
requency Outflow Conc > 50 ppb req Outflow Volume > 10 ppb	%	100%	44% 100%	100%	62% 100%	100%	54% 100%							100
Mean Biomass P Storage	mg/m2	4203	2413	4515	2808	4152	2482							335
Net Storage Turnover Rate	1/yr g/m2-yr	3.5	3.5	3.5	3.5	3.5	3.5							3.5
Mean Water Load Max Water Load	cm/d cm/d	5.3 32.7	3.6 20.9	5.3 32.7	3.6 20.9	3.9 24.0	4.4 26.6							2.2
Mean Depth Minimum Depth	cm cm	53 24	63 44	53 24	63 44	50 10	54 8							56 27
Maximum Depth requency Depth < 10 cm	cm %	90 0.0%	89 0.0%	90 0.0%	90 0.0%	89 0.2%	88 0.7%							89 0.19
low/Width HRT Days	m2/day days	115 9.9	115 17.3	115 9.9	115 17.3	115 12.8	104 12.4							113. 26.
Mean Velocity Seepage Outflow / Total Outflow	cm/sec %	0.25 0%	0.21 0%	0.25 0%	0.21 0%	0.27 10%	0.22 9%							0.23 6%
Release 1 Outflow Volume Release 2 Outflow Volume	hm3/yr hm3/yr	0.0	0.0	0.0	0.0	0.0	0.0							0.0
Sth Percentile Outflow Volume Sth Percentile Outflow Load	hm3/d kg/d	91.25	39.43	96.31	43.63	74.04	37.17							1.9
Release 1 Demand Met	%	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A							#N/. #N/.
Dutflow Demand Met	%	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A							#N/. #N/.
Range Check - Freq Depth < 10 cm Range Check - Flow/Midth	-	-	-		-	-	-							0
Range Check - Inflow Conc Range Check - Outflow Conc	-	-	-	-	-	-	-							0
Vater Balance Error Mass Balance Error	%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%							0.00
Varning or Error Messages		0.0070												0





# Table A.18 STA-6: Case "SEC1\_SDR"

DMSTA2- Inputs & Output	uts		Project: PROJECT_STA6 Model Release:											7/1/2005
Input Variable	Units	Value	Case Descrip	tion:								UL CL	inen Date.	00/04/03
Input Series Name Starting Date for Simulation	-	TS_SDRUnit2	Inflows limite	ed to historic in	flows from US	SC SDR Un	it 2; analysis	s limited to	WY 2001-2	004				
Ending Date for Simulation	-	04/30/04	Target Seep	age Outflow fr	om Cell 3 = 1,5	i were reduc i00 ac-ft/yr (	(no return);	5,000 ac-ft	/yr from Cell	5 (no retur	n)			
Integration Steps Per Day	-	4	Simulation Ty	pe:	Uncertainty A	nalysis	Linner Cl		Discussio	-				
Output Averaging Interval	days	7	FWM Outflow	/ C (ppb)	27.7	34.0	22.1 17.5		H20 Balan	ce Error Me	an & Max	0.0%	0.0%	
Rainfall P Conc Atmospheric R Lood (Dp.)	ppb	10	Load Reducti	on %	70%	63%	76%		Iterations &	& Converge	nce	-0.2%	0.2%	
Cell Label	ing/inz-yi	1	2 5	3	4	5	6	7	8	9 9	10	11	12	
Vegetation Type	>	PEW_3 0.34125	PEW_3											
Downstream Cell Number	- -	0.99	2.64											
Mean Width of Flow Path	km	0.61	1.31											
Minimum Depth for Releases Release 1 Series Name	cm	0.0	0.0											
Release 2 Series Name Outflow Series Name														
Depth Series Name Outflow Control Depth	cm	40	40											
Outflow Weir Depth Outflow Coefficient - Exponent	cm	4	4											
Outflow Coefficient - Intercept Bypass Depth	- cm	1	1											
Maximum Inflow Maximum Outflow	hm3/day hm3/day													
Inflow Seepage Rate Inflow Seepage Control Elev	(cm/d) / cm cm													
Inflow Seepage Conc Outflow Seepage Rate	ppb (cm/d) / cm	0.007	0.007											
Outflow Seepage Control Elev Max Outflow Seepage Conc	cm ppb	-30 20	-40 20											
Seepage Recycle to Cell Number Seepage Recycle Fraction		1	2											
Seepage Discharge Fraction Initial Water Column Conc	- ppb	30	30						1					
Initial P Storage Per Unit Area Initial Water Column Depth	mg/m2 cm	500 200	500 200											
C0 = Conc at 0 g/m2 P Storage C1 = Conc at 1 g/m2 P storage	ppb ppb	3 22	3 22	3 22	3 22	3 22	3 22							
C2 = Conc at Half-Max Uptake K = Net Settling Rate at Steady State	ppb m/yr	300 34.9	300 34.9	300 16.8	300 52.5	300 16.8	300 52.5							
Z1 = Saturated Uptake Depth Z2 = Lower Penalty Depth	cm cm	40 100	40 100	40 100	40 100	40 100	40 100							
Z3 = Upper Penalty Depth	cm	200	200	200	200	200	200		<u> </u>	·	<u> </u>			
Output Variables Execution Time	Units sec/yr	1 8.25	2 9.25	3	4	5	6	7	8	9	10	11	12	9.25
Run Date Starting Date for Simulation		08/04/05 05/01/00	08/04/05 05/01/00											08/04/05 05/01/00
Starting Date for Output Ending Date	-	05/01/00 04/30/04	05/01/00 04/30/04											05/01/00 04/30/04
Output Duration Cell Label	days	1461 3	1461 5											1461 Total
Network Simulation Name	-	none	none											none
Surface Area	km2	0.99	2.64											3.63
Mean ET	cm/yr cm/yr	27.8 21.4	27.8 20.7											27.8 20.9
Cell Inflow Load	kg/yr	1382	45.3 3679 91.2											5061 91.2
Treated Outflow Volume	hm3/yr	15.1	39.6											54.7
Treated FWM Outflow Conc	ppb	27.8	27.6											27.7
Lower Confidence Limit Total Outflow Volume + Bypass	ppb ppb hm3/yr	22.1	22.1											22.1
Total Outflow Load + Bypass Total FWM Outflow Conc	kg/yr	421 27.8	1095 27.6											1515.7 27.7
Bypass Load Bypass Load	kg/yr %	0 0.0	0 0.0											0.0 0.0
Maximum Inflow Maximum Outflow	hm3/d hm3/d	0.23 0.23	0.62 0.62											0.86 0.86
Surface Load Reduction Load Trapped in Sediments	kg/yr kg/yr	960 896	2585 2359											3545 3255
Overall Load Reduction Lower Confidence Limit	% %	70% 63%	70% 64%											70% 63%
Upper Confidence Limit Daily Geometric Mean	% ppb	76% 24.4	76% 24.4											76% #N/A
Outflow Geo Mean - Composites Upper Confidence Limit	ppb ppb	22.9 28.7	22.6 28.26											22.7 28.4
Lower Confidence Limit Frequency Outflow Conc > 10 ppb	ppb %	17.6 100%	17.42 100%											17.5 100%
Frequency Outflow Conc > 20 ppb Frequency Outflow Conc > 50 ppb	%	81% 1%	76% 1%											100% 78%
Freq Outflow Volume > 10 ppb 95th Percentile Outflow Conc	% ppb	100% 30	100% 29											100% 30
Mean Biomass P Storage Storage Increase / Net Removal	mg/m2 %	1362	1346											1350
Net Storage Turnover Rate Unit Area P Removal	1/yr g/m2-yr	2.7 904	2.7 894											2.7 897
Max Water Load Max Water Load	cm/d cm/d	4.7	4.7 23.6											4.7 23.6
Minimum Depth Maximum Depth	cm	46	47											47
Frequency Depth < 10 cm	cm %	2.4%	2.9%											2.7%
HRT Days	days	9.8	10.1											10.0
Seepage Outflow / Total Outflow	cm/sec %	11%	13%											12%
Release 2 Outflow Volume Release 2 Outflow Volume	hm3/yr	0.0	0.0											0.0
95th Percentile Outflow Load Simulated / Specified Mana Dant	kg/d	3.44 #N/A	8.86											12.3
Release 1 Demand Met	%	#N/A #N/A	#N/A #N/A											#N/A #N/A
Outflow Demand Met	%	#N/A #N/A	#N/A #N/A											#N/A #N/A
Range Check - Freq Depth < 10 cm	-	-	-											0
Range Check - Flow/Width Range Check - Inflow Conc Range Check - Outflow Conc	-	-	-											0
Water Balance Error Mass Balance Error	%	0.00%	0.00%											0.00%
Warning or Error Messages	,0	0.1778	0.1778											0
							-		-					_

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#### Table A.19 STA-6: Case "HIST\_SAV"

DMSTA2- Inputs & Outpu	uts		Project:	PROJECT_S	TA6							Mod	el Release: rrent Date:	7/1/2005 08/04/05
Input Variable Design Case Name	Units -	Value HIST_SAV	Case Descrip	tion: on 1 Only, veg	etation conside	red as SAV	in lieu of P	EW						
Input Series Name Starting Date for Simulation	-	TS_SDRUnit2 05/01/00	Inflows limit Flows in dat	ed to historic in a file increased	flows from US by 1.25 (flows	SC SDR Un were reduc	it 2; analysis ed by 20%	s limited to from histor	WY 2001-2 ric)	004				
Ending Date for Simulation Starting Date for Output	1	04/30/04 05/01/00	Target Seep	age Outflow fr	om Cell 3 = 1,5	500 ac-ft/yr (	no return);	5,000 ac-ft	per year fro	m Cell 5 (no	o return)			
Integration Steps Per Day Number of Iterations	-	4 0	Simulation Ty Output Variat	pe: <u>de</u>	Uncertainty Ar	Lower CL	Upper CL		Diagnostic	<u>s</u>		0.004	0.004	
Output Averaging Interval Inflow Conc Scale Factor	days -	1	GM Outflow C	C (ppb) C (ppb)	19.5 15.1 70%	23.3 18.6 76%	16.4 12.1		Mass Bala	ce Error Me nce Error M	lean & Max	-0.1%	0.0%	
Atmospheric P Load (Dry)	mg/m2-yr	20	Bypass Load	(%) 3	0.0%	5	6	7	Warning/E	rror Messag	ges 10	4	12	
Cell Label Vegetation Type	>	3 SAV_3	5 SAV_3						-					
Inflow Fraction Downstream Cell Number	1	0.34125	0.90875											
Surface Area Mean Width of Flow Path	km2 km	0.99 0.61	2.64 1.31											
Number of Tanks in Series Minimum Depth for Releases Release 1 Series Name	cm	3.0	3.0											
Release 2 Series Name Outflow Series Name														
Depth Series Name Outflow Control Depth	cm	40	40											
Outflow Weir Depth Outflow Coefficient - Exponent	cm -	4	4											
Outflow Coefficient - Intercept Bypass Depth	- cm	1	1											
Maximum Inflow Maximum Outflow	hm3/day hm3/day													
Inflow Seepage Rate Inflow Seepage Control Elev	(cm/d) / cm cm													
Outflow Seepage Rate Outflow Seepage Control Elev	(cm/d) / cm cm	0.007	0.007 -40											
Max Outflow Seepage Conc Seepage Recycle to Cell Number	ppb	20	20											
Seepage Recycle Fraction Seepage Discharge Fraction	1													
Initial Water Column Conc Initial P Storage Per Unit Area	ppb mg/m2	30 500	30 500											
C0 = Conc at 0 g/m2 P Storage	ppb	200 3 22	200	3	3	3	3							
C2 = Conc at Half-Max Uptake K = Net Settling Rate at Steady State	ppb m/yr	300 52.5	300 52.5	300 16.8	300 52,5	300 16.8	300 52,5							
Z1 = Saturated Uptake Depth Z2 = Lower Penalty Depth	cm	40 100	40 100	40 100	40 100	40 100	40 100							
Z3 = Upper Penalty Depth	cm	200	200	200	200	200	200		I	l				
Output Variables Execution Time	Units sec/yr	1 6.50	2 7.25	3	4	5	6	7	8	9	10	11	12	7.25
Run Date Starting Date for Simulation	-	05/01/00	05/01/00											08/04/05
Ending Date IOI Output Ending Date	- -	04/30/04	04/30/04											04/30/04
Cell Label Downstream Cell Label	dayo	3 Outflow	5 Outflow											Total
Network Simulation Name Simulation Type	-	none Uncerta	none Uncerta											none Uncerta
Surface Area Mean Rainfall	km2 cm/yr	0.99 27.8	2.64 27.8											3.63 27.8
Cell Inflow Volume	hm3/yr	21.4 17.0 1382	45.3 3679											62.3 5061
Cell Inflow Conc Treated Outflow Volume	ppb hm3/vr	81.3	81.3 39.6											81.3 54.7
Treated Outflow Load Treated FWM Outflow Conc	kg/yr ppb	297 19.6	773 19.5											1070 19.5
Upper Confidence Limit Lower Confidence Limit	ppb ppb	23.4 16.4	23.3 16.4											23.3 16.4
Total Outflow Volume + Bypass Total Outflow Load + Bypass	hm3/yr kg/yr	15.1 297	39.6 773											54.7 1069.5
Bypass Load	kg/yr	0	0											0.0
Maximum Inflow Maximum Outflow	hm3/d hm3/d	0.23	0.62											0.86 0.86
Surface Load Reduction Load Trapped in Sediments	kg/yr kg/yr	1085 1043	2906 2756											3991 3799
Overall Load Reduction Lower Confidence Limit	% %	79% 74%	79% 75%											79% 75%
Upper Confidence Limit Daily Geometric Mean	ppb	82% 15.6	82% 15.7											82% #N/A
Upper Confidence Limit	ppb ppb	18.8	18.55											18.6 12.1
Frequency Outflow Conc > 10 ppb Frequency Outflow Conc > 20 ppb	%	98% 9%	98% 9%											98% 45%
Frequency Outflow Conc > 50 ppb Freq Outflow Volume > 10 ppb	% %	1% 100%	1% 100%											9% 100%
95th Percentile Outflow Conc Mean Biomass P Storage	ppb mg/m2	22 1054	22 1046											22 1048
Storage Increase / Net Removal Net Storage Turnover Rate Unit Area P Removal	1/yr	4.0	4.0											4.0 1047
Mean Water Load Max Water Load	cm/d cm/d	4.7 23.6	4.7 23.6											4.7 23.6
Mean Depth Minimum Depth	cm cm	46 1	47 1											47 1
Maximum Depth Frequency Depth < 10 cm	cm %	79 2.4%	83 2.9%											82 2.7%
Flow/Width HRT Days	m2/day days	76 9.8	95 10.1											89.6 10.0
Mean Velocity Seepage Outflow / Total Outflow Release 1 Outflow Volume	cm/sec %	0.19	0.23											0.22
Release 2 Outflow Volume 95th Percentile Outflow Volume	hm3/yr hm3/d	0.0	0.0											0.0
95th Percentile Outflow Load Simulated / Specified Mean Depth	kg/d %	2.50 #N/A	6.42 #N/A											8.9 #N/A
Release 1 Demand Met Release 2 Demand Met	%	#N/A #N/A	#N/A #N/A											#N/A #N/A
Outflow Demand Met Range Check - Mean Depth	-	#N/A 0.74	#N/A 0.76											#N/A 2
Range Check - Freq Depth < 10 cm Range Check - Flow/Width Range Check - Inflow Conc	-	0.47	0.58											2
Range Check - Outflow Conc Water Balance Error		0.00%	0.00%											0
Mass Balance Error Warning or Error Messages	%	-0.11% Cell# 1 3 Depth of	-0.11% out of calib. range	for SAV_3: 46 vs	. 62 - 87 cm									-0.11% 4
		Cell# 1 3 Flow/W Cell# 2 5 Depth of	idth out of calib. range	ange for SAV_3: for SAV_3: 47 vs	76 vs. 162 - 374 m . 62 - 87 cm	n2/day								
		Cell# 2 5 Flow/W	idth out of calib. ra	ange for SAV_3:	95 vs. 162 - 374 m	n2/day								
	-		-	-		-	-			-		-	-	_





#### Table A.20 STA-6: Case "2006 Base"

DMSTA2- Inputs & Outpu	ıts		Project: PROJECT_STA6 Model Release:											
Input Variable	Units	Value	Case Descrip	tion:								CL	Irrent Date:	08/04/05
Design Case Name Input Series Name	-	2006_Base TS_Base	STA-6 Section	on 1 and Secti ed to USSO his	on 2 storic and 80%	of USSC SI	DR Unit 2 hi	storic						
Ending Date for Simulation	-	04/30/05	Net Area of	Cell 2 = approx	(. 1,300 ac.	DR for STA-	6 DY URS							
Integration Steps Per Day	-	4	Simulation Ty	pe:	Uncertainty A	nalysis	Linner Cl		Disessetia	-				_
Output Averaging Interval	days	7	FWM Outflow	v C (ppb)	14.3	17.6	11.8		H20 Balan	s ce Error Me	an & Max	0.0%	0.0%	
Rainfall P Conc Atmospheric B Lood (Dp)	ppb	10	Load Reducti	ion %	85%	82%	88%		Iterations &	& Converge	nce	3	0.0%	
Cell Number>	mg/m2-yr	1	2 6	( <sup>76)</sup> 3	4	5	6	7	8	9 9	10 10	5 11	12	
Vegetation Type	>	PEW_3	PEW_3	SAV_3										
Downstream Cell Number	- -	0.08	0.22	5.26										
Mean Width of Flow Path	km	0.61	1.31	2.39										
Minimum Depth for Releases Release 1 Series Name	cm	0.0	0.0	0.0										
Release 2 Series Name														
Depth Series Name	cm	40	40	60										
Outflow Weir Depth Outflow Coefficient - Exponent	cm	4	4	4										
Outflow Coefficient - Intercept Bypass Depth	- cm	1	1	1										
Maximum Inflow Maximum Outflow	hm3/day hm3/day													
Inflow Seepage Rate Inflow Seepage Control Elev	(cm/d) / cm cm													
Inflow Seepage Conc Outflow Seepage Rate	ppb (cm/d) / cm			0.005										
Outflow Seepage Control Elev Max Outflow Seepage Conc	cm ppb	-30 20	-40 20	-46 20										
Seepage Recycle to Cell Number Seepage Recycle Fraction	1	1 1	2 1	3										
Seepage Discharge Fraction Initial Water Column Conc	- ppb	30	30	30										
Initial P Storage Per Unit Area Initial Water Column Depth	mg/m2 cm	500 200	500 200	500 200										
C0 = Conc at 0 g/m2 P Storage C1 = Conc at 1 g/m2 P storage	ppb ppb	3 22	3 22	3 22	3 22	3 22	3 22							
C2 = Conc at Half-Max Uptake K = Net Settling Rate at Steady State	ppb m/yr	300 34.9	300 34.9	300 52.5	300 52.5	300 16.8	300 52.5							
21 = Saturated Uptake Depth 22 = Lower Penalty Depth	cm cm	40 100	40 100	40 100	40 100	40 100	40 100							
23 = Upper Penalty Depth	cm	200	200	200	200	200	200		I	·				
Execution Time	sec/yr	6.50	7.25	8.00 08/04/05	4	5			8	9	10	11	12	8.00
Starting Date for Simulation	-	05/01/97	05/01/97	05/01/97										05/01/97
Ending Date	-	04/30/05	04/30/05	04/30/05										04/30/05
Cell Label	days	2922 3	2922 5	2922										Total
Network Simulation Name	-	none	none	none										none
Surface Area Moon Roinfall	km2	0.99	2.64	5.26										8.89 41.0
Mean ET Cell Inflow Volume	cm/yr	38.1	38.1	38.1										38.1
Cell Inflow Load	kg/yr	664 85.5	1825	5807										8296 85.5
Treated Outflow Volume Treated Outflow Load	hm3/yr	7.8	21.4	57.9 851										87.2 1247
Treated FWM Outflow Conc Upper Confidence Limit	ppb	13.3 17.2	13.7	14.7 17.6										14.3 17.6
Lower Confidence Limit Total Outflow Volume + Bypass	ppb hm3/yr	10.5 7.8	10.8 21.4	12.4 57.9										11.8 87.2
Total Outflow Load + Bypass Total FWM Outflow Conc	kg/yr ppb	104 13.3	293 13.7	851 14.7										1247.3 14.3
Bypass Load Bypass Load	kg/yr %	0 0.0	0 0.0	0 0.0										0.0 0.0
Maximum Inflow Maximum Outflow	hm3/d hm3/d	0.13 0.11	0.35 0.29	1.12 0.99										1.60 1.39
Surface Load Reduction Load Trapped in Sediments	kg/yr kg/yr	560 584	1532 1596	4957 4852										7049 7032
Overall Load Reduction Lower Confidence Limit	% %	84% 80%	84% 79%	85% 82%										85% 82%
Upper Confidence Limit Daily Geometric Mean	% ppb	88% 9.8	87% 10.1	88% 9.8										88% #N/A
Outflow Geo Mean - Composites Upper Confidence Limit	ppb ppb	10.0 13.8	10.3 14.24	11.0 13.9										10.3 13.7
Lower Confidence Limit Frequency Outflow Conc > 10 ppb	ppb %	7.2 43%	7.45 53%	8.7 62%										7.7 51%
Frequency Outflow Conc > 20 ppb Frequency Outflow Conc > 50 ppb	%	2%	2%	4%										9% 3%
95th Percentile Outflow Conc	ppb	17	81% 17	83% 19										81% 18
Storage Increase / Net Removal	1/m2	0%	0%	924 0%										0%
Unit Area P Removal	g/m2-yr	5.5	605 2.2	922										791
Max Water Load Mean Depth	cm/d	12.9	13.4	21.3										18.0
Minimum Depth Maximum Depth	cm	37	37	42										40
Frequency Depth < 10 cm Flow/Width	% m2/day	0.0%	0.0%	0.0%										0.0%
HRT Days	days	20.6	20.7	16.9										18.0
Seepage Outflow / Total Outflow	6m/sec %	0.09	0.11	15%										10%
Release 2 Outflow Volume	hm3/yr	0.0	0.0	0.0										0.0
95th Percentile Outflow Load Simulated / Specified Mean Depth	kg/d	1.05 #N/A	2.91 #N/A	9.37 #N/A										13.5 #N/A
Release 1 Demand Met	%	#N/A #N/A	#N/A #N/A	#N/A #N/A										#N/A #N/A
Outflow Demand Met Range Check - Mean Depth	%	#N/A	#N/A	#N/A										#N/A
Range Check - Freq Depth < 10 cm Range Check - Flow/Width	-	- 0.51	- 0.65	- 0.48										0
Range Check - Inflow Conc Range Check - Outflow Conc		-	-	- 0.99										0
Water Balance Error Mass Balance Error	%	0.00%	0.00%	0.00%										0.00%
Warning or Error Messages		Cell#13 Flow/W Cell#25 Flow/W	idth out of calib. r	ange for PEW_3: ange for PEW_3:	35 vs. 69 - 276 m 45 vs. 69 - 276 m	12/day 12/day								5
		Cell#32 Depth o Cell#32 Flow/W	out of calib. range /idth out of calib. r	for SAV_3: 60 vs ange for SAV_3:	. 62 - 87 cm 78 vs. 162 - 374 n	n2/day								
		Cell# 3 2 Outflow	Conc out of calib	. range for SAV_3	15 vs. 15 - 153	ppb								

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#### Table A.21 STA-6: Case "Sect1\_USSO"

DMSTA2- Inputs & Output	uts		Project:	PROJECT_S	TA6							Mode	el Release:	7/1/2005
Input Variable	Units	Value	Case Descrip	tion:								Cu	nem Date.	05/04/03
Input Series Name Starting Date for Simulation	-	TS_USSO 05/01/96	Inflows limit	ed to historic di	ischarges from	C-139 Anne	ex (USSO)							
Ending Date for Simulation Starting Date for Output	-	04/30/05	0000 000pu	ge 100000 00 11	on existing (e									
Integration Steps Per Day	-	4	Simulation Ty	pe:	Uncertainty A	alysis	Lipper Cl		Diagnostic	e				
Output Averaging Interval	days	7	FWM Outflow	C (ppb)	25.1	32.0	19.4		H20 Balan Mass Balar	ce Error Me	an & Max ean & Max	0.0%	0.0%	
Rainfall P Conc Atmospheric P Load (Drv)	ppb ma/m2-vr	10 20	Load Reducti Bypass Load	on % (%)	78% 0.0%	72%	83%		Iterations & Warning/E	Converge	nce les	3	0.0%	
Cell Number> Cell Label	-	1	2	3	4	5	6	7	8	9	10	11	12	1
Vegetation Type Inflow Fraction	>	PEW_3 0.273	PEW_3 0.727											
Downstream Cell Number Surface Area	- km2	0.99	2.64											
Mean Width of Flow Path Number of Tanks in Series	km -	0.61 3.0	1.31 3.0											
Minimum Depth for Releases Release 1 Series Name	cm													
Release 2 Series Name Outflow Series Name														
Depth Series Name Outflow Control Depth	cm	40	40											
Outflow Weir Depth Outflow Coefficient - Exponent	- -	4	4											
Bypass Depth Maximum Inflow	cm hm3/day													
Maximum Outflow Inflow Seepage Rate	hm3/day (cm/d) / cm													
Inflow Seepage Control Elev Inflow Seepage Conc	cm ppb													
Outflow Seepage Rate Outflow Seepage Control Elev	(cm/d) / cm cm	0.007 -30	0.007 -40											
Max Outflow Seepage Conc Seepage Recycle to Cell Number	ppb -	20 1	20 2											
Seepage Recycle Fraction Seepage Discharge Fraction	-													
Initial Water Column Conc Initial P Storage Per Unit Area	ppb mg/m2	30 500	30 500											
C0 = Conc at 0 g/m2 P Storage	ppb	200	200 3	3	3	3	3							
C1 = Conc at 1 g/m2 P storage C2 = Conc at Half-Max Uptake	ppb	300	22 300	300 16.0	300 52 F	22 300	300 52 F							
Z1 = Saturated Uptake Depth	cm	40	40	40	40	40	40 100							
Z3 = Upper Penalty Depth	cm	200	200	200	200	200	200		<u> </u>					
Output Variables Execution Time	Units sec/yr	1 4.78	2 5.33	3	4	5	6	7	8	9	10	11	12	Overall 5.33
Run Date Starting Date for Simulation	-	08/04/05 05/01/96	08/04/05 05/01/96											08/04/05 05/01/96
Starting Date for Output Ending Date	1	05/01/96 04/30/05	05/01/96 04/30/05											05/01/96 04/30/05
Output Duration Cell Label	days	3287 3	3287 5											3287 Total
Downstream Cell Label Network Simulation Name	-	Outflow none	Outflow none											- none
Simulation Type Surface Area	- km2	Uncerta 0.99	Uncerta 2.64											Uncerta 3.63
Mean Rainfall Mean ET	cm/yr cm/yr	71.0 67.9	71.0 67.9											71.0 67.9
Cell Inflow Volume Cell Inflow Load	hm3/yr kg/yr	13.5	36.1 3545											49.6 4877
Treated Outflow Volume	ppp hm3/yr	98.3	98.3 30.4											98.3 42.0
Treated FWM Outflow Conc	ppb	25.2	25.0											25.1
Lower Confidence Limit Total Outflow Volume + Bypass	ppb ppb hm3/yr	19.4 11.7	19.4 30.4											19.4 42.0
Total Outflow Load + Bypass Total FWM Outflow Conc	kg/yr	294 25.2	760											1053.6
Bypass Load Bypass Load	kg/yr %	0 0.0	0 0.0											0.0 0.0
Maximum Inflow Maximum Outflow	hm3/d hm3/d	0.21 0.21	0.56 0.56											0.77 0.77
Surface Load Reduction Load Trapped in Sediments	kg/yr kg/yr	1038 989	2785 2601											3823 3589
Overall Load Reduction Lower Confidence Limit	%	78% 72%	79% 73%											78% 72%
Upper Confidence Limit Daily Geometric Mean	ppb	83% 21.7	83% 22.8											83% #N/A
Upper Confidence Limit	ppb	21.3 28.2	21.2 28.00											21.2 28.0
Frequency Outflow Conc > 10 ppb	ррь %	100%	100%											100%
Frequency Outflow Conc > 50 ppb Freq Outflow Volume > 10 ppb	%	0%	0% 100%											57% 100%
95th Percentile Outflow Conc Mean Biomass P Storage	ppb mg/m2	31 1503	30 1485											30 1490
Storage Increase / Net Removal Net Storage Turnover Rate	% 1/yr	0% 6.0	0% 6.0											0% 6.0
Unit Area P Removal Mean Water Load	g/m2-yr cm/d	998 3.7	985 3.7											989 3.7
Max Water Load Mean Depth	cm/d cm	21.3 45	21.3 46											21.3 45
Maximum Depth Maximum Depth	cm cm	6 76	3 80											4 79
Flequency Depth < 10 cm Flow/Width	m2/day	0.6% 61	2.8%											2.2%
HRT Days Mean Velocity	days cm/sec	12.0 0.16	12.2 0.19											12.1 0.18
Release 1 Outflow Volume	hm3/yr	0.0	0.0											0.0
95th Percentile Outflow Lood	hm3/yr hm3/d	0.11	0.28											0.0
Simulated / Specified Mean Depth Release 1 Demand Met	%	#N/A #N/A	#N/A #N/A											#N/A #N/A
Release 2 Demand Met Outflow Demand Met	%	#N/A #N/A	#N/A #N/A											#N/A #N/A
Range Check - Mean Depth Range Check - Freg Depth < 10 cm	-	-	-											0
Range Check - Flow/Width Range Check - Inflow Conc	1	0.88	1											1 0
Range Check - Outflow Conc Water Balance Error	- %	- 0.00%	- 0.00%											0.00%
Mass Balance Error Warning or Error Messages	%	0.00% Cell# 1 3 Flow/W	0.00% idth out of calib. r	ange for PEW_3:	61 vs. 69 - 276 m	2/day								0.00%

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# Table A.22 STA-6: Case "Sect1\_USSO\_SAV"

DMSTA2- Inputs & Outpu	uts		Project: PROJECT_STA6 Model Release:											
Input Variable	Units	Value	Case Descrip	tion:								Cu	nem Date.	08/04/05
Design Case Name Input Series Name	-	TS_USSO_S	STA-6 Secti Inflows limit	on 1 Only ad to historic di	scharges from	C-139 Ann	ex (USSO)							
Ending Date for Simulation	-	05/01/96	Vegetation of	ge losses as fr considered as \$	SAV in lieu of F	PEW								
Starting Date for Output Integration Steps Per Day	1	05/01/96 4	Simulation Ty	pe:	Uncertainty A	nalysis								
Number of Iterations Output Averaging Interval	- days	0 7	FWM Outflow	o <u>le</u> / C (ppb)	Mean 17.0	Lower CL 20.6	Upper CL 14.1		Diagnostic H20 Balan	<u>s</u> ce Error Me	an & Max	0.0%	0.0%	
Inflow Conc Scale Factor Rainfall P Conc	ppb	1 10	GM Outflow C Load Reduction	C (ppb) on %	13.1 85%	16.7 82%	10.3 88%		Mass Bala Iterations &	nce Error M & Converge	ean & Max nce	0.0%	0.0% 0.0%	
Atmospheric P Load (Dry) Cell Number>	mg/m2-yr	20 1	Bypass Load 2	(%) 3	0.0% 4	5	6	7	Warning/E 8	rror Messag 9	jes 10	4 11	12	
Vegetation Type	>	3 SAV_3	5 SAV_3											
Inflow Fraction Downstream Cell Number	. 1	0.273	0.727											
Surface Area Mean Width of Flow Path	km2 km	0.99 0.61	2.64 1.31											
Number of Tanks in Series Minimum Depth for Releases	- cm	3.0	3.0											
Release 1 Series Name Release 2 Series Name														
Outflow Series Name Depth Series Name														
Outflow Control Depth Outflow Weir Depth	cm	40	40											
Outflow Coefficient - Exponent Outflow Coefficient - Intercept	-	4	4											
Maximum Inflow	hm3/day													
Inflow Seepage Rate	(cm/d) / cm													
Inflow Seepage Conc	ppb	0.007	0.007											
Outflow Seepage Control Elev	cm	-30	-40											
Seepage Recycle to Cell Number	-	1	20											
Seepage Discharge Fraction	-	30	20											
Initial P Storage Per Unit Area	mg/m2	500	500											
C0 = Conc at 0 g/m2 P Storage C1 = Conc at 1 g/m2 P storage	ppb ppb	3 22	3 22	3 22	3 22	3 22	3 22							
C2 = Conc at Half-Max Uptake K = Net Settling Rate at Steady State	ppb m/yr	300 52.5	300 52.5	300 16.8	300 52.5	300 16.8	300 52.5							
Z1 = Saturated Uptake Depth Z2 = Lower Penalty Depth	cm cm	40 100	40 100	40 100	40 100	40 100	40 100							
Z3 = Upper Penalty Depth	cm	200	200	200	200	200	200	<u> </u>	<u> </u>	I				
Output Variables Execution Time	Units sec/yr	1 4.78	2 5.33	3	4	5	6	7	8	9	10	11	12	Overall 5.33
Run Date Starting Date for Simulation	-	08/04/05 05/01/96	08/04/05 05/01/96											08/04/05 05/01/96
Starting Date for Output Ending Date	1	05/01/96 04/30/05	05/01/96 04/30/05											05/01/96 04/30/05
Output Duration Cell Label	days	3287 3	3287 5											3287 Total
Downstream Cell Label Network Simulation Name	-	Outflow none	Outflow none											- none
Simulation Type Surface Area	km2	0.99	Uncerta 2.64											Uncerta 3.63
Mean Raintall Mean ET	cm/yr cm/yr	71.0 67.9	71.0 67.9											71.0 67.9
Cell Inflow Load	kg/yr	1331	3545											49.6
Treated Outflow Volume	hm3/yr	11.7	30.4											42.0
Treated FWM Outflow Conc	ppb	17.0	17.0											17.0
Lower Confidence Limit	ppb ppb	14.2	20.6											14.1
Total Outflow Load + Bypass	kg/yr	199	515											713.8
Bypass Load Bypass Load	kg/yr	0	0											0.0
Maximum Inflow Maximum Outflow	hm3/d hm3/d	0.21	0.56											0.77
Surface Load Reduction Load Trapped in Sediments	kg/yr kg/yr	1133 1103	3030 2915											4163 4018
Overall Load Reduction Lower Confidence Limit	%	85% 82%	85% 82%											85% 82%
Upper Confidence Limit Daily Geometric Mean	% ppb	88% 13.0	88% 13.6											88% #N/A
Outflow Geo Mean - Composites Upper Confidence Limit	ppb ppb	13.2 16.8	13.2 16.76											13.1 16.7
Lower Confidence Limit Frequency Outflow Conc > 10 ppb	ppb %	10.3 87%	10.33 87%											10.3 86%
Frequency Outflow Conc > 20 ppb Frequency Outflow Conc > 50 ppb	% %	7% 0%	7% 0%											25% 7%
Freq Outflow Volume > 10 ppb 95th Percentile Outflow Conc	% ppb	97% 22	97% 22											97% 22
Storage Increase / Net Removal	mg/m2 %	0%	0%											0%
Unit Area P Removal	g/m2-yr	9.0 1113	9.0 1105											9.0 1107
Max Water Load Max Dopth	cm/d	21.3	21.3											21.3
Minimum Depth Maximum Depth	cm	6	3											4 79
Frequency Depth < 10 cm Flow/Width	% m2/day	0.6%	2.8%											2.2% 71.4
HRT Days Mean Velocity	days cm/sec	12.0 0.16	12.2 0.19											12.1 0.18
Seepage Outflow / Total Outflow Release 1 Outflow Volume	% hm3/yr	14% 0.0	16% 0.0											15% 0.0
Release 2 Outflow Volume 95th Percentile Outflow Volume	hm3/yr hm3/d	0.0 0.11	0.0 0.28											0.0 0.4
95th Percentile Outflow Load Simulated / Specified Mean Depth	kg/d %	2.14 #N/A	5.65 #N/A											7.8 #N/A
Release 1 Demand Met Release 2 Demand Met	%	#N/A #N/A	#N/A #N/A											#N/A #N/A
Outflow Demand Met Range Check - Mean Depth	-	#N/A 0.72	#N/A 0.74											#N/A
Range Check - Freq Depth < 10 cm Range Check - Flow/Width	1	0.38	0.47											0
Range Check - Inflow Conc Range Check - Outflow Conc	-	-	-											0
Mass Balance Error	%	0.00%	0.00%	for SAV 2: 45	62 . 97									0.00%
wanning or error messages		Cell# 1 3 Depth o Cell# 1 3 Flow/W	idth out of calib. range	ange for SAV_3: 45 vs	61 vs. 162 - 374 n	n2/day								4
		Cell# 2 5 Flow/W	idth out of calib. r	ange for SAV_3: 46 vs	75 vs. 162 - 374 n	n2/day								

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#### Table A.23 STA-3/4: Case "2006 WS"

	ns		Project:	PROJECT_5	1A34							Cu	rrent Date:	7/1/2005 09/02/05
Input Variable Design Case Name Input Series Name	- -	2006 WS	Case Descrip STA-3/4, all All inflows	tion: cells										
Starting Date for Simulation		05/01/65	Lake water s	upply releases	to low WCA a	ind to Big C	ypress Rese	ervation ind	luded in trea	atment area	inflows.			
Starting Date for Output Integration Steps Per Day		05/01/65	Simulation Tyr	) <del>0</del> .	Base									
Number of Iterations Output Averaging Interval	- days	0 7	Output Variat	Le C (ppb)	Mean 20.3	Lower CL #N/A	Upper CL #N/A		Diagnostic H20 Balan	<u>s</u> ce Error Me	an & Max	0.0%	0.0%	
Inflow Conc Scale Factor Rainfall P Conc	- ppb	1 10	GM Outflow C Load Reduction	(ppb) on %	15.7 76%	#N/A #N/A	#N/A #N/A		Mass Bala Iterations &	nce Error M & Converge	lean & Max nce	0.1%	0.1% 0.0%	
Atmospheric P Load (Dry) Cell Number>	mg/m2-yr	20	Bypass Load 2	(%) 3	0.0% <b>4</b>	5	6	7	Warning/E 8	rror Messag 9	jes 10	3 11	12	
Cell Label Vegetation Type	>	1A EMG_3	1B PEW_3	2A EMG_3	2B SAV_3	3A EMG_3	3B SAV_3							
Inflow Fraction Downstream Cell Number	1	0.34 2		0.36 4		0.3 6								
Surface Area Mean Width of Flow Path	km2 km	12.30 3.42	14.12 4.50	10.29 2.89	11.71 4.02	9.61 4.88	8.92 4.88							
Number of Tanks in Series Minimum Depth for Releases	- cm	6.0	3.0	6.0	3.0	4.0	4.0							
Release 1 Series Name Release 2 Series Name														
Depth Series Name Outflow Control Dopth		60	60	60	60	60	60							
Outflow Weir Depth Outflow Coefficient - Exponent	cm	4	4	4	4	4	4							
Outflow Coefficient - Intercept Bypass Depth	- cm	1	1	1	1	1	1							
Maximum Inflow Maximum Outflow	hm3/day hm3/day													
Inflow Seepage Rate Inflow Seepage Control Elev	(cm/d) / cm cm													
Inflow Seepage Conc Outflow Seepage Rate	ppb (cm/d) / cm	0.0058	0.0029	0.0014		0.0038								
Outflow Seepage Control Elev Max Outflow Seepage Conc	cm ppb	16 20	40 20	-67 20		-64 20								
Seepage Recycle to Cell Number Seepage Recycle Fraction	1	1 0.5	1 0.5	3 0.5		3 0.5								
Seepage Discharge Fraction Initial Water Column Conc	ppb	30	30	30	30	30	30							
Initial P Storage Per Unit Area Initial Water Column Depth	mg/m2 cm	500 200	500 200	500 200	500 200	500 200	500 200							
C1 = Conc at 1 g/m2 P Storage C1 = Conc at 1 g/m2 P storage	ppb	22 200	3 22 200	3 22 300	22 200	22 200	3 22 300							
K = Net Settling Rate at Steady State	ppp m/yr	16.8	300 34.9	16.8 40	52.5	16.8	52.5							
Z1 = Saturated Optake Depth Z2 = Lower Penalty Depth Z2 = Lipper Penalty Depth	cm	100	100	100	100	100	100							
Output Variables	Units	1	200	3	4	5	6	7	8	9	10	11	12	Overall
Execution Time Run Date	sec/yr	7.83 09/02/05	8.26 09/02/05	9.00 09/02/05	9.43 09/02/05	9.97 09/02/05	10.51 09/02/05							10.51 09/02/05
Starting Date for Simulation Starting Date for Output	1	05/01/65 05/01/65	05/01/65 05/01/65	05/01/65 05/01/65	05/01/65 05/01/65	05/01/65 05/01/65	05/01/65 05/01/65							05/01/65 05/01/65
Ending Date Output Duration	- days	04/30/00 12784	04/30/00 12784	04/30/00 12784	04/30/00 12784	04/30/00 12784	04/30/00 12784							04/30/00 12784
Cell Label Downstream Cell Label		1A 1B	1B Outflow	2A 2B	2B Outflow	3A 3B	3B Outflow							Total
Network Simulation Name Simulation Type	1	none Base	none Base	none Base	none Base	none Base	none Base							none Base
Surface Area Mean Rainfall	km2 cm/yr	12.30 130.0	14.12 130.0	10.29 130.0	11.71 130.0	9.61 130.0	8.92 130.0							66.94 130.0
Mean E I Cell Inflow Volume	cm/yr hm3/yr	134.9 290.7	134.9 285.1	134.9 307.8	134.9 312.2	134.9 256.5	134.9 239.2							134.9 855.1
Cell Inflow Load Cell Inflow Conc Traated Outflow Volume	кg/yr ppb bm2/vr	23704 81.5 295.1	14349 50.3	25099 81.5 212.2	55.1	81.5 220 2	12928 54.1							81.5 821.0
Treated Outflow Load	kg/yr	14349	5781 20.6	17201	6290	12928 54.1	4806 20.1							16877
Upper Confidence Limit	ppb	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A							#N/A #N/A
Total Outflow Volume + Bypass Total Outflow Load + Bypass	hm3/yr kg/yr	285.1 14349	280.7 5781	312.2 17201	311.6 6290	239.2 12928	238.7 4806							831.0 16877.3
Total FWM Outflow Conc Bypass Load	ppb kg/yr	50.3 0	20.6 0	55.1 0	20.2 0	54.1 0	20.1 0							20.3 0.0
Bypass Load Maximum Inflow	% hm3/d	0.0 4.34	0.0 4.14	0.0 4.60	0.0 4.38	0.0 3.83	0.0 3.63							0.0 12.77
Maximum Outflow Surface Load Reduction	hm3/d kg/yr	4.14 9355	4.29 8568	4.38 7898	4.58 10912	3.63 7987	3.77 8122							12.64 52842
Load Trapped in Sediments Overall Load Reduction	kg/yr %	9126 39%	8938 60%	8018 31%	11295 63%	7305 38%	8409 63%							53091 76%
Lower Confidence Limit Upper Confidence Limit	%	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A							#N/A #N/A
Outflow Geo Mean - Composites	ppb	44.5 45.3 #N/A	17.0	49.5 50.3	14.6 15.7	47.5 50.1	15.9 #NI/A							15.7 #N/A
Lower Confidence Limit	ppb %	#N/A 100%	#N/A 98%	#N/A 100%	#N/A 91%	#N/A 100%	#N/A 92%							#N/A 91%
Frequency Outflow Conc > 20 ppb Frequency Outflow Conc > 50 ppb	%	100%	29%	100%	25%	100%	28%							55% 24%
Freq Outflow Volume > 10 ppb 95th Percentile Outflow Conc	% ppb	100%	100% 26	100% 61	98% 26	100% 61	97% 26							99% 26
Mean Biomass P Storage Storage Increase / Net Removal	mg/m2 %	2330 0%	954 0%	2446 0%	966 0%	2387 0%	944 0%							1643 0%
Net Storage Turnover Rate Unit Area P Removal	1/yr g/m2-yr	11.1 742	23.2 633	11.1 779	34.9 965	11.1 760	34.9 943							16.9 793
Mean Water Load Max Water Load	cm/d cm/d	6.5 35.3	5.5 29.3	8.2 44.7	7.3 37.4	7.3 39.8	7.3 40.7							3.5 19.1
Mean Depth Minimum Depth	cm cm	69 47	65 32	72 54	68 45	63 22	63 33							67 39
Maximum Depth Frequency Depth < 10 cm	cm %	105 0.0%	99 0.0%	111 0.0%	103 0.0%	93 0.0%	94 0.0%							101 0.0%
How/Width HRT Days	m2/day days	233	173	292	213 9.3	144 8.6	134 8.6							199.9
Mean Velocity Seepage Outflow / Total Outflow	cm/sec	0.39	0.31	0.47	0.36	0.27	0.25							0.34
Release 2 Outflow Volume 95th Percentile Outflow Volume	hm3/yr hm3/d	0.0	0.0	0.0	0.0	0.0	0.0							0.0
95th Percentile Outflow Load Simulated / Specified Mean Depth	kg/d	132.64 #N/A	61.28 #N/A	155.68 #N/A	65.85 #N/A	123.93 #N/A	52.94 #N/A							179.5 #N/A
Release 1 Demand Met Release 2 Demand Met	%	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A							#N/A #N/A
Outflow Demand Met Range Check - Mean Depth	%	#N/A	#N/A -	#N/A	#N/A	#N/A	#N/A							#N/A 0
Range Check - Freq Depth < 10 cm Range Check - Flow/Width	1	- 1.11	-	- 1.39	1	1	- 0.83							0
Range Check - Inflow Conc Range Check - Outflow Conc	1	-	-	1	1	1	1							0 0
Water Balance Error Mass Balance Error	% %	0.00% 0.08%	0.00% 0.01%	0.00% 0.09%	0.00%	0.00%	0.00%							0.00% 0.09%
Warning or Error Messages		Cell# 1 1A Flow/ Cell# 3 2A Flow/	Width out of calib. Width out of calib.	range for EMG_3: range for EMG_3:	233 vs. 26 - 210 292 vs. 26 - 210	m2/day m2/day								3
		Cell# 6 3B Flow/	Width out of calib.	range for SAV_3:	134 vs. 162 - 37	1 m2/day								

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## Table A.24 STA-1W: Case "2006 WS"

Input Variable	Units	Value	Case Descrip	tion:								Cu	rrent Date:	09/02/05
Design Case Name Input Series Name Starting Data for Simulation	-	2006_WS TS_2006_WS	STA-1W with Potential Inc	h Long Term P lude all S-5A E	Plan Enhancem Basin and EBW	ents /CD Runoff	from ECP 2	006 Simula	tion to STA	-1 I&D Wor	ks			
Ending Date for Simulation Starting Date for Simulation	-	04/30/00	Added S-35	2 Water Supply	y Releases Ori	ginally Simu	lated to Byp	ass to LNV	VR VR	pair or ST	A-TE			
Integration Steps Per Day Number of Iterations	1	4 0	Simulation Typ Output Variab	oe: ole	Base Mean	Lower CL	Upper CL		Diagnostic	5				
Output Averaging Interval Inflow Conc Scale Factor	days -	7	FWM Outflow GM Outflow 0	C (ppb) (ppb)	20.5 14.0	#N/A #N/A	#N/A #N/A		H20 Balan Mass Balar	ce Error Me nce Error M	an & Max lean & Max	0.0% -0.2%	0.0% 0.4%	
Rainfall P Conc Atmospheric P Load (Dry)	ppb mg/m2-yr	10 20	Load Reducti Bypass Load	on % (%)	88% 0.0%	#N/A	#N/A		Iterations & Warning/E	k Converge	nce Jes	3 6	0.2%	
Cell Number> Cell Label	-	1 1A	2 1B	3	4 2A	5 2B	6 4	7 5A	8 5B	9	10	11	12	1
Vegetation Type Inflow Fraction	>	EMG_3 0.38	SAV_3	SAV_3	EMG_3 0.17	SAV_3	SAV_3	EMG_3 0.45	SAV_3					
Surface Area Moap Width of Flow Bath	km2	3.02	3 3.02	4.15	1.91 2.40	1.91	1.45	2.27	9.28					
Number of Tanks in Series Minimum Depth for Releases	- cm	2.0	2.0	2.0	2.0	2.00	2.0	2.0	3.0					
Release 1 Series Name Release 2 Series Name														
Outflow Series Name Depth Series Name														
Outflow Control Depth Outflow Weir Depth	cm cm	55	55	46	60	60	60	60	60					
Outflow Coefficient - Exponent Outflow Coefficient - Intercept	-	4 1	4 1	4 1	4 1	4	4 1	4 1	4 1					
Bypass Depth Maximum Inflow Maximum Outflow	cm hm3/day													
Inflow Seepage Rate	(cm/d) / cm	0.0035	0.0018	0.0023										
Inflow Seepage Conc Outflow Seepage Rate	ppb (cm/d) / cm	20	20	20 0.0014	0.0016	0.0016	0.0021	0.0156	0.0049					
Outflow Seepage Control Elev Max Outflow Seepage Conc	cm ppb			-60 20	-46 20	-46 20	-46 20	-46 20	-46 20					
Seepage Recycle to Cell Number Seepage Recycle Fraction	1							1 0.91	1 0.8					
Seepage Discharge Fraction Initial Water Column Conc	- ppb	30	30	30	30	30	30	30	30					
Initial Mater Column Depth	mg/m2 cm	200	200	200	200	200	200	200	200					
C1 = Conc at 0 g/m2 P Storage C2 = Conc at 1 g/m2 P storage C2 = Conc at Half-Max Uptake	ppb ppb	22 300	22 300	22 300	22 300	22 300	22 300	22 300	22 300					
K = Net Settling Rate at Steady State Z1 = Saturated Uptake Depth	m/yr cm	16.8 40	52.5 40	52.5 40	16.8 40	52.5 40	52.5 40	16.8 40	52.5 40					
Z2 = Lower Penalty Depth Z3 = Upper Penalty Depth	cm	100 200	100 200	100 200	100 200	100 200	100 200	100 200	100 200					
Output Variables	Units	1	2	3	4	5	6	7	8	9	10	11	12	Overall
Execution Time Run Date	sec/yr -	6.89 09/02/05	7.26 09/02/05	7.63 09/02/05	8.00 09/02/05	8.37 09/02/05	8.74 09/02/05	9.11 09/02/05	9.57 09/02/05					9.57 09/02/05
Starting Date for Simulation Starting Date for Output	1	05/01/65	05/01/65	05/01/65	05/01/65	05/01/65	05/01/65	05/01/65	05/01/65					05/01/65
Output Duration	days	12784	12784 1B	12784	12784	12784 2B	12784	12784 54	12784 5B					12784 Total
Downstream Cell Label Network Simulation Name	-	1B none	3 none	Outflow	2B none	4 none	Outflow	5B none	Outflow					- none
Simulation Type Surface Area	- km2	Base 3.02	Base 3.02	Base 4.15	Base 1.91	Base 1.91	Base 1.45	Base 2.27	Base 9.28					Base 27.00
Mean Rainfall Mean ET	cm/yr cm/yr	134.9 129.8	134.9 129.8	134.9 129.8	134.9 129.8	134.9 129.8	134.9 129.7	134.9 124.8	134.9 127.4					134.9 128.6
Cell Inflow Volume Cell Inflow Load	hm3/yr kg/yr	83.8 14494	111.6 11599	113.9 5450	37.5 6484	36.4 3994	35.4 1295	99.2 17164	87.2 11219					220.5 38143
Cell Inflow Conc Treated Outflow Volume	ppb hm3/yr	173.0 111.6	103.9	47.9	173.0 36.4	109.6 35.4	36.6 34.4	173.0 87.2	128.6					173.0 222.1
Treated FWM Outflow Conc	ppb	103.9 #N/A	47.9 #N/Δ	20.0 #N/A	109.6 #N/A	36.6 #N/A	19.9 #N/A	128.6 #N/A	21.7 #N/A					4562 20.5 #N/A
Lower Confidence Limit Total Outflow Volume + Bypass	ppb hm3/vr	#N/A 111.6	#N/A 113.9	#N/A 115.5	#N/A 36.4	#N/A 35.4	#N/A 34.4	#N/A 87.2	#N/A 72.3					#N/A 222.1
Total Outflow Load + Bypass Total FWM Outflow Conc	kg/yr ppb	11599 103.9	5450 47.9	2313 20.0	3994 109.6	1295 36.6	682 19.9	11219 128.6	1567 21.7					4562.2 20.5
Bypass Load Bypass Load	kg/yr %	0 0.0	0 0.0	0 0.0	0 0.0	0 0.0	0 0.0	0 0.0	0 0.0					0.0 0.0
Maximum Inflow Maximum Outflow	hm3/d hm3/d	2.45 2.60	2.60 2.73	2.73 2.86	1.10 1.10	1.10 1.16	1.16 1.21	2.91 2.82	2.82 3.06					6.46 7.13
Load Trapped in Sediments	kg/yr kg/yr	2895 3505 20%	6290 62%	3138 3304	2490	2699	646 47%	2784	9652 9196					33581
Lower Confidence Limit	%	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A					#N/A #N/A
Daily Geometric Mean Outflow Geo Mean - Composites	ppb	90.9 92.4	39.0 39.6	14.2 14.5	93.9 100.8	22.2 28.4	9.2 12.9	138.4 118.2	11.7 14.6					#N/A 14.0
Upper Confidence Limit Lower Confidence Limit	ppb ppb	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A					#N/A #N/A
Frequency Outflow Conc > 10 ppb Frequency Outflow Conc > 20 ppb	%	100% 100%	100% 100%	86% 16%	100% 100%	100% 88%	71% 14%	100% 100%	84% 18%					84% 39%
Frequency Outflow Conc > 50 ppb Freq Outflow Volume > 10 ppb	%	100%	17% 100%	0% 96%	100%	3% 100%	0% 89%	100%	0% 95%					12% 96%
Mean Biomass P Storage Storage Increase / Net Removal	mg/m2	3650	2090	25 797 0%	3997 0%	45 1430 0%	446 0%	3844	993 0%					1836 0%
Net Storage Turnover Rate	1/yr g/m2-yr	11.1	34.9 2086	34.9 796	11.1 1273	34.9 1428	34.9 446	11.1	34.9 991					21.8 1144
Mean Water Load Max Water Load	cm/d cm/d	7.6 81.4	10.1 86.1	7.5	5.4 57.6	5.2 58.0	6.7 80.3	11.9 127.8	2.6 30.4					2.2 23.9
Mean Depth Minimum Depth	cm cm	67 55	68 53	67 43	57 20	56 19	55 1	50 1	49 1					58 22
Maximum Depth Frequency Depth < 10 cm	cm %	122 0.0%	124 0.0%	126 0.0%	81 0.0%	86 0.0%	97 1.3%	110 8.6%	106 6.2%					109 2.9%
Flow/Width HRT Days	m2/day days	209 8.8	278 6.7	283 8.9	43 10.6	50 10.7	75 8.2	153 4.2	102 19.2					156.4 25.7
Mean Velocity Seepage Outflow / Total Outflow	cm/sec %	0.36	0.47	0.49	0.09	0.10	0.16	0.35	0.24					0.30
Release 1 Outflow Volume Release 2 Outflow Volume	hm3/yr hm3/yr	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0					0.0
95th Percentile Outflow Load Simulated / Specified Mean Depth	kg/d	119.53 #N/A	55.57 #N/A	24.38 #N/A	47.80 #N/A	16.97 #N/A	9.25 #N/A	138.20 #N/A	22.33 #N/A					55.7 #N/A
Release 1 Demand Met Release 2 Demand Met	%	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A					#N/A #N/A
Outflow Demand Met Range Check - Mean Depth	%	#N/A	#N/A	#N/A	#N/A -	#N/A 0.90	#N/A 0.88	#N/A	#N/A 0.80					#N/A 3
Range Check - Freq Depth < 10 cm Range Check - Flow/Width	1	1		1	1	- 0.31	- 0.46	1	- 0.63					0
Range Check - Inflow Conc Range Check - Outflow Conc	-	-	-	-	-	-	-	-	-					0
Water Balance Error Mass Balance Error Wareing or Error	%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%					0.00% -0.16%
warning or Error Messages		Cell# 5 2B Flow/ Cell# 6 4 Depth	Width out of calib.	range for SAV_3: 56 v for SAV_3: 55 ve	50 vs. 162 - 374 50 vs. 162 - 374	m2/day								0
		Cell# 6 4 Flow/W Cell# 8 5B Depth	idth out of calib. range	inge for SAV_3: for SAV_3: 49 v	75 vs. 162 - 374 n /s. 62 - 87 cm	n2/day								
		Cell# 8 5B Flow/	Width out of calib.	range for SAV_3:	102 vs. 162 - 37	4 m2/day								

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## Table A.25 STA-1E: Case "3\_4 2006PSTA"

DMSTA2- Inputs & Outputs Project: PROJECT_STA1E Model Release: Current Date:										7/1/2005 09/06/05				
Input Variable Design Case Name	Units -	Value 3 4 2006PSTA	Case Descrip	tion: fSTA-1E with	western half of	East Distrit	oution Cell							
Input Series Name Starting Date for Simulation	-	TS_1_42006AI 05/01/65	Inflows from Cell-to-cell s	C-51 West ad	justed for same nsidered in ana	e-day bypas alysis.	is to S-155A	of L-8 run	off through	S-5AE, limit	ed to desigr	n capacity of	S-366	
Ending Date for Simulation Starting Date for Output	1	04/30/00 05/01/65	Cells 1 and 2	2 considered o	ff-line due to co	onstruction a	and operatio	on of PSTA	Demonstra	tion Project				
Integration Steps Per Day Number of Iterations	1	4 0	Simulation Typ Output Variab	pe: ole	Uncertainty Ar Mean	alysis Lower CL	Upper CL		Diagnostic	5				
Output Averaging Interval Inflow Conc Scale Factor	days -	7 1	FWM Outflow GM Outflow 0	/ C (ppb) C (ppb)	37.1 32.3	45.6 40.7	29.2 24.6		H20 Balan Mass Bala	ce Error Me nce Error M	an & Max ean & Max	3.2% 0.6%	0.0% 0.0%	
Rainfall P Conc Atmospheric P Load (Dry)	ppb mg/m2-yr	10 20	Load Reducti Bypass Load	on % (%)	70% 0.0%	63%	77%		Iterations & Warning/E	k Converger rror Messag	nce Jes	3	0.0%	
Cell Number> Cell Label	-	1 EDCW	2	3 4N	4 4S	5	6	7	8	9	10	11	12	
Vegetation Type Inflow Fraction	>	1 2	EMG_3	SAV_3	SAV_3									
Surface Area Moon Width of Flow Both	km2	0.95	2.38	2.61	3.04									
Number of Tanks in Series Minimum Depth for Releases	- cm	0.5	3.0	3.0	3.0									
Release 1 Series Name Release 2 Series Name														
Outflow Series Name Depth Series Name														
Outflow Control Depth Outflow Weir Depth	cm cm	90	40	60	60									
Outflow Coefficient - Exponent Outflow Coefficient - Intercept	-	4 1	4 1	4 1	4 1									
Bypass Depth Maximum Inflow	cm hm3/day	3.768												
Inflow Seepage Rate	(cm/d) / cm													
Inflow Seepage Conc Outflow Seepage Rate	ppb (cm/d) / cm	0.0095			0.0054									
Outflow Seepage Control Elev Max Outflow Seepage Conc	cm	-87 20			-38 20									
Seepage Recycle to Cell Number Seepage Recycle Fraction	1	4			7 1									
Seepage Discharge Fraction Initial Water Column Conc	- ppb	30	30	30	30									
Initial P Storage Per Unit Area Initial Water Column Depth	mg/m2 cm	500 50	500 50	500 50	500 50									
CU = Conc at 0 g/m2 P Storage C1 = Conc at 1 g/m2 P storage	ppb ppb	3 22	3 22	3 22	3 22	3 22	3 22	3 22						
K = Net Settling Rate at Steady State	ppp m/yr	16.8 40	16.8 40	52.5	52.5	16.8	52.5	52.5						
Z1 = Saturated Optake Depth Z2 = Lower Penalty Depth Z3 = Upper Penalty Depth	cm	100	100	100	100	100	100	100						
Output Variables	Units	1	2	3	4	5	6	7	8	9	10	11	12	Overall
Execution Time Run Date	sec/yr -	3.94 09/06/05	4.40 09/06/05	4.86 09/06/05	5.31 09/06/05									5.31 09/06/05
Starting Date for Simulation Starting Date for Output	1	05/01/65 05/01/65	05/01/65 05/01/65	05/01/65 05/01/65	05/01/65 05/01/65									05/01/65 05/01/65
Ending Date Output Duration	- days	04/30/00 12784	04/30/00 12784	04/30/00 12784	04/30/00 12784									04/30/00 12784
Cell Label Downstream Cell Label		EDCW 3	3 4N	4N 4S	4S Outflow									Total -
Simulation Type	- - km2	Uncerta	Uncerta	Uncerta	Uncerta									Uncerta
Mean Rainfall Mean FT	cm/yr	142.9	142.9	142.9	142.9									142.9
Cell Inflow Volume Cell Inflow Load	hm3/yr kg/yr	199.0 24056	193.0 22214	193.3 19423	193.6 12332									199.0 24056
Cell Inflow Conc Treated Outflow Volume	ppb hm3/yr	120.9 193.0	115.1 193.3	100.5 193.6	63.7 193.5									120.9 193.5
Treated Outflow Load Treated FWM Outflow Conc	kg/yr ppb	22214 115.1	19423 100.5	12332 63.7	7181 37.1									7181 37.1
Upper Confidence Limit Lower Confidence Limit	ppb ppb	116.4 113.5	104.8 95.2	71.9 54.9	45.6 29.2									45.6 29.2
Total Outflow Load + Bypass Total Outflow Load + Bypass	kg/yr	22217	193.3	12332	7181									7183.5
Bypass Load	kg/yr	3	0	0	0									2.9
Maximum Inflow Maximum Outflow	hm3/d hm3/d	3.44 3.42	3.42 3.43	3.43 3.42	3.42 3.38									3.44 3.38
Surface Load Reduction Load Trapped in Sediments	kg/yr kg/yr	1842 1197	2791 2871	7092 7180	5151 5105									16876 16353
Overall Load Reduction Lower Confidence Limit	% %	8% 7%	13% 10%	37% 31%	42% 37%									70% 63%
Upper Confidence Limit Daily Geometric Mean	% ppb	9% 109.3	16% 95.3	42% 56.9	47% 31.9									77% #N/A
Upper Confidence Limit	ppb	109.5	95.4 100.11	57.4 65.9	32.3 40.7									32.3 40.7
Frequency Outflow Conc > 10 ppb	рро % %	107.5	100%	48.5 100%	100%									24.0 100% 99%
Frequency Outflow Conc > 50 ppb Freq Outflow Volume > 10 ppb	%	100%	100%	77%	3% 100%									96% 100%
95th Percentile Outflow Conc Mean Biomass P Storage	ppb mg/m2	142 4073	111 3790	77 2758	48 1684									48 2807
Storage Increase / Net Removal Net Storage Turnover Rate	% 1/yr	0% 10.8	0% 11.1	0% 34.9	0% 34.9									0% 22.7
Unit Area P Removal Mean Water Load	g/m2-yr cm/d	1259 57.3	1206 22.2	2751 20.3	1679 17.4									1821 6.1
Max water Load Mean Depth Minimum Dooth	cm/a cm	361.3 99	143.7 70 24	131.3 74 45	112.4 74									38.3
Maximum Depth Frequency Depth < 10 cm	cm %	150	122	122	121									125
Flow/Width HRT Days	m2/day days	825 1.7	341 3.2	341 3.6	342 4.2									392.7 12.4
Mean Velocity Seepage Outflow / Total Outflow	cm/sec %	0.97 0%	0.56	0.54 0%	0.54 0%									0.59 0%
Release 1 Outflow Volume Release 2 Outflow Volume	hm3/yr hm3/yr	0.0 0.0	0.0 0.0	0.0 0.0	0.0 0.0									0.0 0.0
95th Percentile Outflow Volume 95th Percentile Outflow Load	hm3/d kg/d	1.44 174.23	1.46 153.36	1.48 104.65	1.49 64.62									1.5 64.6
Simulated / Specified Mean Depth Release 1 Demand Met	%	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A									#N/A #N/A
Outflow Demand Met Range Check - Mean Dooth	%	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A									#N/A #N/A
Range Check - Freq Depth < 10 cm Range Check - Flow/Width		- 3.93	1.62											0
Range Check - Inflow Conc Range Check - Outflow Conc	1	-	-	1	1									0
Water Balance Error Mass Balance Error	% %	0.00% 0.00%	0.00% 0.01%	0.00% 0.01%	0.00% 0.01%									3.16% 0.57%
Warning or Error Messages		Cell# 1 EDCW D Cell# 1 EDCW F	epth out of calib. r low/Width out of ca	ange for EMG_3: alib. range for EM	99 vs. 35 - 76 cm G_3: 825 vs. 26 -	210 m2/day								4
		Bypass occurred Cell# 2 3 Flow/W	around cell 1 idth out of calib. ra	ange for EMG_3:	341 vs. 26 - 210 n	n2/day								



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## Table A.26 STA-1E: Case "3\_4 PSTADiv"

DMSTA2- Inputs & Outpu	ıts		Project: PROJECT_STA1E Model Release:											7/1/2005 09/06/05
Input Variable Design Case Name	Units -	Value 3 4 PSTA Div	Case Descrip	tion: f STA-1E with	western half of	East Distrit	oution Cell							
Input Series Name Starting Date for Simulation		TS_1_42006AI 05/01/65	Inflows from permitted by	C-51 West ad Draft Water C	justed for same ontrol Plan for	e-day bypas STA-1E (ta	s to S-155A	of L-8 run	off through s nual bypass	S-5AE, incr of 72,500 a	eased to ap icre-feet per	prox. maxim • year)	um	
Ending Date for Simulation Starting Date for Output	1	04/30/00 05/01/65	Cells 1 and	2 considered o	ff-line due to co	onstruction	and operatio	n of PSTA	Demonstra	tion Project	-			
Integration Steps Per Day Number of Iterations	1	4 0	Simulation Ty Output Variat	pe: <u>ole</u>	Uncertainty Ar Mean	halysis Lower CL	Upper CL		Diagnostic	s				
Output Averaging Interval Inflow Conc Scale Factor	days -	7 1	FWM Outflow GM Outflow 0	/ C (ppb) C (ppb)	17.0 13.5	22.6 18.9	12.8 9.4		H20 Balan Mass Bala	ce Error Me nce Error M	an & Max lean & Max	5.1% 0.8%	0.0% 0.0%	
Rainfall P Conc Atmospheric P Load (Dry)	ppb mg/m2-yr	10 20	Load Reducti Bypass Load	on % (%)	87% 0.0%	82%	90%	_	Iterations & Warning/E	& Converge rror Messag	nce jes	3	0.0%	
Cell Number> Cell Label	-	1 EDCW	2	3 4N	4 4S	5	6	7	8	9	10	11	12	
Vegetation Type Inflow Fraction	>	0.55	EMG_3	SAV_3	SAV_3									
Surface Area Moon Width of Flow Both	km2	0.95	2.38	4 2.61	3.04									
Number of Tanks in Series Minimum Depth for Releases	- cm	0.5	3.0	3.0	3.0									
Release 1 Series Name Release 2 Series Name														
Outflow Series Name Depth Series Name														
Outflow Control Depth Outflow Weir Depth	cm cm	90	40	60	60									
Outflow Coefficient - Exponent Outflow Coefficient - Intercept	-	4 1	4 1	4 1	4 1									
Bypass Depth Maximum Inflow	cm hm3/day													
Inflow Seepage Rate	(cm/d) / cm													
Inflow Seepage Conc Outflow Seepage Rate	ppb (cm/d) / cm	0.0095			0.0054									
Outflow Seepage Control Elev Max Outflow Seepage Conc	cm	-87 20			-38 20									
Seepage Recycle to Cell Number Seepage Recycle Fraction		4 1			7 1									
Seepage Discharge Fraction Initial Water Column Conc	- ppb	30	30	30	30									
Initial P Storage Per Unit Area Initial Water Column Depth	mg/m2 cm	500 50	500 50	500 50	500 50									
C0 = Conc at 0 g/m2 P Storage C1 = Conc at 1 g/m2 P storage	ppb ppb	3 22 200	3 22 200	3 22 200	3 22 200	3 22 200	3 22 300	3 22						
K = Net Settling Rate at Steady State	ррь m/yr	16.8	16.8	52.5 40	52.5 40	16.8	52.5	300 52.5						
Z2 = Lower Penalty Depth Z3 = Loper Penalty Depth	cm	100	100	100	100	100	100	100						
Output Variables	Units	1	2	3	4	5	6	7	8	9	10	11	12	Overall
Execution Time Run Date	sec/yr -	3.94 09/06/05	4.40 09/06/05	4.86 09/06/05	5.31 09/06/05									5.31 09/06/05
Starting Date for Simulation Starting Date for Output	1	05/01/65 05/01/65	05/01/65 05/01/65	05/01/65 05/01/65	05/01/65 05/01/65									05/01/65 05/01/65
Ending Date Output Duration	- days	04/30/00 12784	04/30/00 12784	04/30/00 12784	04/30/00 12784									04/30/00 12784
Cell Label Downstream Cell Label		EDCW 3	3 4N	4N 4S	4S Outflow									Total -
Simulation Type	- - km2	Uncerta	Uncerta	Uncerta	Uncerta 2.04									Uncerta
Mean Rainfall Mean FT	cm/yr	142.9	142.9	142.9	142.9									142.9
Cell Inflow Volume Cell Inflow Load	hm3/yr kg/yr	109.4 13233	103.6 11446	104.0 8923	104.3 3999									109.4 13233
Cell Inflow Conc Treated Outflow Volume	ppb hm3/yr	120.9 103.6	110.4 104.0	85.8 104.3	38.3 104.4									120.9 104.4
Treated Outflow Load Treated FWM Outflow Conc	kg/yr ppb	11446 110.4	8923 85.8	3999 38.3	1777 17.0									1777 17.0
Upper Confidence Limit Lower Confidence Limit	ppb ppb	112.7 107.6	92.8 77.7	47.1 30.2	22.6 12.8									22.6 12.8
Total Outflow Volume + Bypass Total Outflow Load + Bypass	kg/yr	103.6	8923	3999	104.4									104.4 1776.7
Bypass Load	kg/yr	0	0	0	0									0.0
Maximum Inflow Maximum Outflow	hm3/d hm3/d	1.95 1.94	1.94 1.94	1.94 1.93	1.93 1.89									1.95 1.89
Surface Load Reduction Load Trapped in Sediments	kg/yr kg/yr	1786 1190	2523 2604	4924 5013	2222 2325									11456 11133
Overall Load Reduction Lower Confidence Limit	%	14% 12%	22% 17%	55% 49%	56% 52%									87% 82%
Daily Geometric Mean	ppb	16% 105.0	28% 81.6	61% 32.9	58% 13.0									90% #N/A
Upper Confidence Limit	ppb ppb	105.5	81.1 88.30 72.80	33.5 42.2 25.5	13.5									13.5
Frequency Outflow Conc > 10 ppb Frequency Outflow Conc > 20 ppb	%	100%	100%	100%	82% 10%									82% 39%
Frequency Outflow Conc > 50 ppb Freq Outflow Volume > 10 ppb	%	100% 100%	100%	3% 100%	0% 94%									10% 94%
95th Percentile Outflow Conc Mean Biomass P Storage	ppb mg/m2	131 3945	94 3435	48 1924	22 766									22 2147
Storage Increase / Net Removal Net Storage Turnover Rate	% 1/yr	0% 11.1	0% 11.1	0% 34.9	0% 34.9									0% 20.2
Unit Area P Removal Mean Water Load	g/m2-yr cm/d	1252 31.5	1094	1921 10.9	765 9.4									1240 3.3
Max Water Load Mean Depth Minimum Depth	cm/a cm	205.3 93	60 22	74.5 66 26	66 29									21.7 68 26
Maximum Depth Frequency Depth < 10 cm	cm %	131	106	105	105									108
Flow/Width HRT Days	m2/day days	454 3.0	183 5.1	184 6.1	184 7.1									212.3 20.2
Mean Velocity Seepage Outflow / Total Outflow	cm/sec %	0.56 0%	0.35 0%	0.32 0%	0.32 0%									0.35 0%
Release 1 Outflow Volume Release 2 Outflow Volume	hm3/yr hm3/yr	0.0 0.0	0.0 0.0	0.0 0.0	0.0 0.0									0.0 0.0
95th Percentile Outflow Volume 95th Percentile Outflow Load	hm3/d kg/d	0.79 90.63	0.80 73.00	0.82 35.84	0.84 17.45									0.8 17.5
Simulated / Specified Mean Depth Release 1 Demand Met	%	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A									#N/A #N/A
Outflow Demand Met Bange Check - Mean Dooth	%	#N/A #N/A	#N/A #N/A	#N/A #N/A	#N/A #N/A									#N/A #N/A
Range Check - Freq Depth < 10 cm Range Check - Flow/Width	-	2.16	-	-	-									0
Range Check - Inflow Conc Range Check - Outflow Conc	1	-	1	1	1									0
Water Balance Error Mass Balance Error	% %	0.00% 0.00%	0.00% 0.00%	0.00% 0.01%	0.00% 0.01%									5.11% 0.78%
Warning or Error Messages		Cell# 1 EDCW D Cell# 1 EDCW F	epth out of calib. r low/Width out of c	ange for EMG_3: alib. range for EM0	93 vs. 35 - 76 cm G_3: 454 vs. 26 -	210 m2/day								2



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