OPERATIONS REPORT Of the South Miami-Dade Seasonal Operations For October 2009 through April 2010

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I. INTRODUCTION

The seasonal water level draw-downs of the coastal structures in the south Miami-Dade County area for agricultural purposes have occurred for over thirty years. The authorization is provided by specific South Florida Water Management District (SFWMD) Governing Board actions. The key affected structures are S-21A, S-20F and S-179. The specific operational actions are taken by the Water Management District (District) operations staff based upon field visits to agricultural operations, predicted and past precipitation events, observed water levels in the areas, and proposed planting schedules by local growers.

A. PURPOSE

The purpose of this report is to document, for the 2009/2010 dry season, the actual procedures, field conditions and observations affecting the seasonal operations.

B. BACKGROUND

The operations and maintenance of the S-21A, S-20F and S-179 structures is the responsibility of the South Florida Water Management District, acting as local sponsor for the federally authorized Central and Southern Florida Flood Control Project. These structures were designed and constructed by the U. S. Army Corps of Engineers (USACE). Structure S-21A was transferred from the USACE to the Water Management District for operations and maintenance in 1966 and S-20F and S-179 were transferred in 1967. The seasonal operations of these water control structures to facilitate agricultural practices in the South Miami-Dade County area has occurred for over 30 years. For this dry season (2009/2010) the Governing Board of the District authorized a modified operational range beginning on October 15th. Structures S-20F and S-179 were to be set at their low ranges whereas Structure S-21A would be set at an "interim" level with gate openings set at 2.0 feet NGVD and closings at 1.6 feet NGVD.

II. STRUCTURES

A. STRUCTURE S-21A

This is a concrete, gated spillway with two vertical lift gates located on the C-102 canal at its junction with the L-31E levee. The design of this structure was to pass the design flood flows without exceeding upstream design flood stages. It is designed to pass 1330 cfs with a design upstream stage of 1.9 feet NGVD.



Figure 1 – Structure S-21A

There are three operational ranges in effect for this structure: the High (upstream held between 2.2 and 1.8 feet NGVD), Intermediate (upstream held between 1.8 and 1.4 feet NGVD) and Low (upstream held between 1.4 and 1.0 feet NGVD) Ranges. Normal operations would schedule these three ranges as follows: High – from April 30th to October 15th; Intermediate – from December 30th to April 30th; and Low – from October 15th to December 30th. The selection of operational range is meant to be flexible based upon field conditions and agricultural activity.

For the 2009/2010 Dry Season an "interim" range of 2.0 to 1.6 feet NGVD was established. As long as rainfall persists within the basin, the low range setting will remain in effect until the water table recedes below the root zone of the seasonal crops. The details on structure operations are in Appendix A.

B. STRUCTURE S-20F

This is a concrete gated spillway with three vertical lift gates located near the mouth of C-103 at its junction with the L-31E levee. The design of this structure was to pass the design flood flows without exceeding upstream design flood stages. It is designed to pass 2900 cfs with a design upstream stage of 1.9 feet NGVD.



Figure 2 – Structure S-20F

There are three operational ranges in effect for this structure: the High (upstream held between 2.2 and 1.8 feet NGVD), Intermediate (upstream held between 1.7 and 1.3 feet NGVD) and Low (upstream held between 1.4 and 1.0 feet NGVD) Ranges. Normal operations would schedule these three ranges as follows: High – from April 30^{th} to October 15^{th} ; Intermediate – from

December 30th to April 30th; and Low – from October 15th to December 30th. The selection of operational range is meant to be flexible based upon field conditions and agricultural activity. As long as rainfall persists within the basin, the low range setting will remain in effect until the water table recedes below the root zone of the seasonal crops. The details on structure operations are in Appendix A.

C. STRUCTURE S-179

This is a concrete gated spillway with two vertical lift gates located on C-103 upstream of S-20F. The design of this structure was to pass the design flood flows without exceeding upstream design flood stages. It is designed to pass 1920 cfs with a design upstream stage of 3.8 feet NGVD. There are two operational ranges in effect for this structure: the High (upstream held between 3.9 and 3.1 feet NGVD) and the Low (upstream held between 3.1 and 2.7 feet NGVD) Ranges. Normal operations would attempt to maintain an optimum headwater elevation which varies seasonally. Most of the time the target optimum level is 3.5 feet NGVD. During the first month of the growing season (October to April) the automatic operation is set to the Low Range, then, depending on weather conditions, the high setting is used. During wet conditions the low setting may be used. As long as rainfall persists within the basin, the low range setting may remain in effect until the water table recedes below the root zone of the crops. The details on structure operations are in Appendix A.

III. OPERATIONS COORDINATION

During this period of operations (October 1, 2009 through April 30, 2010) the observed and documented communication protocols for structure operations involved the growers, the Homestead Field Station staff and the Operations staff at District headquarters. Several key agricultural sites were selected and monitored during this draw-down period. These sites and associated canals and structures are shown in Figure 3.



Figure 3 – Agricultural Indicator Sites

These four sites within the South Dade Agricultural Area were visited by field station personnel and photo-documented on October 7, November 9, November 25 and December 29 of 2009, as well as on January 11, February 3, March 15 and April 12, 2010. The photos are available for each site for background information. Prior to and after each visit the Director of the Homestead Field Station discussed with the on-duty operations manager the conditions observed, past and predicted precipitation, water levels and appropriate actions to be taken to minimize property damages. When the operations manager felt that conditions warranted structural operation changes, he would direct the Control Staff as such. His decisions were recorded in a set of official log books maintained in the Control Room. Example sheets from this period are included in Appendix B to show the types of operational decisions and actions taken.

IV. RAINFALL

A. ANTECEDENT CONDITIONS

The precipitation records for the period of June 1 through September 30th were evaluated to determine the general water resource conditions in the basins prior to the draw-down period. Rainfall at the Homestead Field Station was evaluated in relation to the previous 40-year period as shown on Figure 4.



Figure 4 – Rainfall at Homestead Field Station from June to September

The value for 2009 was 29.05 inches which was the 8th lowest in the period of record and more than 5 inches below the mean. This resulted in "naturally" lower water levels as the area entered into the draw-down period. The result is that the field conditions, as will be seen in the October 7, 2009 photos, were almost ready for cultivation with very little standing water remaining to be

removed. In accordance with the agreed operating criteria the structures were placed on lower operating ranges on October 15, 2009. A look at one of the indicator sites in Figure 5 gives a view of conditions prior to and during the draw-down. As seen on October 7th, the water levels are just below the surface but still too shallow to bring in equipment for cultivation, whereas by November 9th the field conditions were compatible with the necessary land forming and planting processes.



Figure 5 – Field Conditions at Site 1 on October 7, 2009 (Looking West)



Figure 6 – Field Conditions at Site 1 on November 9, 2009

Figure 7 gives a time-elapsed photographic journal of the field conditions at Site 1 throughout the draw-down period. It is important to note that even with the draw-down, which allowed for the land preparation and planting of seeds, there were still periods of significant above-ground ponding which impacted the productivity of the area. An additional meteorological condition that had an effect on the crops was a very unusual cold period during the winter that inhibited some crop growth success.



Figure 7 – Chronological Photography of Site 1

B. RAINFALL DURING DRAW-DOWN

The rainfall at the Homestead Field Station from October 1, 2009 to January 31, 2010 was 13.05 inches (shown in blue), which was only about one-half inch more than the mean (shown as multi-colored) for the period of record as shown in Figure 8a. The rainfall for the



Figure 8a - Rainfall for period of October through January at HFS

period of October 1, 2009 to April 30, 2010 was 22.6 inches (shown in red), which was 3 inches more than the mean (shown as multi-colored) as shown in Figure 8b. The daily distribution is shown in Figure 9. The area received over 5 inches of rain on November 25th which explains the extensive field flooding at Site 1 for that date shown in Figure 7.



Figure 8b – Rainfall for period of October through April at HFS



Figure 9 – Daily Distribution of Rainfall at HFS

V. WATER LEVEL DATA

The next series of Figures will show the headwater stages that occurred at the associated structures for the period of October 1, 2009 through April 30, 2010. An extensive surface and groundwater collection effort is underway by the District. Figure 10 shows the distribution of the monitoring sites.



Figure 10 – South Miami-Dade County Monitoring Stations



Figure 11 – S-21A Headwater during Draw-down Period



Figure 12 - S-21A Discharges during Draw-down Period

The management of the water levels during this draw-down period in relation to the multiple operating zones can best be viewed on individual plots for the structures. For example, Figure 11 shows the headwater stages held at S-21A in relation to its three normal operational ranges. It is clear to see that the use of an "interim" water level zone of 2.0 to 1.6 feet NGVD helped in the management of the water levels at S-21A such that the minimum amount of water was lost from the basin while balancing the needs of the individual growers. In reality, it can be seen that it was not really necessary to bring the water levels all the way down to the lowest range until the onset of the November 25th precipitation event. By lowering the stage by two tenths on October 15th the growers were able to get their equipment in and prepare the fields. One positive contributor to this somewhat unique opportunity was the significantly below normal antecedent rainfall season. This way the District water managers were able to balance water resource protection with the local agricultural requirements.

An interesting observation can be made from the discharge data shown in Figure 12. This plot is hourly data retrieved from the USACE web-site. If this were the only data downloaded then someone would get the impression that the gates on S-21A were open during a tailwater/headwater reversal caused by high tides and that a flow of over 80 cfs would have come from the saltwater side into the fresh (headwater) water side. By looking at the data for the month of October using this data (Figure 13) it can also be seen that the tailwater is higher than the headwater with anticipated backflow rates predicted. It can be seen that the telemetry data for the incident it can be seen that there was in reality very little reversal of flow as the gates automatically closed when the high tailwater was measured. The tailwater began to rise above the headwater at 19:57 on October 17th and the gates began closing and were fully closed 4

minutes later at 20:01. The point here is simply that assumptions cannot be made about structure operations based upon a limited data set (hourly in this case).



Figure 13 – HEC-DSS plot of S-21A data for October 2009

Figure 14 shows the headwater stages at S-20F for the full period of October 1, 2009 through April 30, 2010. The only unusual period is the morning of January 6th when the headwater stage fell below the bottom of the low range for a 7-hour period. There was no recorded precipitation event or written operational order that identified why. There may have

been a malfunction of the gate operator. Some detailed investigation discovered that there was a malfunction in the software (DDSP protocols) that was detected on that day and rectified within a short time period. There were no instances of back-flow as there was sufficient freshwater flow during the period to maintain a positive head balance.



Figure 14 – S-20F Headwater Stages

During the draw-down operations, as can be seen from Figure 14, it was not necessary to pull the stages down to the lowest operational levels because of the below-normal antecedent conditions and the fact that, except for the heaviest rainfall, the operations were adequately managed within the Intermediate Range. There were no complaints received from the agricultural community and the water resources upstream were preserved by these operations. As can be seen in Figure 15, ground water levels responded soon after the rise and fall of the nearby control structure.



Figure 15 – Relationship Between S-21A and G-3701 Levels

VI. SUMMARY

The management of the water levels in the South Miami-Dade County area during the period of October 1, 2009 through April 30, 2010 has been presented herein along with supporting data to demonstrate the overall system performance. The water control operations considered both the agricultural needs and the preservation of the water resources of the area with a balancing process. The system skills and experience of the Operation Managers and the Homestead field staff along with the field observations were critical for operating the system to optimize performance. Substantial data has been collected and is available for review. Within the appendices to this report are detailed operational guidelines, operational logs and field inspection photographs.

APPENDIX A

Detailed Structure Operations

STRUCTURE 20F

This structure is a reinforced concrete, gated spillway with discharge controlled by three cable operated, vertical lift gates. Operation of the gates is automatically controlled so that the gate hydraulic operating system opens or closes the gates in accordance with the operational criteria. The structure is located near the mouth of Canal 103 at its junction with Levee 31E and about 2000 feet from the shore of Biscayne Bay.

PURPOSE

This structure maintains optimum water control stages upstream in Canal 103; it passes the design flood (40 percent of the Standard Project Flood) without exceeding upstream flood design stage, and restricts downstream flood stages and discharge velocities to non-damaging levels; and it prevents saline intrusion during periods of high flood tides.

OPERATION

This structure will operate to maintain an optimum headwater elevation which varies seasonally from a low during the dry season of 1.4 feet to a maximum during the wet season of 2.2 feet, when sufficient water is available to maintain this level. The automatic controls on gates will function as follows:

High Range

When the headwater elevation rises to 2.2 feet, the gates will open at six inches per minute; When the headwater elevation rises or falls to 2.0 feet, the gates will become stationary; When the headwater elevation falls to 1.8 feet, the gates will close.

Intermediate Range

When the headwater elevation rises to 1.7 feet, the gates will open at six inches per minute; When the headwater elevation rises or falls to 1.5 feet, the gates will become stationary; When the headwater elevation falls to 1.3 feet, the gates will close.

Low Range

When the headwater elevation rises to 1.40 feet, the gates will open at six inches per minute; When the headwater elevation rises or falls to 1.20 feet, the gates will become stationary; When the headwater elevation falls to 1.00 feet, the gates will close.

The selection of the operational range will be based on the field conditions and agricultural activity. The approximate periods for the three modes of operation are:

High Range Operation (April 30th to October 15th) Intermediate Range Operation (December 30th to April 30th)

Low Range Operation (October 15th to December 30th)

Note: As long as rainfall persists within the basin, the low range setting will remain in effect until the water table within the basin recedes below the root zone of seasonal crops. This operating criteria is used for seasonal agriculture. It does not provide any protection for year-round agriculture below the high range settings, from April 30th to October 15th.

Salinity Regulation

In addition to maintaining optimum upstream fresh water control, as described above under Flood Control Regulation, the automatic controls on this structure have an over-riding control which closes the gates, regardless of the upstream level in the rare event of a high tide, whenever the differential between the head and tailwater pool elevations reaches 0.3 feet.

STRUCTURE 21A

This structure is a reinforced concrete, gated spillway, with discharge controlled by two cable operated, vertical lift gates. Operation of the gates is automatically controlled so that the gate hydraulic operating system opens or closed the gates in accordance with the seasonal operational criteria. The structure is located near the mouth of Canal 102 at its junction with Levee 31E and about a mile from the shore of Biscayne Bay.

PURPOSE

This structure maintains optimum water control stages upstream in Canal 102; it passes the design flood (40 percent of the Standard Project Flood) without exceeding upstream flood design stage, and restricts downstream flood stages and discharge velocities to non-damaging levels; and it prevents saline intrusion during periods of high flood tides.

OPERATION

This structure will be operated to maintain an optimum headwater elevation which varies seasonally from a low during the dry season of 1.4 to a maximum during the flood season of 2.2 feet, when sufficient water is available to maintain this level. The automatic controls on gates function as follows:

High Range

When the headwater elevation rises to 2.20 feet, the gates will open at six inches per minute; When the headwater elevation rises or falls to 2.0 feet, the gates will become stationary; When the headwater elevation falls to 1.8 feet, the gates will close at six inches per minute.

Intermediate Range

When the headwater elevation rises to 1.8 feet, the gates will open at six inches per minute; When the headwater elevation rises or falls to 1.6 feet, the gates will become stationary; When the headwater elevation falls to 1.4 feet, the gates will close at six inches per minute.

Low Range

When the headwater elevation rises to 1.40 feet, the gates will open at six inches per minute; When the headwater elevation rises or falls to 1.20 feet, the gates will become stationary; When the headwater elevation falls to 1.0 feet, the gates will close.

The selection of operational range will be based on field conditions and agricultural activity. The approximate periods for the three modes of operation are:

High Range Operation (April 30th to October 15th) Intermediate Range Operation (December 30th to April 30th) Low Range Operation (October 15th to December 30th)

Note: As long as rainfall persists within the basin, the low range setting will remain in effect until the water table within the basin recedes below the root zone of seasonal crops. This operating criteria is used for seasonal agriculture. It does not provide any protection for year-round agriculture below the high range settings, from April 30th to October 15th.

Salinity Regulation

In addition to maintaining optimum upstream fresh water control, as described above under Flood Control Regulation, the automatic controls on this structure have an over-riding control which closes the gates, regardless of the upstream water level in the rare event of a high flood tide, whenever the differential between the head and tailwater pool elevations reaches 0.2 feet.

STRUCTURE 179

This structure is a reinforced concrete, gated spillway with discharge controlled by cable operated, vertical lift gates. Operation of the gates is automatically controlled so that the gate operating system opens or closes the gates in accordance with the operational criteria. The structure is located on Canal 103 about 650 feet west of Tallahassee Road in Southern Dade County.

PURPOSE

This structure maintains optimum upstream water control stages in Canal 103; it passes the design flood (40% of the Standard Project Flood) without exceeding the upstream flood design stage, and restricts downstream flood stages and channel velocities to non-damaging levels.

OPERATION

This structure will be operated to maintain an optimum headwater elevation which varies seasonally. Most of the time, this optimum is 3.5 feet, when sufficient water is available to maintain this level. During the first month of the growing season (October 15 to the end of April), the automatic operation is adjusted to the low setting. And then, depending on weather conditions, the high setting is used. During wet conditions, that is, when the high setting would raise the water table into the root zone, the low setting is used. During the non-growing season, the high setting is used. The automatic controls function as follows:

Low Setting

When the headwater elevation rises to 3.1 feet, the gates will open at six inches per minute; When the headwater elevation rises or falls to 2.9 feet, the gates will become stationary; When the headwater elevation falls to 2.7 feet, the gates will close at six inches per minute.

High Setting

When the headwater elevation rises to 3.9 feet, the gates will open at six inches per minute; When the headwater elevation rises or falls to 3.5 feet, the gates will become stationary; When the headwater elevation falls to 3.1 feet, the gates will close at six inches per minute.

APPENDIX B

Control Room Operational Logs



"Avoid Verbal Orders" Pg. <u>1</u> of _____ [<u>Always Log ALL Water Managers' "Phone-In" Directives Below.</u>] Date: ______/0/15/09

Memo To	Init.	Directives / Remarks	Why	Time	Init.
FTL	PFL	OPEN (6') 5144, 5145, 5146		07:24	nt
WPB	PPL	C-18 DAJAL RAISE STRUCTURE		06:41	の必
		LEENT OF ~ 16.9 AT C-14A & C-12		1	1
OPPS	BFL	568MS GATE #2 to 0.40' DDSP ON		05:35	JR
1	1	582 BOTH 6ATTS TO 0.15'		1	1
		575 OPEN GATE TO 0.30'			
		570 GATE #1 TO 0.10-			
		538 GATE #1 TO 0.50'			
*	t	537B GATE #1 TO 0.20'		4	*
OCL	zc	G342EF set DDSp to M3/5.5/10/14.0/0.2/01		0124	A
i	V	Dasj on.		1	1
clew FS	zc	G151, G152 Set top of the beard elevition	C603	08:54	CB
		at 18.5		1	1
J		GISD Set are given to 0.3		08:54	CP
OFFE	FFL	5390 PUMP REG SHIET TOPAY &		09:45	GB
ł	4	FRIDAY . OFF WHEN S.391 HW 224.1		1	6
OCC	zc	6409 Run pumps to Keep TW 13.5-13.8	0414	09:54	6B
WPB FS	ZC	S319 pump one unit (550 cfs) regular shift		69:58	CB
L	1	tomame. If Hw = 11.0, secone pumping		4	6
100	W	S20F set to low DDSP	C603	1206	84
1	1	SITS set to low DDSP	1	1	1
1	×	521A set M3 /50/2.0/1.6/0.3/0.1	\downarrow	1	1
220		540, 541, SISS to High BongE		12:45	CB
OLL	PPL	STO GATE 2 FROM 0.0 TO 0.1		1311	×
OCL	PFL	SING CLOSE BOTH GADE			1

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6		"Avoid Verbal Orders" [Always Log ALL Water Managers' "Phone-In" Directives Below.]	Pg.	<u>1</u> of	
		Date: <u>11</u>	124/8	09	
Memo To	Init.	Directives / Remarks	Why	Time	Init.
Dos	PPZ	5179 Open 2 @ 1.0'		0502	9
11	1	Let dosp run site		1	1
Dob	ph	S82 open #1 b clo to ,50'		0636	\$
1	1	STS spen agte & che to . 50'		1	1
OCC	PFL.	SIJAN to Normal DDSp open gate to 0.50		07 "	pr.
Have	PFL	SZIA OPEn gates White if commany he		0795	ん
		down		Ĺ	L
OCC	PFL	5381 open 3 gates to (11,50)		0749	nul
FfL	PFL	59 Pump to keep Hw 3.50-300'		*	*
ac	AFL	529, custom NASP, M3, 5, 2, 1-5, 0.3, 0.1		0847	RB
αε	PFL	SIZ DDSP -> Low		047	RB
ouc	Pril	5179 open 2 rates to 3.00 Kpddspo	и	08:49	his
WPB 3	ema	5-6 test pap ok replan shift	(419	09:05	94
Clewich,	Sat	6-372 pap popular shiff	<427	09:14	Hu
occ	417	5-40, 5-41, 8-155 to normal DUSP	(60)	09:10	quy
NPR 3	417	5-362 page regular shift	C409	09 16	N
ouc	pri	SI79 open 2 gates from 3.00' to y.co'		0951	Ry
Ga	PFL	Sizy to DDSP M3/13.5/2.5/2.1/03/01		10 66	N
	-	gren gates to 6.0		t	1
Hmsl	PFC	Sigt open 3 culurach open hull		10 00	p
cton: 14	GAD	G- 370 pay regulas shift	C427	1010	n
		G-\$07 Les papir today		1	1
	Г	6-409 will not pup on Thes. Sot.			
		Son Sun will prop on Fri			
		G-410 will out pro Thurs Son		L	1

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6)	"Avoid Verbal Orders" [<u>Always Log ALL Water Managers' "Phone-In" Directives Below.]</u> Date:	Pg.	1 of_ 109	
Memo To	Init.	Directives / Remarks	Why	Time	Init.
NB	FC	RUN I SSO CES DUMP		asm	K
Í	1	FOR CHAISSIONS JEST BODEOX Y br		1	1
üke	PF(5135 OPEN 2 galo to 10		079	ph
OIN		St5 close gale to 01		1	L
Dec	zc	Gibi set all give to 0.2. DDSP off		D832	Ber
WPB FS	zc	S319 when emission test starts, sigher to		0837	Que
		provide water for the pumping. Stop siphon		1	
V	V	when some emission test finishes		4	+
clew FS	zC	G 507 Secure pumping		1031	RUS
000	zc	5351 From 3@ 1.05 to 3@ 1.5 DOSP on	c603	10:50	10
l	L	5:351 DOSP to M41.5/10.6/10.4/02/01 DDSPON	C603	11:10 0	CB
Dec	M	SZIA set DDSP to low range	C603	14:29	Ry
DIC	ZC	5363 close getes and cancel DD3P	C409	14:50	CB
1	1	when 5319 pump emission test ends. set		15:48	CB
		5.55A DDSp to M4/0.6/8.7/8.6/02/0.1 and			1
l		tum onsp on			V
occ	PFL	SITT GATE FROM 0.3 to 0,2		17:10	CB
QCL	PFC	SIBC BOTH GATES 0,8 to 0.4		17:12	CB
0<0	PFL	S38 DOSP ON		17:05	C13
ore	ma	SIFT more gathe from 0.20 to 0.10		2134	MUT
occ	ZC	SI55A TO HIGH DDSP		22:25	nis



Pg. ____ of ___

Date: UNID

Memo To	Init.	Directives / Remarks	Why	Time	Init.
WIB/FS	PFL	PB Aggrey LS reservoir open North	/	06:44	CB
		bate from 0.40 to 0.60 feet.	1	1	1
àc	ZC	SBAW move 2 gata to o.g. DOSP off		d 36	W
DCC	PFL	SSAE open 2 Gates to 0.20 Feet		06:43	CB
204	PFL	576 200,10' to #Zeclosed		0759	4
1		575 #1@.10'		1	1
		582 2@ 0.1 to 0.2'			1
BUC	ZC	5750 close gates and cancel DIDSP	C409	08:04	46
OCL	zí	G56 DDSp to "High" DDSp on		0814	Кß
	1	5361 pump to keep Hw 10.5-100	6409	2835	68
		At 1/12 6.00, try to get Hw to		1	c
		# 100. (No flow on Tuesday for 24 hors			
J	\downarrow	for the construction work)	ł		1
₽ ¢ ¢	zc	SISSA DOSP to low	(603	als	FB
occ	12	SZOF set to (intermediate) normal	C603	1027	Or
1	1	DUSP	1	1	
t	Ý	521A set to normal DDSP	t	1	D
Ł.	V	521 to hope DDSP	V	10:38	<i>fb</i>
stold	PF	360 FROM 0,5 TO 0.1		1140	4
OCC	FFL	563 FROM 0.2 to 0.0)	
occ	PFL	537 BOTH GATES 0.9 to 0.9			
Bil	PAL	561 FROM 0.3 to 0.4		l	
FTL	PR	SIHY, SIYS, SIYC OPENTS 6.0' BY 1500 TOES		255	46
GLL	PPL	538 DDSP ON BOTH GATES TO 0.4'		255	AS
OLL	PFL	6160 GATEL TORO. TO 0.1'		1255	KS



Pg. ____ of ____

es be/ow.		The second second	
Date: 02	101	12010	

Memo To	Init.	Directives / Remarks	Why	Time	Init.
ikee	PFL	5131 PUMP REGULAR SHIFE off P 12.75		0630	N
	PFL	Sizg Pump REGILAR ALIFL ALGIZ.75		1	1
occ	ZC	SISO Reature DIDSP		0815	RB
V	V	540, 541, 5155 DIDJP to Normal" DDSP on	c603	1	1
1	1	G384 Cancel DDSP. (The all		0827	RS
V	V	gates to 7.0			1
J.	\downarrow	G259 DOSP to M3/2.0/11.5/11.3/02/01. DOSPON	64.9	1.	1
Ý	V	SI50 More gales to 5.5 Dissp on	1	0854	HB
FTL	he	59 pump regular slift	C427	0917	to
Itew FS	ZC	G370, G372 pump night shift	C427	0943	RB
NPB FS	ZC	56, SSA pump nyet shift	C427	0943	NB
FTL	Me	S13 pump regular shift	c427	1001	D
1	1	529 set to low DDSP	C603	1015	RZZ
		526 set to low DDSP	1		1
		S25 sets to low DDSP			
		525B set to low DDSP			
	J	\$793 set to low DDSP	V	V	V
V		522 set to	1		
Occ.	INL	520F set to low DDSP		1041	RB
1	1	521A set to las DDSP	V		1
ì	V	\$173 open 1.5' keep DDSp on	1	1042	KS
Dec	in	S381 open delates to 3@-115'	C427	100	H
OCC	At	- S30 set to low DDSR	1		
J.	14	G54 set to low DDSP	V	11/4-	FB
1154	70	"-370 DUMO TELT OF TEG SHIFT +	C427	1154	9



<u>es Below.]</u> Pg. <u>3</u> of _____ Date: <u>02-02-2010</u>

Memo To	Init.	Directives / Remarks	Why	Time	Init.
elew FS	ZC	6370			
ace	m	S20F set to normal DDSP	C603	1532	60
Ļ	4	SZIA set to normal RDSP	L	1	1
G324	ZS	G3=4 More all gates to to, Turn on	c409	1500	Mot
Ý	V	DOSP	C		
	1	G303 More all gates to 4.0 Keep DDSP			1
V	V	on	V	Ļ	4
as	74	6334 11.21/10.99 202 to 201.	þ	2076	M
		and to m3, 1, 11.10, 10.90, 0.20, 0.1		h	¥
4	J	6332 to old by max gate 1.0'	_	\$	1
000	MI	693 operate manually due to bad The	,	2150	M
		using ddsp setting as for open open			
		and doring stage and 525B tw			
W/	4	as fasis for tw.		4	+



Pg. <u>1</u> of _____ 2/24/10

Date: ____

Memo To	Init.	Directives / Remarks	Why	Time	Init.
de	PR	559 Roduce gate fr 2.5 to 2.0		0740	RB
de	PFL	S61 Reduce gate &r 40 to 3.5		0760	RB
oci	z.C.	540, 541, 515 DDSP to "Nome", DDSP on	6603	0936	NB
1	1	G56 More all available gates to 0.5. DOSP	1	Divers	0
V	V	an	V	site	
1	1	GBB G374 DOSP to MB/75/14.5/140/0.2/01.	C409	0940	45
	V	Dosp on	V	1	
V	V	5351, 5352, 5354 DDSP off	C6.3	0941	HS
OCL	zC	G343EH DDSp max gate to 0.5 DDSP on	c409	0033	15
020	PFL	520P LOW REGITE		1036	MA
FTL	mt	59 pump night shift	C427	1137	MA
1	4	513 pump night shift	4	1133	M
OCC	zc	SISSA DOSP to "Low". DDSP on	(6.3	1140	HS.
= lew Fs	zc	G370, G372 pullup extended regular shift today	C427	1203	HS
WIB FS	26	56 pump extended regular shift today	0427	1204	PS
DCL	zc	G56 D05p to M3/8=15.0/8.2/7.5/0.3/0.1	C6.3	1804	145
L	1	Dosp on			1
Dec	M	5381 open to -11.5 (deflate)	C603	1325	お
Clew FJ	20	G372 S# secure pumping now	6417	1712	by
nac	MI	513 AW set ddsp M3/1.00/30/2.5/1.(1.4	/	19:46	py
4	4	and open 2 gates to 0.50'		-11	4
		, 0			



Pg. ____ of ____

Date: 2/26/10

Memo To	Init.	Directives / Remarks	Why	Time	Init.
DCC	ZC	6353AB cuncel DDSp due to		06:57	CB
		WORK AT STA-60		1	6
ocic	20	6396 cancel DDSp. due to		06:57	CB
		WORK at STA-6:		1	1
occ	25	6352 cancel posp due to		20:50	CB
		WORK at sta-6-		k	6
N(C	PFL	582 OPEN 2 60100 10 0.10		07:29	CB
DCC	Pfl	5131, 5129 pump recolar shift		07:31	CB
V	4	leday off 12.75 Feet		1	h
ac	PPL	557 both gates & 3.0 ito 3.5		0755	RB
	PFL	565 Reduce gates 23,4,5 to 1.5'		6757	MB
occ	M	5381 close gates to + 5.5' (mflate)	C603	0820	m
occ	ZC	6,351, 6352, 6354 Toosp on	c6=3	0827	KB.
OLC	ZC	STA6 tempte Rente control of structures		0924	on
1		between 7:00 am and 13 17:00 Pt not			1
		available during week days (Mon Fri).			
		Remote control available between 17:00 p	h		\square
		and 7:00 am weekdays and all the			\square
		weekond. During the period of no-remo	2		\square
		copiel, caused DDSP. When remote courted	-		\square
V	V	is wailable, restrie DDSP at G396.		4	4
POT -	m	5331 secure pumping, revert to	C603	9:19	Tr
¥	¥	syphoning and keep how 5.0' ~ 5.5'	4	6	4
Sec	+	\$20F to normal DDSP	*	9:19	Sh
acc	ZC	when s6 pumps, set		1055	143



es Below.] Pg. ____ of ____ Date: _______////2010

Memo To	Init.	Directives / Remarks	Why	Time	Init.
046	PFL	SIS FROM CLOSE TO 1,2		1457	Ad
021	PFL	SEL FROM CLOSE TO Q.5			ad
066	FFL	S65 FROM 0.3 TO 1. & FOR GATOS 2.3, 4,5		L	nel
operel	sik	9-56 Enneel NPSJ, ope 262'	c603	1536	N
i)seck	call	5-207, 5-21/4 to low DDSP,	C60 2	1630	N
		(consult with Scott bend up rain		1	1
		forseasted)		L	1
WPB	IN	5319 pump regular shift formorrow	C427	1631	pr.
OPL	PF(Sizy to Low RANKS		1145	AL.
ope	PFC	559 pate From 1.7 to 4.0.		1700	pr
	t	561 got from 25 to 60.		1	1
Lackt	PFC	565 9060 2345 Am 10 to 2.0.			1
		Remail SGAR of gate change @ 565		BR.	V
upert	any	5-97 × 2005 to 201	(6-3	1726	N
		set 5t to normal DDSP		1	
		6-56 to parmal 1305th		4	t.
561 grow	:Filmer	"I'EI to man sute to 5.0' did to lad		1.194.F	M
V	V	centor roly.		J	Ļ
fre	Khia	199 to Normal dd Ap		2027	MJ
1		- (6Pmsconner (ddsp			
		SB2 wave gate to 1.0 st			
J	4	(63 more gete to 20 Ht .	-		
J	J.	549 change do 193 3. 20. 20 18.500.	0 10"	1.V	4
	641	\$155 All gates to 1.0. DOSP ON		1/17	143
	MI	SISSA HI gates to 2.8 DASPON		1117	143



Pg. ____ of ____

Date: 3/13/2010

Memo To	Init.	Directives / Remarks	Why	Time	Init.
FFL	FFL	Sido extended shift until 1800.	6422	cg "	N
		Pump Regular shift on Sunday		1	1
		Sq Decure End Regular shift.			
		SI3 DECURE End Regular shift.			
Okee	PFL	SI35 pump Regular shiff. off 13.25	C421		
		SIZZ pump regular shift off 13.30.	1		1
		5131 N.S			
		Sny N.S.			1
200	GH	549 net NORMAL REGIMENT.		04:40	CB
04	MI	5169 close fates and cancel Dosp		09:45	CB
OCC	GH	520F SLIA TO Normal DDSP		10 04	pr.
Okee	PFL	5135 5127 5133 N.S		1027	r
		S 131 exiter dad shift til 1800		J.	1
Clew	H1	52,53 PUND Night shift	427	1150	CB
clew	MZ	6370, 6372 Pump Night shift	C427	1150	CB
Ft/Luu	NZ	58 secore after recolar shift today	c409	1150	Ŵ
WPB	MI	56, SSA, and SBIA PUMP NIGht shift	c427	1152	CB
NPB	MI	6335, G310, and S36Z PUMP Night shift	(409	1152	B
clew	11	54 PUNP NIGHT SHIFT	C 427	1150	CB
oc	MJ	5365A Set M3/2-0/12,50/12-0/020/010		1204	CB
Occ	PFL.	Sty got 1 to 100 Disp on		152	N
Kisslack	PFL	565 increase butes 1-to5 23.10' to 1 to5 e	4.01	1520	CB
LUCASH. CONST	PFC	549 close # gate from 1.50' to 0.50'	C683	17:05	py
oee	PFC	SIZAW INCREASE MAX. from 2.0 to 3.0'		16.45	S.K
V		apon both gate from do to 2.5	X	V.	V

APPENDIX C

Field Inspection Documentation



October 7, 2009



November 9, 2009



November 25, 2009



January 11, 2010



February 3, 2010

SITE 1



October 7, 2009



November 9, 2009



November 25, 2009



December 29, 2009



January 11, 2010



February 3, 2010

Site 2



October 7, 2009



November 25, 2009



January 11, 2010



November 9, 2009



December 29, 2009



February 3, 2010

Site 3